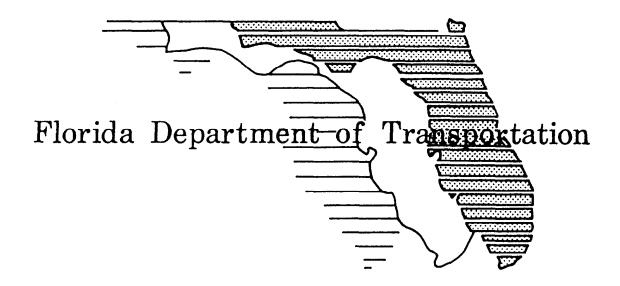
ROADWAY PLANS PREPARATION MANUAL VOLUME I

DESIGN CRITERIA AND PROCESS



DOCUMENT NO. 625-000-101-B ROADWAY DESIGN OFFICE TALLAHASSEE, FLORIDA 1989

ROADWAY PLANS PREPARATION MANUAL

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TABLE OF CONTENTS

VOLUME I

Design Criteria and Process

Introduct	<u>tion</u>		xiii
Chapter :	<u>l</u> De	sign Parameters	
1.1	Ge	neral	I-1- 1.0
1.2	Tr	affic	I-1- 2.0
1.3	Ca	pacity and Level of Service	I-1- 3.0
1.4	Ro	adway Functional Classification	I-1- 4.0
1.5	3R	Design	I-1- 5.0
1.6	De	sign Consistency and Driver Expectancy	I-1- 6.0
1.7	Ae	sthetics	I-1- 9.0
Chapter :	<u>2</u> Ge	ometrics	
2.1	st	andards	I-2- 1.0
	2.1.1	Fixed Standards	I-2- 1.0
	2.1.2	Design Standard Ranges	I-2- 2.0
2.2	La	ne Widths	I-2- 4.0
	2.2.1	Through Lanes	I-2- 4.0
	2.2.2	Transitions of Pavement Widths	I-2- 5.0
	2.2.3	Wide Curb Lanes	I-2- 6.0
	2.2.4	Other Lane Widths	I-2- 8.0
	2.2.5	Bicycle Ianes	1-2- 8.0
	2.2.6	Shoulder Widths	I-2- 9.0
	2.2.7	Limits of Friction Course on Shoulders	I-2-12.0
2.3	ot	her Cross Section Elements	I-2-14.0
	2.3.1	Cross Slopes	I-2-14.0
	2.3.2	Roadway Structure	I-2-14.0
	2.3.3	Geometry of Side Slope and Side Ditches	I-2-14.0
	2.3.4	Border Width	I-2-14.0

2.4	но	rizontal Alignment	1-2-17.0
	2.4.1	General	I-2-17.0
	2.4.2	Supplemental Alignment Control	I-2-17.0
	2.4.3	Two-Lane to Four-Lane Transitions	I-2-20.0
2.5	Ve	rtical Alignment	I-2-21.0
2.6	Su	perelevation	I-2-22.0
2.7	Cr	oss Sections	I-2-23.0
2.8	In	tersections and Interchanges	I-2-24.0
2.9	Li	mited Access Limits at Interchanges	I-2-25.0
2.1	0 De	sign of Rural Projects	I-2-26.0
	2.10.1	Profiles	I-2-26.0
	2.10.2	Cross Sections	I-2-28.0
	2.10.3	Preliminary Review	I-2-29.0
2.1	1 De	sign of Urban Projects	I-2-30.0
	2.11.1	Preparation	I-2-30.0
	2.11.2	Geometrics	I-2-31.0
	2.11.3	Profiles	I-2-32.0
	2.11.4	Cross Sections	I-2-35.0
	2.11.5	Preliminary Review	I-2-35.0
Chapter	<u>3</u> Ea	rthwork	
3.1	Во	rrow Excavation	I-3- 1.0
	3.1.1	Borrow from within the Right-of-Way	I-3- 1.0
	3.1.2	Earthwork Paid for as Embankment	I-3- 1.0
	3.1.3	Measurement of Borrow Excavation	I-3- 2.0
3.2	Su	bsoil Excavation	I-3- 6.0
3.3	Cl	earing and Grubbing	I-3- 6.0
3.4	Fi	nal Dressing	I-3- 6.0
Chapter	_4 Ro	adside Safety	
4.1	Cl	ear Zone	I-4- 1.0
	4.1.1	Clear Zone Concept	I-4- 1.0
	4.1.2	Clear Zone Criteria	I-4- 2.0
	4.1.3	Early Removal of Fixed Object Hazard	I-4- 2.0
4.2	Ca	nals	I-4- 5.0

4.3	1	Roadside Barriers	1-4- 8.0
	4.3.1	Warrants	I-4- 8.0
	4.3.2	Barrier Selection	I-4- 9.0
	4.3.3	End Treatments	I-4- 9.0
	4.3.4	Transitions	I-4-11.0
	4.3.5	Placement	I-4-11.0
	4.3.6	Resetting Guardrail	I-4-12.0
4.4	1	Median Barriers	I-4-15.0
	4.4.1	Warrants	I-4-15.0
	4.4.2	Selection	1-4-15.0
	4.4.3	End Treatments	I-4-15.0
4.5	(Crash Cushions	I-4-16.0
	4.5.1	Warrants	I-4-16.0
	4.5.2	Selection	I-4-16.0
	4.5.3	Design	1-4-17.0
4.6]	Roadside Appurtenances	I-4-19.0
	4.6.1	Sign Supports	I-4-19.0
	4.6.2	Mailbox Supports	I-4-20.0
	4.6.3	Other Appurtenances	I-4-20.0
	4.6.4	Location Criteria	I-4-20.0
Chapter	<u>5</u> 1	Utilities	
5.1	(General	I-5- 1.0
5.2	1	Relocation	I-5- 2.0
5.3	1	Utility - Accommodation Guide	I-5- 3.0
5.4	•	Verification of the Location of	
]	Major Existing Utilities	I-5- 4.0
	5.4.1	Locating Underground Utilities	I-5- 5.0
5.5	•	Coordination Process	I-5- 8.0
Chapter	<u>6</u>	Railroad Crossing	
6.1	(General	I-6- 1.0
6.2	•	Devices	I-6- 2.0
6.3	;	Surfaces	I-6- 3.0
6.4	•	Clearance at Railroad/Highway Structures	I-6- 4.0

Chapter 7	Signing, Marking, Lighting and Signals	
7.1	General	I-7- 1.0
7.2	Signing and Marking	I-7- 2.0
7.2.	1 Design Criteria	I-7- 3.0
7.2.	2 Project Coordination	I-7- 5.0
7.3	Lighting	I-7- 6.0
7.3.	1 Design Criteria	I-7- 6.0
7.3.	2 Conventional Lighting Criteria	I-7- 7.0
7.3.	3 High—Mast Lighting Criteria	I-7- 8.0
7.3.	4 Pole Design Criteria	I-7- 8.0
7.3.	5 Underdeck Lighting Criteria	I-7- 9.0
7.3.	6 Foundations Criteria	I-7- 9.0
7.3.	7 Wind Loading Criteria	I-7- 9.0
7.3.	8 Lighting Project Coordination	I-7-11.0
7.4	Traffic Signals	I-7-14.0
7.4.	1 Design Criteria	I-7-14.0
7.4.	2 Certification and Speciality Items	I-7-15.0
7.4.	3 Stop Line Location	I-7-15.0
7.4.	4 Controller Timing	1-7-16.0
7.4.	5 Left Turn Treatments	I - 7-16.0
7.4.	6 Signal Pre-emption	I-7-19.0
7.4.	7 Intersection Design - Lane Configuration	I-7-19.0
7.4.	8 Signal Loops	I-7-22.0
7.4.	9 Wind Loading	I-7-22.0
<u>Chapter 8</u>	Bicycle and Pedestrian Facilities	
8.1	General	I-8- 1.0
8.2	Sidewalks	I-8- 2.0
8.3	Handicap Access	I-8- 4.0
8.4	Bicycle Facilities	I-8- 5.0
Chapter 9	Landscaping	
Q 1	Ceneral	T-9- 1.0

Chapter 10	Work Zone Traffic Control	
10.1	General	I-10- 1.0
10.2	References	I-10- 2.0
10.3	Comprehensive Work Zone	
	Traffic Control Planning	I-10- 3.0
10.4	Traffic Control Plans	I-10- 5.0
10.5	ICP Development	I-10- 8.0
10.6	Coordination	I-10-12.0
10.6.	1 FHWA Review	I-10-12.0
10.6.	2 TCP Revisions After Letting	I-10-13.0
10.7	Work Zone Traffic Control Training	I-10-14.0
10.7.	1 Background	I-10-14.0
10.7.	2 Training Requirements	I-10-14.0
10.8	Traffic Control Devices	I-10.16.0
10.9	Signs	I-10-17.0
10.9.	1 Advance Warning Signs	I-10-17.0
10.9.	2 Length of Construction Sign	I-10-17.0
10.9.	3 Sign Covering	I-10-17.0
10.9.	4 Existing Signs	I-10-18.0
10.9.	5 Construction Identification Sign	I-10-18.0
10.10	Lighting Units	I-10-19.0
10.10	.1 Warning Lights	I-10-19.0
10.10	.2 Advance Warning Arrow Panels	I-10-20.0
10.10	.3 Variable Message Signs	I-10-21.0
10.10	.4 Traffic Signals	I-10-24.0
10.11	Channelizing Devices	I-10-25.0
10.11	.1 Type III Barricades	I-10-25.0
10.11	.2 Separation Devices	I-10-25.0
10.12	Pavement Markings	I-10-26.0
10.12	.1 Removing Pavement Markings	I-10-26.0
10.12	.2 Reflectorized Raised Pavement Marker	I-10-26.0
10.12	.3 Temporary Pavement Markings	I-10-26.0

10.13 Sa:	fety Appurtenances for Work Zones	I-10-27.0
10.13.1	Traffic Barriers	I-10-27.0
10.13.2	Portable Concrete Safety Shapes	I-10-27.0
10.13.3	End Treatments	I-10-28.0
10.13.4	Modifications of Existing Barriers	1-10-28.0
10.13.5	Crash Cushion	I-10-28.0
10.13.6	Truck Mounted Attenuator	I-10-29.0
10.14 Fla	aggers	I-10-31.0
10.14.1	General	I-10-31.0
10.14.2	Location of Flaggers	I-10-31.0
10.14.3	Police Agencies	I-10-31.0
10.15 Lay	youts	I-10-32.0
10.15.1	Taper Lengths	I-10-32.0
10.15.2	Intersecting Road	
	Signing and Signals	I-10-33.0
10.15.3	Sight Distance to	
	Delineation Devices	I-10-34.0
10.15.4	Pedestrians and Bicyclists	I-10-34.0
10.15.5	Superelevation	I-10-34.0
10.15.6	Lane Widths	I-10-35.0
10.15.7	Iane Closure Analysis	I-10-35.0
10.15.8	Detours	I-10-47.0
10.15.9	Above Ground Hazards	1-10-48.0
10.15.10	O Drop offs in Work Zones	1-10-49.0
10.15.1	l Narrow Bridges	I-10-50.0
10.16 Sp	eed Zoning	I-10-51.0
10.16.1	Regulatory Speeds in Work Zones	I-10-51.0
10.16.2	Advisory Speeds in Work Zones	I-10-55.0
<u>Chapter 11</u> Pag	y Items, Quantities and Specifications	
11.1 It	ems and Quantities and Specifications	I-11- 1.0
11.2 New	w Pay Items	I-11- 4.0

Chapter 1	<u>2</u> Ri	ght-of-Way	
12.1	Ger	neral	I-12- 1.0
Chapter 1	<u>3</u> Proje	ect Development	
13.1	Ger	neral	I-13- 1.0
13.2	Pro	ocess	I-13- 1.0
	13.2.1	Five Year Work Program	I-13- 1.0
	13.2.2	PD & E Process	I-13- 2.0
	13.2.3	Final Design	I-13- 3.0
	13.2.4	Project Letting	I-13- 3.0
13.3	3R	Design	I-13-5.0
13.4	In-	-House (DOT) Design	I-13- 5.0
13.5	Con	nsultant Design	I-13- 6.0
13.6	Pro	oject Scheduling	I-13- 7.0
Chapter 1	<u>4</u> Dat	ta Collection	
14.1	Ger	neral	I-14- 1.0
14.2	Pro	oject Data	I-14- 1.0
	14.2.1	Project Description	I-14- 1.0
	14.2.2	Project Number	I-14- 2.0
	14.2.3	Project Cost	I-14- 2.0
	14.2.4	Production Schedule	I-14- 2.0
14.3	Des	sign Data	I-14- 3.0
	14.3.1	Surveys	I-14- 3.0
	14.3.2	Traffic Data	I-14- 5.0
	14.3.3	Pavement Design	I-14- 5.0
	14.3.4	Environmental Documents	I-14- 5.0
	14.3.5	Original Plans	I-14- 6.0
		Accident Data	I-14- 6.0
14.4	Lis	st of Requests and Contracts	I-14- 7.0

Chapter 15	Scheduled Submittals	
15.1	General	I-15- 1.0
15.2	Design Plans Phase Review Submittal	I-15- 2.0
15.	2.1 Plans Disposition	I-15- 3.0
15.3	Other Submittals and Requests	I-15- 5.0
15.	3.1 Structures	I-15- 5.0
15.	3.2 Surveys	I-15- 8.0
15.	3.3 Typical Sections	I-15- 8.0
15.	3.4 Pavement Design	I-15-13.0
15.	3.5 Permits	I-15-13.0
15.	3.6 Right-of-Way Engineering	I-15-15.0
15.	3.7 Value Engineering	I-15-17.0
Chapter 16	Plans Review	
16.1	General	I-16- 1.0
16.2	Design Plans Phase Review Checklist	I-16- 1.0
16.3	3 R Projects' CheckList	I-16- 2.0
16.4	Field Reviews	I-16- 2.0
Chapter 17	Quality Assurance	
17.1	General	I-17- 1.0
17.2	Authority	I-17- 1.0
17.3	Areas of Responsibilities	I-17- 2.0
17.4	Design Functions	I-17- 2.0
17.5	Documentation	I-17- 3.0
17.6	Consultants Role	I-17- 3.0
17.7	Plans Processing	I-17- 4.0
Chapter 18	Pay Items	
18.1	General	I-18- 1.0
18.2	Pay Items	1-18- 1.0
18~3	CES	I-18- 1.0
18-4	Computations Book	I-18- 2.0
18-5	Contract Time	1-18- 2.0
18-6	Shop Drawings	I-18- 2.0

Chapter 19		Sig	ning and Sealing Design Drawings	
19.1		Gen	I-19- 1.0	
19.2		Sig	ning and Sealing of Plans	I-19- 2.0
	19.2.	. 1	Signing and Sealing of Manually	
			Drafted or Partially CADD Produced Plans	I-19- 2.0
	19.2.	.2	Signing and Sealing of CADD Produced Plans	I-19- 3.0
	19.2.	. 3	Signing and Sealing Revisions	I-19- 4.0
19.3		Sig	ning and Sealing Other Engineering	
		Do	ocuments	I-19- 6.0
19.4		Sup	plemental Specifications and	
		Sp	pecial Provisions	I-19- 8.0
19.5	•	Inf	formation Requiring Certification	I-19- 9.0
	19.5	.1	18 KIP Equivalent Single	
			Axle Loads	I-19- 9.0
	19.5	.2	Project Traffic	I-19-10.0
Chapter	<u>20</u>	Pla	an Revisions	
20.1	-	Ger	neral	I-20- 1.0
20.2		Rev	vision Process for Each Stage	I-20- 2.0
	20.2	.1	Signed and Sealed	I-20- 2.0
	20.2	.2	Signed Sealed and Shipped	I-20- 2.0
	20.2	.3	Let to Contract	I-20- 7.0
20.3		Aut	tomated Plans	I-20- 9.0
Chapter	<u>21</u>	Con	nsultant Project Management	
21.1	•	Ger	neral	I-21- 1.0
21.2	!	Pro	oject Definition	I-21- 2.0
	21.2	.1	Project Concept Report	I-21- 2.0
	21.2	.2	Scope of Services	I-21- 2.0
21.3	}	Con	nsultant Acquisition	I-21- 3.0
21.4	•	Tec	chnical Management	I-21- 3.0
21.9	,	Cor	ntract Administration	I-21- 4.0
21.6	;	Sch	neduling and Coordination	I-21- 4.0
21.7	•	Con	nsultant Use of Department	
			Computer Programs	T-21- 5.0

Chapter 22		Architectural Plans			
22.	22.1 Ger		eral	I-22- 1.0	
22.	22.2 Code		es, Rules and Regulations	I-22- 3.0	
22.	3	Star	ndard Document Requirements	1-22- 8.0	
22.	4	Sche	ematic Design Phase Requirements	I-22-10.0	
22.	5	Desi	ign Development Phase	I-22-13.0	
	22.5	.1	General	I-22-13.0	
	22.5	.2	Development Procedures	I-22-13.0	
	22.5	.3	Reviews	I-22-13.0	
	22.5	. 4	Submission Requirements	I-22-14.0	
22.	6	Cont	tract Document Phase	I-22-35.0	
	22.6	.1	General	I-22-35.0	
	22.6	.2	Development Procedures	I-22-35.0	
	22.6	.3	Review and Approvals	I-22-35.0	
	22.6	.4	Documents Required	I-22-37.0	
	22.6	.5	Release for Printing	I-22-40.0	
	22.6	.6	Pre-Bid Conference	I-22-40.0	
22.	.7	Con	struction Phase Requirements	I-22-41.0	
	22.7	.1	Notice to Proceed	I-22-41.0	
	22.7	.2	Preconstruction Conference	I-22-44.0	
	22.7	.3	Periodic Construction Conference	I-22-41.0	
	22.7	.4	Shop Drawings, Materials and		
			Equipment Submittals	I-22-43.0	
	22.7	7.5	Clarification Drawings &		
			Specifications	I-22-43.0	
	22.7	7.6	Substantial Completion	I-22-45.0	
	22.7	7.7	Final Acceptance &		
			Contract Close-out	I-22-46.0	
~4	22	Door	sign Exceptions		
Chapter 23			eral	I-23- 1.0	
			partment Design Standards	2 2 2 2 2 2	
23	• 4	_	l Criteria	I-23- 1.0	
23	3		sign Exceptions	I-23- 2.0	
23			sign Concurrence	I-23- 2.0	
23			minology	I-23- 3.0	
2.3	• •		····		

LIST OF EXHIBITS

EXHIBIT	NUMBER	PAGE
Geometrics		
Interstate and Expressway Shoulder Widths	I-2-A	I-2-11.0
Added Paved Shoulders	I-2-B	I-2-13.0
Roadside Safety		
Design Criteria Related to Highway Safety	I-4-A	I-4-3.0
Design Criteria Related to Highway Safety	I-4-B	I-4-4.0
Canals: Design Speed 50 MPH or greater	I-4-C	I-4-6.0
Canals: Design Speed 50 MPH or greater	I-4-D	I-4-7.0
Work Zone Traffic Control		
Lane Closures	I-10-A	I-10-37.0
Pay Items, Quantites and Specifications		
Pay Item Request Form	I-11-A	I-11-5.0
Project Development		
Plans Process Flow Chart Design Project Activities	I-13-A	I-13-4.0
Flow Chart	I-13-B	I-13-11.0
Final Plans Transmittal	I-13-C	I-13-12.0
Data Collection Transmittal Letter to: Highway Statistic		
Engineers	I-14-A	I-14-11.0
Soils Engineer	I-14-B	I-14-12.0
Request for Survey	I-14-C	I-14-13.0
Transmittal Letter to: Materials Engineer	I-14-D	I-14-15.0
Transmittal Letter to: Environmental Permit Coord.	I-14-E	I-14-16.0
Transmittal Letter to: District Railroad Coord.	I-14-F	I-14-17.0
Transmittal Letter to: Utilities Engineer	I-14-G	T_1/-10 0
Computation Book Transmittal	I-14-G I-14-H	I-14-18.0 I-14-19.0
Request for CADD and Cross-		1 11 15.0
Sections	I-14-I	I-14-20.0

Scheduled Submittals		
Typical Section Data Sheet		
(Consultant)	I-15-A	I-15-9.0
Typical Section Data Sheet		
(In-House)	I-15-B	I-15-10.0
Proposed Typical Section		
(Consultant)	I-15-C	I-15-11.0
Proposted Typical Section		
(In-House)	I-15-D	I-15-12.0
Quality Assurance		
Plans Processing Procedure	I-17-A	I-17-5.0
Plans Revisions		
Revision Letter	I-20-A	I-20-4.0
Plans Key Sheet	I-20-B	I-20-6.0
Sheets Revision Letter	I-20-C	I-20-10.0
Architectural Plans		
Title Block Requirements	I-22-A	I-22-47.0
Title Block	I-22-B	I-22-48.0
Title Sheets, Pages and		
Covers	I-22-C	I-22-50.0
Key Sheet	I-22-D	I-22-51.0
Standard Method of Area		
Calculation and Space		
Measurment	I-22-E	I-22-53.0
Area Analysis - Summary	I-22-F	I-22-58.0
Facilities Computation Form	I-22-G	I-22-59.0

VOLUME I

ROADWAY DESIGN CRITERIA AND PROCESS

INTRODUCTION

The <u>Roadway Design Criteria and Process Manual</u> is part of a two volume set of guidelines, standards and techniques used to develop roadway plans for the Florida Department of Transportation.

This volume contains the Florida DOT's roadway design criteria, which, along with other Florida DOT publications, establish highway design policy. This manual includes both design criteria and material describing the design process.

Volume II of this manual, <u>Plans Preparation and Assembly</u>, was prepared to aid the technician and drafter in the development of a set of roadway plans.

Revisions, additions, deletions and interpretations to this manual will be issued quarterly following discussions at the District Design Engineers' meeting, or on an as-needed basis. Changes will be issued with instructions for insertion into this manual.

DESIGN CRITERIA AND REFERENCES

There are many excellent publications on design criteria available to the roadway designer. The following list includes criteria manuals or publications which are required or recommended by the Department for use in the development of highway plans.

- 1.1 Florida Department of Transportation Published References
 - o <u>Manual of Uniform Minimum Standards for Design</u>, <u>Construction</u>, and <u>Maintenance for Streets and Highways</u> (Green Book). Adopted by Rule Number 14-15:02.
 - o <u>Florida's Geometric Design Guide for Resurfacing</u>,

 <u>Restoration and Rehabilitation of Streets and Highways</u>

 (3R Manual Orange Book) Doc. No. 625-010-004-a
 - o <u>Roadway and Traffic Design Standards</u>
 (Roadway Design Standards) Doc. No. 625-010-003
 - o <u>Policy and Guidelines for Vehicular Connections to</u>
 Roads on the State Highway System

 (Driveway Manual) Doc. No. 0850-010-001-a
 - o <u>Standard Specifications for Road and Bridge</u> Construction and Supplements
 - o Bicycle Facilities Planning and Design Manual
 - o <u>CADD Roadway Standards and Guidelines</u>
 Doc. No. 625-010-007
 - o <u>Drainage Manual</u> (four volumes)
 Doc. No. 625-040-XXX
 - o <u>Handbook for Drainage Connection Permit</u>
 Doc. No. 625-040-400 (Rule 14-86)
 - o <u>Soils and Foundations Manual</u>
 Doc. No. 675-020-012-a
 - o <u>Project Development and Environmental Studies Manual</u>
 Doc. No. 625-010-002-a

o <u>Flexible Pavement Design Manual for New Construction</u> and for Rehabilitation

Doc. No. 625-010-002

o <u>Rigid Pavement Rehabilitation Manual</u>

Doc. No. 625-010-005

o <u>Rigid Pavement Design Manual</u>

Doc. No. 625-010-006

o R/W Survey and Mapping Manual

Doc. No. 575-010-XXX

o <u>Location Survey Manual</u>

Doc. No. 550-030-XXX

- o Utilities Rule # 14-46.001 Policy 710-010-XXX Procedure
- o <u>Utility Accommodation Guide</u>

710-020-0019 Procedure

- o Value Engineering Design Review Procedure 625-030-0029
- o <u>Basis of Estimates Manual</u>

Doc. No. 001-600-210-a

Contract Estimating System

Document No. 001-600-200-a

- o <u>Project Management Guidelines</u>
- o <u>Landscaping Guidelines</u>

Doc. No. 650-050-0016

o Railroad Procedures Manual

Doc. No.725-080-XXX

o <u>Highway-Railroad Grade Crossing</u>

Material Selection Guide

Doc. No. 625-030-010-b

o <u>Structures Design Guidelines</u>

Doc. No. 625-020-XXX

o <u>Sample Computation Manual</u>

Doc. No. 001-600-215-a

o <u>Design Training Aids</u>

Design Checklist

Proc. No. 001-600-400-a

- o <u>Guidelines for Contract Duration</u> Proc. No. 700-010-044-a
- o <u>Design Traffic Forcasting Procedure</u>
 Proc. No. 525-030-120-a
- o <u>Transportation Engineering Quality Assurance Plan 1989</u>
- 1.2 American Association of State Highway and Transportation Officials (AASHTO) Published References:
 - o <u>A Policy on Geometric Design of Highways and Streets</u>
 (Policy on Geometrics Design)
 - o <u>Geometric Design Guide for Resurfacing, Restoration,</u> and Rehabilitation (R-R-R) of Highways and Streets (Purple Book).
 - o <u>A Guide on Safety Rest Areas for the National System of</u>
 <u>Interstate and Defense Highways</u>
 - o <u>A Policy on the Accommodation of Utilities on Freeway</u>
 Right-of-Way
 - o A Guide for Bicycle Routes
 - o <u>A Guide for Accommodating Utilities on Highway</u>
 Rights-of-Way
 - o <u>A Policy on Access Between Adjacent Railroads and</u>
 <u>Interstate Highways</u>
 - o <u>Roadside Design Guide</u>
 - o <u>Standard Specifications for Structural Suports for</u>
 Highway Signs, <u>Luminaires and Traffic Signals</u>
 - o <u>An Information Guide for Roadway Lighting</u>
 - o A Guide on Erecting Mailboxes on Highways
 - o <u>Guidelines for Value Engineering</u>
 - o A Guide for Highway Landscape and Environmental Design
 - o Highway Drainage Guidelines

1.3. Federal Highway Administration Published References:

- o <u>Manual on Uniform Traffic Control Devices</u> (MUTCD)
- o <u>Standard Highway Signs</u>
- o <u>Handbook of Highway Safety Design and Operational</u>
 Practices
- o <u>Accommodation of Utility Plant Within the Right-of-Way</u> of Urban Streets and Highways - FHWA-RD-75-8 and 9.

1.4 Transportation Research Board Publications:

- o <u>Highway Capacity Manual</u>
- o <u>Policies for Accommodation of Utilities on Highway</u>
 Rights-of-Way.

Chapter 1

DESIGN PARAMETERS

1.1 General

Designs for highway and street projects are normally based on established design standards for the various elements of the project, such as width of roadway, side slopes, horizontal and vertical alignment, drainage considerations and intersecting roads. Selection of the appropriate standards is influenced by traffic volume and composition, desired levels of service, terrain features, roadside developments, environmental considerations and other individual characteristics.

The identification of applicable design controls is basic to providing the desired level of service, optimum safety, and cost effectiveness.

1.2 Traffic

The existing and design year traffic volumes, the traffic composition, directional distribution and design speed are all essential elements to the calculation of highway capacity, levels of service and pavement structural designs. This traffic information must be developed and approved during the programming and Project Development and Environmental (PD&E) studies stages of each project. PD&E studies may not be performed on all projects. When that is the case the necessary traffic studies must be part of the design process.

Preliminary capacity analyses and levels of service calculations will produce the numbers and configurations of traffic lanes required for the project.

Designs shall be based upon the projected traffic volume for the expected service life of the project. The pavement design manuals should be referenced for specific information on design periods. Design Hourly Volume (DHV) shall be the 30th highest hour.

1.3 Capacity and Level of Service

The AASHTO A Policy on Geometric Design of Highways and Streets and the Transportation Research Board Highway Capacity Manual provide the detailed analysis and calculation guides necessary for the number and configurations of lanes required and the resulting levels of service provided. As illustrated in those texts, gradients, roadside developments, number, spacing and types of crossings and intersections, traffic volumes, and signalization patterns all greatly influence capacity and levels of services. Those factors, in addition to the roadway functional classification, have a direct influence on the design speed to be adopted at the preliminary design level.

When the design speed and roadway functional classification (and the resulting geometric controls) have been established, the capacity and level of service analyses will have to be checked and adjusted against the more advanced determination of those variable factors outlined above.

Level of Service (LOS) "C" should be considered the minimum design LOS, except for urban projects with extreme right-of-way costs.

1.4 Roadway Functional Classification

The AASHTO Policy on Geometric Design presents an excellent discussion on highway functional classifications. Florida Statutes, Title XXVI, Chapters 334, 335 and 336 give similar definitions, and establish classifications for road design in the State of Florida.

Those classifications serve as the basis for the "Highway Function and Classification" portion of the section on Planning in the <u>Manual of Uniform Minimum Standards for Design</u>, <u>Construction</u>, and <u>Maintenance of Streets and Highways</u>, known as the Florida Green Book, issued by the Florida Department of Transportation in 1986. Much of the design criteria and controls adopted by the Florida Department of Transportation are categorized by those classifications and the design speeds adopted for each of those classifications.

1.5 3R Design

Florida's <u>Design Standards for Resurfacing, Restoration, and Rehabilitation (R-R-R) of Streets and Highways</u>, defines design controls for 3R projects, which are generally less restrictive than the criteria for new construction. These standards do not apply to Interstate or freeway facilities.

1.6 <u>Design Consistency and Driver Expectancy</u>

Design consistency is achieved when the geometric features of the roadway are consistent with the operational characteristics expected by the driver. Inconsistencies normally relate to:

- o changes in design speed.
- o changes in cross section.
- o incompatibility in geometry and operational requirements.

Changes in design speed may occur on a given stretch of roadway because portions of the highway were built as separate projects over an extended period of time. Inconsistencies may be due to a number of factors: changes in standards or DOT policy, re-classification of the facility, and lack of necessary funding.

There are two major types of design inconsistencies relative to cross section. These are point inconsistencies and a general incompatibility between cross section and alignment. A point inconsistency may be, for example, the narrowing of lane widths, a narrow bridge, a lane drop, or a change from multi-lane section to two lanes.

A cross sectional inconsistency is usually the result of upgrading a highway cross section without upgrading the alignment. Sometimes pavements are widened and shoulders added on an older two lane highway. The wider cross section on an old alignment might convey a conflicting message to the driver and lead to an inappropriate expectancy based on the visual aspects of the cross section, because cross section features can be more apparent than the alignment.

Of course, this is not to say that widening creates unsafe conditions. Widening alone can measurably improve the safety characteristics of a road, particularly on very narrow, low-volume roads. Designers should, however, be aware of potential inconsistencies that frequently can be overcome with relatively low cost treatments. In the case of widened roads on old alignments, pavement markings, warning signs, and delineation devices can be very helpful to the driver.

Inconsistencies may also relate to incompatibility in geometric and operational requirements. Occasionally elements of the design appear to have been selected for the purpose of fitting together the geometric components conveniently and economically rather than for the purpose of satisfying operational requirements. An example of an inconsistency resulting from the incompatibility is a direct entry ramp which is intended to permit vehicles to enter the stream of traffic without coming to a complete stop but which, in reality, forces the vehicle to stop when a gap in the traffic stream is not immediately available.

Design inconsistencies can result in driver uncertainty, an increase in response time and an increase in the probability of inappropriate driver response.

Driver expectancy relates to the readiness of the driver to respond to events, situations, or the presentation of information. It can be defined as an inclination, based on previous experience, to respond in a set manner to a roadway or traffic situation. It should be stressed that the initial response is to the expected situation rather than the actual one. Expectancy can affect the perception and use of information. In most circumstances, the expected and actual conditions are the same. However, when design inconsistencies occur and a driver's expectancy is incorrect, it takes longer to respond properly, there may be no response, or the response may be inappropriate to actual conditions.

There are certain elements in the design of various components of the roadway which particularly affect design consistency, driver expectancy, and vehicular operation. These components include horizontal and vertical alignment, embankments and slopes, shoulders, crown and cross slope, superelevation, bridge widths, signing and delineation and guardrail.

1.7 Aesthetics

Highways are built first and foremost for functional purposes, but the designer should be sensitive to how the highway will be perceived by the users. Designing-in aesthetics is more than just providing for landscape plantings. The roadway should blend into the landscape, avoiding large cuts and fills and round side slopes into the existing terrain. Horizontal and vertical alignment should be coordinated so that a driver has an opportunity to gain a sense of the local environment. Combinations of horizontal and crest vertical curves, and broken-back curves should be avoided. Excessively long tangent sections become monotonous. Either curvature or other features should be added to maintain drivers interest.

Application of the clear zone concept discussed in Chapter 4 of this volume will result in a clean, uncluttered and pleasing roadside. Landscaping of the roadside should be considered early in the design process, so that plantings blend in with the geometric design. Chapter 9 of this volume discusses landscape design criteria. At times extra right-of-way may be obtained for treatments if the need is identified early. Retention/detention ponds and other wetlands can be attractive if well-designed and placed in a location where they can be viewed from the roadway.

Vistas of exceptional beauty should be accentuated by the roadway geometrics. Ideally, such vistas should be on the outside of horizontal curves, without excessive roadside appurtenances and signs to clutter the view.

"Streetscaping" techniques in urban areas include an emphasis on pedestrian accommodation, trees and other plantings, access control, careful signing, and zoning restrictions on commercial signs. Parkways, and other roads specifically intended for pleasing aesthetics should be designed by a multi-disciplined team including landscape architects and planners.

Chapter 2

CEOMETRICS

2.1 Standards

The design controls discussed in the previous chapter serve as guides for identifying specific levels of design standards that are most appropriate for various highway functional classifications. This chapter discusses specific standards and how they are to be applied by designers.

Standards adopted by the Department, described herein, generally adhere to the AASHTO design policies. Some specific standards reflect judgments by the Department as to appropriate applications for conditions in Florida.

2.1.1 Fixed Standards.

Some standards are defined in terms of fixed values — standards for elements such as lane width, shoulder width and bridge width. Widths less than standard would not provide adequate levels of service and safety. Widths greater than standard usually would be a considerable economical investment.

Fixed standards apply principally to projects of new construction. These standards are also desirable for rehabilitation and resurfacing projects, but it sometimes becomes necessary to accept a less desirable standard because of economic, social or environmental considerations.

2.1.2 Design Standard Ranges

Many standards will be expressed in terms of a range of Examples are criteria for elements such as acceptable values. sight distance, degree of curvature, median width, vertical and lateral clearances, and slopes. Many of these elements are closely related to traffic safety. The designer should always attempt to satisfy the mid to upper portion of the range. lower portion of ranges should be used only when dictated by serious constraints imposed by economics, environmental considerations or similar considerations. While minimum values may be acceptable, the accumlative affect of establishing all critical design parameters at the minimum, may result in a project of marginal safety for motorists, pedestrians and Minimum values may also limit future rehabilitation bicyclists. options.

Many design standard considerations are related directly to the design speed, including vertical and horizontal geometry and required sight distances. Always provide at least the minimum values for these standards, regardless of traffic volumes, functional classification or any other considerations. The minimum design elements are very closely related to traffic safety and cannot be compromised without an approved design exception.

The design speeds adopted for the various functional classifications are given in Table III-1 of the Florida Green Book. It should be noted that some design speeds in that table are in excess of nationally imposed speed limits. An expected low speed limit by itself does not provide a basis for using a low design speed. Maximum degrees of curvature and maximum rates of superelevation are given for those design speeds, for both rural and urban applications, in Table III-3 of the Florida Green Book. Roadway Design Standard Indexes 510 and 511 should be referred to for application of superelevation.

Stopping sight distances and the controls they impose on vertical curvature, developed in AASHTO's Policy on Geometric Design, are repeated in Table III-6 of Florida's Green Book. Established passing sight distances for the various design speeds are also given in that table, along with minimum vertical curve lengths adopted in consideration of highway aesthetics.

2.2 Lane Widths

2.2.1 Through Lanes

Both the AASHTO Policy on Geometric Design and Florida's Green Book tabulate the minimum travelled way widths for two-lane rural local, collector and arterial highways, supplemented by discussions that are intended to provide guidance, but not establish a fixed standard for all conditions. Iane widths for all classifications of urban highways are covered by discussions of acceptable widths for various traffic conditions, right-of-way widths and existing development controls.

In general, lane widths should be as wide as possible (up to 12 feet) whenever it is reasonable and prudent. The following shall be minimum criteria:

<u>Freeways</u> - 12 foot lane widths shall be used. AASHTO Policy on Geometric Design give further guidance.

<u>Arterials (Major and Minor)</u> - 12 foot lane widths shall be used. 11 foot lanes may be used on two lane rural facilities under restrictive or special conditions. In these cases, the file must be documented with adequate justification.

- 11 foot lane widths can be used for urban curb and gutter sections under at least one of the following conditions:
- a. Right-of-way and existing development are stringent controls.
- b. Facility operates under interrupted flow conditions.
- c. Low Design Speed (40 MPH or less).
- Intersection Design controls are not adversely affected.
- e. Design hour truck volume is 10% or less.

The AASHTO Policy on Geometric Design gives further guidance.

<u>Collectors (Major and Minor)</u> - 11 foot lane widths should be used. 12 foot lanes may be provided on rural two lane highways under the following conditions:

- o Average Daily Traffic (ADT) above 1,600.
- o ADT 750 to 1,600 and design speed in excess of 45 MPH.
- o ADT 400 to 750 and design speed in excess of 60 MPH.

12 foot lanes shall be provided in urban industrial areas when right-of-way is available.

The AASHTO Policy on Geometric Design gives further guidance on selection of lane widths.

Local Roads - 10 foot lane widths should be used on rural roads. Greater widths on rural roads may be necessary as required by Table III-8, Florida Green Book. Urban local roads should be 11 foot lane widths. 12 foot lanes should be used in industrial areas when right-of-way is available.

2.2.2 Transitions of Pavement Widths

When new pavement widths are not substantially greater than the joining pavement, grade differentials are slight and future widening is expected, striped transitions may be considered. An alternative approach is an abrupt change in width, with appropriate pavement markings, reflectors and rumble strips.

2.2.3 Wide Curb Lanes

In addition to the safety benefits for bicyclists, wide curb lanes provide benefits that will improve traffic flow, add to the capacity of the roadway and enhance overall highway safety. Some of those benefits are:

- Assist a vehicle in turning right into driveways and narrow connecting streets without encroachment into the adjacent lane.
- o Assist a vehicle in entering the roadway from an intersecting roadway or driveway without encroachment into the adjacent lane.
- Allow a motorist to pass a bicyclist without delay.
- Reduce the need for vehicles to change lanes because of a bicyclist.
- o Improve drainage in constricted areas.

Wide curb lanes are to be provided as the minimum treatment in conjunction with other roadway improvements (curb and gutter construction) in or within one mile of all urbanized (population 50,000 or more) areas unless right of way is inadequate and the cost associated with acquisition for this purpose is not feasible. For those projects that require additional right-of-way for the construction of the road, the additional

width to provide wide curb lanes will be acquired unless the additional cost is extreme. With severe right of way limitations 11 foot continuous two-way turn lanes or painted medians may be used under interrupted flow operating conditions at low speeds, up through 40 MPH. The presence of heavy truck traffic (design hour trucks greater than 10%) and intersection design controls should be evaluated in reducing the center-most lane to 11 feet. The minimum width for curb lanes is 14 feet, measured from the edge of the adjacent travel lane to the lip of the gutter, or 15 feet to the face of the curb, if the 1 foot 6 inch gutter is not constructed.

Heavily congested roadways with significant levels of commerce and numerous intersections are served best with wide curb lanes that are not marked as a bike lane or bike route. In no case should an edge line be marked 2 or 3 feet in on a 14 foot wide curb lane, since this tends to channel bicyclists into a space that is too narrow.

Wide curb lanes are also to be considered in urban areas (5,000 - 50,000 population) based on anticipated bicycle travel needs as previously identified.

Urban resurfacing projects may include restriping to provide wide curb lanes by using 11 foot interior lanes. This policy may be applied on all appropriate urban and urbanized area (curb and gutter) resurfacing projects.

2.2.4 Other Lane Widths

Collector-distributor lanes and auxiliary lanes for speed change, turning, storage for turning, weaving and other purposes supplementary to through-traffic movement should be of the same width as the through lanes. Left-turn storage lanes may be 10 feet wide where speeds are low and the intersection is controlled by traffic signals. Two-way turn lanes should have a desirable minimum of 12 feet wide and an absolute minimum of 10 feet and should not exceed 15 feet.

Ramp widths are discussed in detail in the AASHTO Policy on Geometric Design.

2.2.5 Bicycle Lanes

Bicycle lanes (4 foot minimum width) may be warranted in lieu of wide curb lanes in some areas of the State. Collectors and the more lightly traveled arterials that have only a moderate level of commerce, and have fewer turning movements, may serve bicyclists with a bike lane.

Roadway sections with low to moderate traffic and where it is desirable to attract bicyclists should be considered for 4 foot wide bike lanes in the initial roadway improvement. A 4 foot minimum width with urban curb and gutter construction or 5 foot minimum width with rural type (no curb) construction will be required.

Chapter 8 of this volume discusses bicycle facility design criteria in detail.

2.2.6 Shoulder Widths

Shoulder width criteria are provided in the AASHTO Policy on Geometric Design and the Florida Green Book. In addition to the suggested and minimum criteria presented in those volumes, it is the Department's policy that 4 foot paved shoulders are required on all new construction, reconstruction and lane addition projects for rural, open drainage, free access highways of the following types (does not apply to urban curb and gutter construction):

- a. On all two-lane principal arterial highways.
- b. On all other two-lane highways with a post construction ADT of 5,000 or greater.
- c. On the outside shoulder of all rural multilane facilities. The inside shoulder may be paved 2 feet in those areas of the State where establishing and maintaining a turf shoulder has been difficult. The inside shoulder will be paved (4 foot minimum) on the low side of pavement through superelevated curves, and extended approximately 300 feet beyond the P.C. and P.T. of the curve, irrespective of other conditions. Roadway Design Standard Index 104 contains additional criteria for paved shoulders.
- d. On all coastal routes where material is of poor quality for growing grass. This will generally be within 1/2 mile of the coast but may extend beyond this on some projects. A project-by-project evaluation based on discussions with construction and maintenance personnel will be necessary.
- e. On all projects in or within one mile of urbanized areas with a post construction ADT greater than 1600.

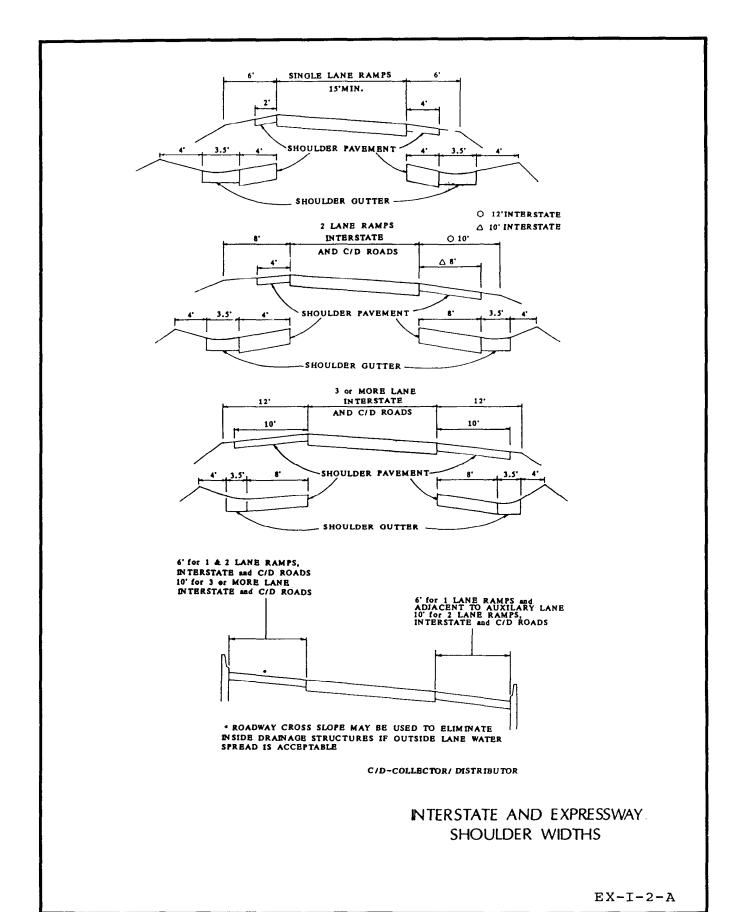
- f. On other projects in urban and rural areas, based on safety and operational needs (including anticipated bicycle travel).
- g. On projects within 5 miles of urbanized areas where adjacent development is such that significant pedestrian travel would be expected and post construction ADT will exceed:

Two lanes - 5,000 Four lanes - 20,000

h. Two to four foot paved shoulders can be used in other areas where pavement dropoffs or problems with turf shoulders exists. Maintenance and construction personnel should be consulted for applicable locations

Roadway Design Standard, Index 104 (Erosion Control Details for Permanent Construction) should be applied on all projects. Index 105 should be applied at the edge of shoulder pavement on existing facilities.

Specific widths have also been adopted for interstate, expressway, single and double lane ramps and collector-distributor road shoulders. Total shoulder widths, paved shoulder widths, widths of paved shoulder separations between through pavement edge and the near edge of any shoulder gutter are given for both right (outside) and left (inside) edges of the roadway. These widths and clearances are illustrated in Exhibit I-2-A.

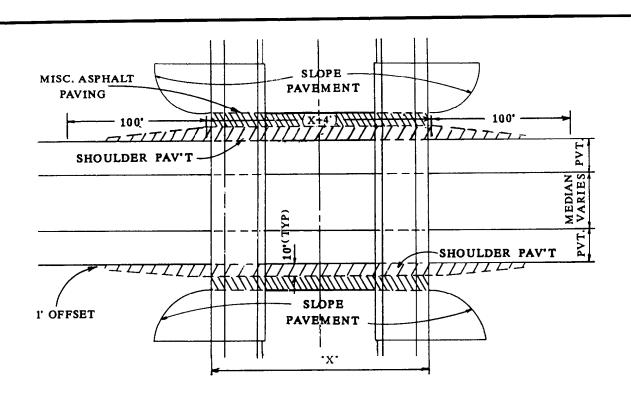


It is desirable to pave the outside shoulder and adjacent area under overpass bridges on all projects that do not normally include paved shoulders. This shoulder pavement will provide additional safety, enhance drainage, reduce maintenance and improve appearance. When paved shoulders are not normally provided, indicate paving for the outside shoulders in accordance with Exhibit Ex-I-2-B.

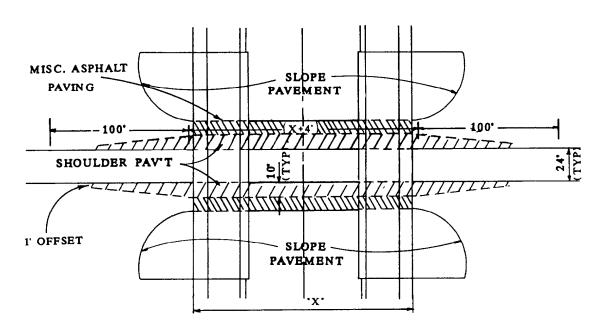
2.2.7 Limits of Friction Course on Shoulders

Friction courses on limited access facilities shall be extended one foot onto both the median and outside shoulders. FC-1 or FC-4, if used on ramps, may be extended one foot onto both the inside and outside shoulders.

Friction courses (FC-1, FC-2 or FC-4) should be extended the full width of the shoulder on free access highways because of bicyclist usage. Terminating the friction course at the edge of travel lane or within the paved shoulder is considered to be a safety problem for bicyclists since they must cross over the drop-off. Bicyclists in rural areas use the mainline pavement and cross to the shoulder when a car approaches from behind. Most paved shoulders on free access facilities are 4 foot in width.



DIVIDED HIGHWAY OVER DIVIDED HIGHWAY



DIVIDED HIGHWAY OVER UNDIVIDED HIGHWAY

ADDED PAVED SHOULDERS

EX-1-2-B

2.3 Other Cross Section Elements

2.3.1 Cross Slopes

Standard pavement cross-slopes and shoulder cross slopes are defined for Florida roads in Section III of the Florida Green Book. Chapter 4 of this volume discusses the safety requirements of slopes and other cross section elements in detail.

2.3.2 Roadway Structure

The type of pavement usually is determined by analysis of the volume and composition of traffic, the soil conditions, the availability of materials, the initial cost and the estimated cost of maintenance.

Criteria and procedures for selecting the type of pavement and the structural design of the various surfacing courses are discussed in the pavement design manuals.

2.3.3 Geometry of Side Slope and Side Ditches

Volume II of this manual discusses the layout of side slopes and ditches in detail. Variations imposed by parallel ditches or canals, barriers and clear zone requirements are addressed in Chapter 4, Roadside Safety, in this volume.

2.3.4 Border Width

Border width requirements for urban collector and arterial highways for new construction or major modifications to existing facilities (with curb or curb and gutter), require a separate set of design controls, as discussed below.

A minimum border width of 12 foot, measured from the edge of pavement to the right-of-way line, should be used for those curb and gutter projects in areas where right-of-way is available or can be obtained. Desirable lane and border widths, as opposed to minimum, should be used when right-of-way can be obtained with minimal additional cost (from business damages or taking of buildings or parking areas).

Minimum border widths of 8 foot measured from the edge of pavement to the right-of-way line may be used when the gutter is of contrasting color, is not designed for vehicular use, and the following restricted conditions exist:

- (1) The project can be constructed within existing right-of-way with minimum lane width requirements satisfied.
- (2) Existing right-of-way is not adequate for construction; an alternate location is not feasible; and the cost for obtaining right-of-way with more generous dimensions is excessive in relation to the added service and safety that would result.
- (3) The cost of additional right-of-way at isolated locations is excessive.

The standard sidewalk width is 5 foot. When the sidewalk is adjacent to a curb, the width of the sidewalk shall be 6 foot, minimum.

Exceptions to the 8 foot minimum border width requirement may be made in those special cases where sidewalks are not required, such as those facilities where the roadway parallels a railroad or drainage canal and pedestrians would not be expected. As an absolute minimum, the border width must be adequate to provide space and clearances for signs, signal supports, drainage structures or guardrail, if required.

2.4 Horizontal Alignment

2.4.1 General

Design speed is the principal factor controlling horizontal alignment. Several geometric standards related to design speed are very specific. Other criteria cannot be defined as specifically and require that judgments be made by designers in consideration of local conditions. Guidelines for some of these decisions are outlined in Section III of the Florida Green Book. Limiting values on degrees of curvature are also given in the Green Book, Section III.

2.4.2 Supplemental Alignment Control

Further guidelines have been established by the Department for lengths of horizontal curves, maximum deflections without curves, redirection of through lanes at intersections and minimum transition lengths between reverse curves. The criteria given below are intended for use in establishing minimum lengths for both rural and urban conditions.

Wherever the criteria differ for different types of facilities, the term "interstate" shall mean "interstate and other high speed controlled access facilities", the term "rural" shall mean "rural arterials and collectors and urban arterials and collectors with open drainage", and the term "urban" shall mean "urban arterials and collectors (curb and gutters with design speed 45 MPH or less)."

Table 2.1 Length of Horizontal Curves

Desirable length = 30(S) (INTERSTATE)

15(S) (RURAL & URBAN)

Minimum length = 15(S) (INTERSTATE)

500 Foot (RURAL) 400 Foot (URBAN)

Where S = Design Speed

The desirable length should be provided to the maximum extent practical on new construction projects. The minimum length may be necessary for major modifications of existing facilities and environmentally sensitive areas. Values less than the minimum stated for interstate may be necessary in developed or environmentally sensitive areas.

For small deflection angles (5° or less) curves should be suitably lengthened to avoid the distracting appearance of a kink. Curves should be at least 500 feet long for a central angle of 5° and the minimum increased 100 feet for each 1° decrease in the central angle (900 feet for a 1° central angle.) This treatment may not be practical in developed or environmentally sensitive areas or for major modifications of existing facilities.

For design, the aesthetic control given above should be considered where practical, but may be compromised where other considerations warrant such action. Discernment of alignment changes in an urban setting is normally minimal due to the masking effects of development, traffic signs, various items of interest and similar distracting stimuli.

Curves should be of such length to provide at least a minimum of 200 feet of full superelevation within the curve when superelevation is required.

The length of curves for turning roadways may be less, but sufficient length for superelevation should be provided. The lengths shown in Table III-18 of the AASHTO Policy on Geometric Design, should be used for compound curves on turning roadways.

Curves are required on all interstate and rural roads when a deflection of over 0°45' occurs in alignment on the open highway.

Curves are required on arterials and collectors when deflections on the open highway exceed the following:

Design Speed equal to or less than 40 MPH - 2°00' Design Speed equal to 45 MPH - 1°00'

For redirection of through lanes at intersections the formula L=WS should be used for speeds of 45 MPH or greater and L=WS²/60 for speeds of 40 MPH or less; where "L" is the length of taper in feet, "W" is the width of lateral shift in feet and "S" is design or posted speed in MPH. Curves are not required if these formulas are used.

Short curves may be desirable at the above locations in lieu of angular breaks. If the maximum deflection without curvature as given above is not exceeded, the minimum length of curve given in the AASHTO Policy on Geometric Design does not apply for design and superelevation is not required.

The general controls for horizontal alignment shown in the AASHTO Policy on Geometric Design and Section III of the Florida Green Book, should be used to the extent practical.

A related area is the desirable minimum transition length between reverse curves. This desirable minimum tangent length should be determined as follows:

- 80% of the transition for each curve should be located on the tangent.
- The desirable minimum tangent length is the sum of the two 80% distances.
- Where alignment constraints dictate a less than desirable minimum tangent length, AASHTO allows reduction of the 80/20 superelevation transition treatment, (i.e., 20% to 50% of the transition may be placed on the curve).

Sound engineering judgement for establishing the minimum horizontal curve lengths and transition lengths between reverse curves should always be used. Before utilizing minimums less than noted above, full documentation and justification should be placed into the project file and approved in advance by the FHWA for federally funded projects. Transition lengths should be sufficiently long to take water off the pavement without allowing it to flow back across the pavement.

2.4.3 Two Lane to Four Lane Transitions

Transitions have been developed to eliminate reverse curvature alignment, to provide suitable length for pavement marking transitions, and to control development at the end of limited access right-of-way at interchange locations.

2.5 Vertical Alignment

The profile grade line defines the vertical alignment for road and street construction. As with other design elements, the characteristics of vertical alignment are influenced greatly by basic controls related to design speed, traffic volumes, functional classification, drainage and terrain conditions. Within these basic controls, several general criteria must be considered. These considerations, along with some specific limiting values, are presented in Section III of the Florida Green Book.

The Department's minimum for structure clearance over all highways provides for 16 feet. Exceptions to this policy shall be permitted only when justified by extenuating circumstances. This clearance should be increased on new facilities to allow for resurfacing. Increased clearance from 16 foot-3 inches to 16 foot-6 inches is recommended. The structures manual should be referenced for details on clearances for all types of structures.

Clearance required above design high water for roadway base courses, as well as the limiting relationships between shoulder/pavement elevations vs. high water and flood elevations, are discussed and charted in Chapter 6, "Roadway Grades," Volume 2A of the Florida Drainage Manual.

Clearance over major underground utilities is also a vertical alignment control.

The Roadway Design Standards Index 205 & 280 lists minimum covers and maximum fill heights for all types of pipe culverts.

2.6 Superelevation

Superelevation and transition standards and the application of those standards are discussed and detailed in the Florida Green Book and the Roadway Design Standard Indexes 510 and 511. Further discussion, illustrations and charts are to be found in Chapters III and VI thru IX of the AASHTO Policy on Geometric Design. The AASHTO publication covers the full range of design values, therefore, the use of those design guides and aids is restricted by maximum and minimum rates established for use in Florida.

2.7 Cross Sections

The mechanical composition of cross-section sheets i described, and examples of completed cross-section sheets are given, in Volume II - Plans Preparation and Assembly.

The cross section is the most widely used tool today for the depiction and measurement of the various earthwork volumes that are part of all highway construction projects. Cross sections are used in the review process to check construction clearances and utility adjustment requirements, the appropriateness of roadway and ditch side slopes and adequacy of right-of-way. Their importance as a design tool is often overlooked.

The designer should use the development of the final cross sections as a step by step guide in assuring the proper fit of the various items to be included in the construction plans. As cross section elements are assembled, they provide clear evidence of the incompatibility of certain design features, thus allowing adjustments to be made, or exceptions to be verified, before other designs and plan developments have advanced to a point that revisions would be cumbersome and costly.

Cross section preparation and study is also an essential element in the preparation of staged construction plans, sequence of construction and traffic control plans, particularly where there are detour or bypass alignments to be considered.

2.8 Intersections and Interchanges

Design guides and criteria presented heretofore are also applicable to the proper design of intersections, including interchanges with their inherent ramps, speed change, merging and weaving lanes. Because high speed maneuvers and at-grade merging and crossing patterns impose additional responses and responsibilities upon the driver, it is incumbent upon the designer to ease those driver decisions and responses to the extent possible. Additional and extended criteria and guidelines are necessary for the designer to accomplish his tasks in a manner that will provide safety, reliability and uniformity for the driver.

Where curb is desirable for delineation and channelization, at islands on rural highways, the full approach shoulder width on the main roadway should be carried through the interchange or intersection. On urban sections this curb should not encroach on the refuge lane (if provided on the approach section) and the full section should be carried through the intersection or interchange.

The criteria and guidelines necessary are presented in Sections III-Geometric Design, Subsections 8 & 9 - "Access Control and Intersection Design" of the Florida Green Book. Chapters IX and X of the AASHTO Policy on Geometric Design and the TRB <u>Highway Capacity Manual</u> should also be referenced. They provide invaluable aids for the proper design of high speed weaving sections and at-grade intersection layouts. Their use should only be restricted by specific individual parameters adopted by the Department.

2.9 Limited Access Limits at Interchanges

The following criteria will be used in establishing limited access limits along crossroads at interchanges:

For rural interchanges, limited access will extend along the crossroad to a point 300 feet minimum beyond the end of the acceleration or deceleration taper. In the event these points are not opposite, the point most remote from the project will be the control and the limited access on both sides will end at that station along the crossroad. Where no taper is used, the limited access will be carried to a point 300 feet minimum beyond the radius point of the return. In this case also, the radius point most remote from the project will control.

For interchanges in urban areas, the criteria given above will apply except that the limited access will end a minimum of 100 feet beyond the end of taper or the radius point of the return.

In both cases of interchanges in rural and urban areas, a cross-over is to be centered 50 feet beyond the end of limited access except that a minimum distance of 600 feet to the ramp median opening will be required. In no case should access be permitted between the interchange proper and the cross-over as established by this criteria.

For partial cloverleaf, the limited access right-of-way along the cross road on that side having no ramp will extend to a point opposite that point controlled by the ramp.

Special cases or exceptions to the above will have to be handled on an individual basis and must be fully supported by realistic right-of-way cost information and other pertinent data covering any recommended alternative.

2.10 <u>Design of Rural Projects</u>

Highway designs should begin with the following steps:

- 1. Familiarization of the squad leader with pertinent project data.
- 2. Indexing field notes.
- 3. Checking field notes.
- 4. Drawing proposed typical section or sections.
- 5. Plotting alignment, existing topography, underground utilities and ground line profile in plan.
- 6. Outlining the drainage map.
- 7. Plotting alignment, topography and profile of outfalls.

All alignments should be reviewed for undesirable alignment and aesthetic effects such as broken curves, curves in opposite directions with insufficient intervening tangent, curvature exceeding the maximum for the design speed, etc., and a study made of possible remedial measures by realignment. A check should also be made of undesirable horizontal-vertical curve combinations.

2.10.1 Profiles

After determining that the horizontal alignment is satisfactory, a tentative grade can be established. Existing structures, including major utility facilities, are plotted in plan and profile. Several factors influence the choice of grades as listed below:

o A minimum stopping sight distance, based on the design speed, should be maintained throughout the entire project. The minimum length of vertical curves will be adhered to as established by the Florida Green Book. Values approaching, or at the upper limit of, the range should be used for design wherever conditions permit.

- o Established clearance between the bottom of base and design high water (DHW) must be maintained. Superelevation of curves must be taken into account so that the required clearance criteria is contained in Chapter 6, Volume 2 of the Drainage Manual.
- o Grades should not exceed the established ranges, maximum or minimum.
- o Soils data should be examined for the possibility of eliminating or reducing the undercutting of unsuitable material.
- o Crest vertical curvature that results in restricted passing sight distance for two lane roads should be avoided.
- o Sizes of existing drainage structures may give some indication of drainage requirements and allow establishment of grades that will provide the required cover.
- o When railroads are crossed at grade, the proposed grade must meet the rail elevations. The District Railroad Coordinator can determine if a railroad track adjustment is necessary or planned by the railroad company. The grades of intersecting streets or highways also should be matched as closely as possible.
- o Economy of construction is an important factor in establishing grades. Total earthwork should be held to a minimum. When practical, cuts should produce sufficient material for construction of fills.

2.10.2 <u>Cross Sections</u>

The roadway cross section ground lines can be plotted using the tentative grade for spacing the sections. The ground line, ground line elevation at the centerline of survey, and the station number are then plotted.

Prints of the typical sections, cross sections, plan and profile sheets will be sent to the District Geotechnical Engineer for use in obtaining soils data. Any subsequent changes in alignment, grade or typical section should be forwarded to the Geotechnical Engineer for reconsideration of recommendations. All projects with bad materials, whether they are to be removed or left in place, should have the 90% plans reviewed by the Geotechnical Engineer for final concurrence. The District Drainage Engineer needs similar information for grade review.

Outfall and infall ditch surveys can now be plotted. Alignment, topography and profile for each ditch are plotted. Cross sections are plotted after it has been determined at which locations outfall ditches will be constructed. Property lines should be shown to assure field consideration of right-of-way takings, including access. The preliminary plan and profile sheets will be used in the field inspection of drainage and later in establishing lateral ditch grades.

The roadway cross section templates are plotted now. Any special ditches should be planned at this stage. On superelevated curves in well drained soils, the same elevation for both ditches need not be maintained unless drainage considerations dictate otherwise. The standard depth ditch is 3.5 feet deep, measured from the shoulder point. Use of standard depth ditch design in superelevated sections must be carefully considered. The shoulder point varies with superelevation and could create steep

grades or flat spots. Consider the use of special ditches in these areas or note that the ditch is to be measured at an adjusted depth. Special ditch grades must be used when necessary to avoid water pockets. The use of too flat a ditch may create a trapped or poorly drained area and potentially have water reach the pavement under servere rainfall events.

Only the total thickness of the pavement structure needs to be plotted on the templates.

2.10.3 Preliminary Review

The grades can now be re-examined for possible improvements such as:

- o Elimination of poor grade alignment combinations.
- o Improved passing sight distance.
- o Avoidance of unnecessary handling of unsuitable material.
- Accommodation of special conditions revealed by examination of the plotted roadway cross sections.
- o Improved match with existing side streets, driveways, railroad grade crossing and other controls.

2.11 <u>Design of Urban Projects</u>

Many procedures discussed in Section 2.9 are also applicable to the design of urban projects. Since these procedures have been discussed, complete descriptions will not be repeated. Opportunities for blending roadways into an urban environment for aesthetic effect should not be overlooked.

2.11.1 Preparation

The first several steps in urban plans preparation are the same as for rural projects.

The location of outfalls for urban projects cannot always be determined until the storm sewer system is designed, so that operation may be deferred. Outfall and retention/detention locations may dictate the design.

In urban type construction, there is usually relatively little departure of grade from the existing ground, so variation in spacing of cross sections seldom is necessary. Existing pavement must be plotted. It is especially important to show existing pavement such as service drives, parking areas and similar construction at the right-of-way line. Driveways may need to be plotted in cross section when it is necessary to establish right-of-way limits.

Underground utilities are plotted using standard symbols as in rural plans preparation design. Front of building and other improvements near the right-of-way line are indicated at the correct distance from the centerline, and existing floor elevations are shown. Underground storage tanks should also be shown.

2.11.2 Geometrics

The geometric layout is plotted, including proposed sidewalk, curb and gutter returns at intersecting streets, median curbs, traffic separators and left turn storage and other auxiliary lanes. Proposed right-of-way lines are indicated. Care must be exercised to insure conformance of geometrics to the approved typical section and design criteria for the project.

Connections with intersecting streets are centered on the street dedications, except in rare cases when an existing street with curb and gutter is off center and proposed construction must meet existing construction. Offset, if any, to the centerline of pavement should be shown. A minimum width of 24 feet face to face of curbs is used for side street connections. Standard widths should be used for side streets except when connections must conform to existing curb and gutter sections. Right-of-way should accommodate future turn lanes on intersecting streets if the need for such lanes is anticipated.

2.11.3 Profiles

Profiles used for establishing back-of-sidewalk grades now are plotted.

Grade requirements for urban projects are quite different from those for rural projects. Ideally, the proposed grade for the back of the sidewalk should match the existing ground in profile. This condition rarely occurs except for short distances if back-of-sidewalk grades are established in accordance with design criteria, and compromise grades are selected which will minimize damage to adjacent property and reduce drainage complications.

To establish back-of-sidewalk grades, profiles along each proposed back-of-sidewalk location are plotted against the same elevation datum. These profiles usually are plotted from elevations obtained from roadway cross sections at the proposed back-of-sidewalk location. This method is satisfactory when sufficient cross sections or part sections are available to produce a reasonably accurate profile.

Cross sections will be used to establish direction of drainage arrows which are placed above and below the profile. Care must be taken to avoid plotting invalid or misleading elevations, which would be the case if the right-of-way line were to fall in an existing ditch. In this event, the ground line immediately beyond the ditch limits should be plotted.

Elevations and limits of floors, locations of building entrances, and limits of existing pavements along the right-of-way line are indicated. Limits of private drives and entrance walks need not be shown.

Tentative grades for back-of-sidewalk profiles now are established by plotting grade lines as close as possible to existing ground at the back-of-sidewalk location using the following criteria:

- o The desirable minimum tangent length of the grade line is 300 feet, with an absolute minimum of 250 feet unless otherwise specifically authorized.
- o A minimum longitudinal gradient is important for a curbed pavement, since it is susceptible to stormwater spread. Flat gradients on uncurbed pavements can lead to a spread problem if vegetation is allowed to build up along the pavement.

Desirable gutter grades should not be less than 0.3 percent for curbed pavements, and not less than 0.2 percent in very flat terrain. Minimum grades can be maintained in very flat terrain by use of a sawtooth profile.

To provide adequate drainage in sag vertical curves, a minimum slope of 0.3 percent should be maintained within 50 feet of the low point in the curve. This is accomplished where the length of the curve (L) divided by the algebraic difference in grades (A) is equal to or less than 167 (L/A <167). Special gutter profiles should be developed to maintain a minimum slope of 0.2 percent up to the inlet. Although ponding is not usually a problem at crest vertical curves, on extremely flat curves a similar minimum gradient should be provided to facilitate drainage.

o Vertical curves will be required for breaks in grade as shown in the Florida Green Book.

- o The high point should be at or near the centerlines of cross streets where possible.
- o The placing of low points in the grade at locations which would be detrimental to existing development, such as in intersections, should be avoided.
- o Standard clearance above high water elevations should be maintained. Methods of controlling high water, such as underdrains, should be included in the design when the grade cannot be set sufficiently above the high water elevation.

The grade must be such that the back-of-sidewalk will not be above building floor elevations at entrances, particularly in the case of buildings at or near the right-of-way line. The grade should be sufficiently lower than floor elevations to allow for provision of adequate drainage away from the entrance. If, at all practical, the grade of the sidewalk should be such that water will not be ponded behind it at locations where ground slopes toward the project. It is generally undesirable to have fill sections at the back of the proposed sidewalk. In case of a definite cross slope of the ground from one side of the project to the other, a compromise grade, compatible to the property development is established, with cut on one side and fill on the other.

Curves requiring superelevation should be avoided in urban areas. When necessary, separate profiles must be used for establishing grades for the right and left back-of-sidewalk.

Occasionally, a situation will arise where extensive development exists on both sides of the street and the ground of development on one side is somewhat higher than on the other. In this situation, a grade line fitting existing development on one side may cause extensive property damage on the opposite side.

In such cases, a reversed crown section may be used on one roadway. Separate profile grades would be required for each side. During transitions from reverse crown to normal sections and in superelevation transitions, care must be taken to avoid gutter grades that are less than the minimums. When preliminary back-of-sidewalk grades have been established, proposed sidewalks are plotted on prints of roadway cross section sheets. The prints are examined carefully for every possible improvement of grades and revised, if necessary.

2.11.4 Cross Sections

Preliminary cross sections are prepared in a similar manner as described in Section 2.9.2 for rural projects. However, special emphasis should be placed on matching existing streets, business drives, etc. and allowing for proper drainage.

2.11.5 Preliminary Review

The preliminary on-site review is now in order, the primary purpose of which is to determine feasibility of the geometrics and to make a careful review of proposed grades. A preliminary drainage inspection also may be made at this time, designating possible outfalls. However, the final drainage system design naturally will depend upon the final grades adopted.

The most important function of preliminary review is to carefully study the proposed grades, and to note any possible improvements. Of particular importance is the matching of grades at entrances of commercial buildings, parking areas, service station drives, etc. Matching of grade at side street intersections also is reviewed carefully. Conditions not evident in location field notes should receive careful attention, and notes concerning possible improvements should be made directly on preliminary prints.

Frequently, state highways slated for improvement are in fast growing areas. Notes on development, that has taken place subsequent to the location survey, should be made, and the District Location Surveyor notified so pertinent additional field information can be obtained. Close coordination with the District Permits Engineer is necessary.

The proposed geometrics at street intersections must be reviewed. This includes the location and appropriate width of side street connections and median openings, turning and storage lanes, and design of channelized intersections, railroad crossings, etc.

Chapter 3

EARTHWORK

3.1 Borrow Excavation

3.1.1 Borrow from within the Right-of-Way

Material from within the right-of-way may be used for projects which require borrow, including projects that are to be paid for under the embankment pay item. Each project must be analyzed separately to establish if obtaining borrow from within the right-of-way is feasible. For federal-aid projects concurrence of FHWA is required prior to utilization of material within the right-of-way.

Obtaining material from the project right-of-way shall not create an unsafe condition or an unprotected hazard. Criteria for canal protection shall be applied to areas which will fill with water.

Areas and amounts of vegetation removed shall be coordinated through the District Environmental Coordinator to ensure minimum disturbance and maximum retainage of the natural appearance.

3.1.2 Earthwork Paid for as Embankment

Earthwork should be paid for as "Embankment" in all cases where suk payment is appropriate. The following guidelines shall be applied to determine which projects are considered appropriate for this method of payment:

- (1) Filling is the predominate earthwork operation.
- (2) New construction and reconstruction projects should generally specify or require embankment.

The decision to use embankment or borrow should be made with recommendations from the responsible district construction office.

In general, no payment will be made for excavation when the embankment pay item is used. Exceptions will be made when there is existing subsoil and there is a possibility of large overruns, or there are large quantities of lateral ditch excavation. When excavation is paid for separately, no deduction will be made from the embankment (fill) quantity.

When earthwork is paid by embankment and there is no pay item for excavation, a standard note shall be placed under the earthwork box as noted in Chapter 7, Volume II, indicating there is no direct pay for excavation.

3.1.3 Measurement of Borrow Excavation

Borrow can be measured by two methods, (1) cross section measurement in the pit, or (2) truck measure.

Once the borrow quantity has been calculated, a percentage should be added for shrinkage (moisture loss and compaction during placement), and another percentage should be added for bulkage when using truck measure (bulking up of material when placed loose in truck). Recommendations on percentages should be obtained from the district soils and construction offices.

When a borrow excavation item is used, pay items for regular, lateral or channel excavation shall also be used as required.

To accommodate the various methods of measuring borrow, the following should be used in establishing appropriate pay item and footnotes:

(1) When the intent of the plans is to provide borrow by pit measure, the following pay item and footnote shall be used.

Pay Item: 120-2-1 Borrow Excavation (Pit Measure)

<u>Footnote:</u> To be furnished by the contractor from areas provided by him.

Example:	Fill	253 cy
	Shrinkage (+60%)	152 cy
	Total Fill	405 cy
	Roadway Excavation	<u>115 cy</u>
	Borrow Excavation	290 су

The quantity for item 120-2-1 would be 290 cy.

(2) When the intent of the plans is to provide borrow by truck measure, the following pay item and footnote shall be used:

Pay Item: 120-2-2 Borrow Excavation (Truck Measure)

<u>Footnote:</u> To be furnished by the contractor from areas provided by him.

Example:	Fill	253 c y
	Shrinkage (+60%)	152 cy
	Total Fill	405 cy
	Roadway Excavation	115 cy
	Borrow Excavation	290 су
	Bulkage (+25%)	_73 cy
	Total Borrow Excavation	363 cy

The quantity for item 120-2-2 would be 363 cy.

Truck measure should be specified only for projects which have small amounts of earthwork. Typical projects of this type are resurfacing, widening and resurfacing and safety projects.

(3) Projects for which payment is to be determined by pit measure but the need for flexibility to measure by truck volume is foreseen, the following pay item and footnote shall be used:

Pay Item: 120-2 Borrow Excavation

Footnote: To be furnished by the contractor from areas provided by him. Measurement shall be based on measurement of the borrow pit. At the contractor's option and approval of the engineer, measurement may be based on loose truck volume, in which case payment will be made on _____ % of the truck measured quantity.

Example:	Fill	253 cy
	Shrinkage (+60%)	<u>152 cy</u>
	Total Fill	405 cy
	Roadway Excavation	<u>115 cy</u>
	Borrow Excavation	290 cy

The quantity for item 120-2 would be 290 cy.

To calculate the percentage for the pay item footnote, the bulkage shall be estimated but not shown in plans. For the above example, the percentage is calculated as follows:

Borrow Excavation 290 cy
Bulkage (+25%) 73 cy
Total Borrow Excavation 363 cy

% = 290 cy/363 cy = 80%

3.2 Subsoil Excavation

Subsoil excavation consists of the excavation and disposal of muck, clay, rock or any other material that is unsuitable in its original position and that is excavated below the finished grading template. Subsoil excavation also includes all suitable material excavated within the above limits in order to excavate the unsuitable material.

The subsoil areas and volumes for unsuitable material shall be tabulated on the left side of the cross section sheet, and areas and volumes for suitable subsoil excavation shall be tabulated on the right (see Volume II). The fill volumes on the right shall also include areas and volumes to fill the excavated limits created by subsoil removal.

3.3 Clearing and Grubbing

On projects where right-of-way or median widths are adequate and where it is desirable to leave selected areas of natural growth, the area 5 feet to 10 feet outside the construction limits should be specified for no clearing and grubbing.

No delineation of clearing and grubbing is required, except on the typical section. If selective clearing and grubbing is specified, the limits of these areas should be shown as described in Chapter 11, Volume II.

3.4 Final Dressing

Final dressing is not used as a pay item. This work will be included incidental to other items in the plans.

Chapter 4

ROADSIDE SAFETY

4.1 Clear Zone

4.1.1 Clear Zone Concept

A significant number of accidents involve a single vehicle leaving the roadway and either overturning or colliding with a fixed object. A roadside that is traversable and unobstructed by fixed objects will allow vehicles that leave the roadway to recover safely. The designer should provide as much traversable and unobstructed area (clear zone) as possible. If natural or man-made design features that may constitute a hazard occur within the needed clear zone the designer should attempt the following treatments, in order of priority:

- 1. Remove the obstacle from the needed clear zone.
- 2. Make the obstacle traversable or crashworthy.
- 3. Shield the obstacle with a longitudinal barrier or crash cushion. This action should only be taken if the obstacle represents <u>more</u> of a hazard than the barrier or crash cushion.
- 4. Leave the obstacle unshielded. This action should be taken only if a barrier or crash cushion is more hazardous than the obstacle, if the likelihood of striking the obstacle is very small or if the expense of treatment outweighs the benefits in terms of accident reduction.

Conditions are considered traversable if a vehicle can traverse the condition without seriously endangering the occupants. Conditions are considered recoverable if there is a reasonable probability of regaining control of a vehicle and returning to the roadway.

4.1.2 Clear Zone Criteria

Criteria have been developed with the objective of providing the necessary recovery area for the vehicles that might leave the roadway. The criteria are based on limited empirical data which was then extrapolated for a wide range of conditions. The criteria represent a reasonable degree of roadside safety, but they are neither absolute nor precise. These criteria must be applied with judgement. In some cases, the clear zone can be adjusted higher or lower than shown. In all cases, the most clear zone that can be practically provided is desirable.

Exhibits I-4-A and I-4-B are copies of Roadway Design Standard Index 700, which includes specific instructions for determining clear zones, as well as other design criteria related to highway safety.

4.1.3 Early Removal of Fixed Object Hazards

If accident data or safety reports indicate that early removal of fixed objects such as trees, endwalls or utility poles will result in fewer or less severe accidents, designers should direct that those conditions be removed, replaced or shielded as the first order of work.

			DE.	SIGN	C	4//E	:R	IA RE	LATED TO HIGHW	AY	SAFET	Υ			
TYPE OF FACILITY	RURAL & URBAN FRE RURAL ARTERIALS & DESIGN SPEED OF 4. GREATER AND PROJE (20 YR) OF 1500 OR (OF SIGN	N SPEI	ED OF 4	45 A JECT	LLECTORS: IPH OR ED ADT	, 	DE SIG	N ARTERIALS W SPEEDS OF HOUT CURB AN	30 -	URBAN ARTERIALS & COLLECTORS: DESIGN SPEED OF 45 MPH OR LESS (CURB AND GUTTER)				
EMBANKMENT SLOPE	FIII H1. 0' - 5' 6:1 5' - 10' 6:1 to edge 10' - 20' 6! to edge > 20' 2:1 (with gue RZW cost must be cor urban areas in using	0		5'6:l ei Insuf edge be pe 0'6:l l excep Insuf	rillittea. To edge o I where ficient ti	of C R/W hen	R/W Is 5: to 5: will Z & 3: V Is 2: with permitted.	Fili HT. O' - 5'6' I except where R/W is insufficient then 6:1 to edge of CZ & 3:1 will be permitted. 5' - 20'6:1 to edge of CZ & 3:1 Insufficient then 2:1 with quadratil will be permitted	5' -	5'6:1 except winsufficient edge of CZ be permitte 20'6:1 to edge except when insufficient guardrall w	then 6: & 3:1 d. of CZ e R/W then 2:	2:1 or to sult property owner, not flatter than 6:1. R/W cost must be considered for high fill sections in urban areas.			
CLEAR WIDTHS FOR BRIDGES	Freeways, divided ar, collectors (4 or more Travel lanes plus 10' R 10' Lt. for 6' or more Undivided arterials & Travel lanes plus approvidth.	6'Lt.		lanes , ler wla	olus appi	roac	h	Trovel lanes plus approach shoulder widths.	appro 6'Lt. Is ca Undiv	ed: Travellane oach shoulder w unless full med rried across s rided: Iravella oach shoulder v	ildth Rt Ilan sec tructuri mes plus	Full section (face to face of curb) plus sidewalk. When concrete barrier wall is placed in front of sidewalk a minimum clearance of 2.51s required between the travel lane, wide curb lane or blycie lane and the toe of the barrier wall.			
BACKSLOPES	4:1 (Normal)		4:/ w	here A	/W peri	mIts	or 3:1.	4:1 where R/W permits or 3:1.	4:/ v	where R/W per	mits or	2:1 or to sult property owner, not flatter than 6:1.			
CLEAR ZONE	Design Fravel Lanes & MPH Lanes Ramps 60 -7U 30' 18'	Design Speed MPH 60 -/0	Travel Lanes 24	lmum AuxII. Lanes & Ramps	Tro Lan		i4' destrable; IO' minimum from edge of both travel and auxiliary lanes.	Design Speed MPH 50	Travel Lanes Lanes Ramps	& Trave Lanes	Ramps /4'	4'from face of curb. Ø Bridge piers normally will be 16'min. from edge of travellane. *			
003	55 24' 14' 45 -50 18'€ 14'€		18' 14'	55 45 -50	18' 14'	14'	24	1' 14' 7' 14'		45 30 -40	18'50 10' 14'60 6'3	24' 18'	14' 10'		
GUARDRAIL LOCATION	Shoulder width plus a guardrall (at shoulder shoulder width is 12' desirable to locate or see Detall K. Index No	when be tsiope;	auard1	rall (8' i ite on i	min.). Ma rontsiop	w be	face of desirable ee Detall K.	Shoulder width plus 2' to face of guardrall (8' min.). May be destrable to locate on frontslope; see Detall K Index No. 400.	quar	ilder width plui drall (8' mln.).	5 2' to f	Normally flush with face of curb.			
SIGNS	Not generally in medi- clear zone or behind that is justified for reasons. Confilever to be located inside cite profected by barrier single column signs in in accordance with T Design Standards and All supports are bree frangible except over lever or fruss signs	barrie other may be zone p Frang, be lock Traff, Index are br except	r that reason locate rotecte lible sir ated in lo Desi No.173	zone a. Is Justi s. Canti d inside d by bai agle colui accorda gn Stand 02. All s ad canti	ified ileve e cle rric imn s ance dard supp rangli	for r signs ar r. signs to with s orts	Outside clear zone or behind borrier that is justified for other reasons. Contilever signs may be located inside clear zone protected by borrier. Frangible single column signs to be located in accordance with Traffic Design Standards Index No. 17302. All supports are breakway or frangible except overhead contilever or truss signs.	barrothe may zone Fran be id Tran inde, are exce	Ide clear zone iler that is jus r reasons. Car be located insi protected by t ngible single co cated in accor ffic Design Sta x No.17302. A breakaway or i ppt overhead car s signs.	atified t atiliever de clear warrier. lumn sig dance w ndards il suppor	2' mln. From face of curb to Inside edge of sign ponel. Sign pacement shall not black sidewalk.				
LIGHT POLES	Not generally In media when shielded by bor. Outside clear zone o. base 20'from edge o lane and 14'min. from auxiliary lane or beh barrier that is justi other reasons.	Outsid frongi if clea at 20' i4' fro behind Justif Frang 20' fro auxilla	le clear ible and ir zone from edge m edge i appro led for lble bas om thr	zone for the control of the control	rangi less thru illar rier reas be L	ible bases Destrably lane and y lane or that is sons. laced at 4' from	Outside clear zone or behind approved barrier that is justified for other reasons.	Outs appr Just Fran 20' f	Ide clear zone oved barrier tifled for other tifled for other orgalishe bases may room thru lane for other than 20'.	hat Is reasor y be pla and 14'	4'mln.from face of curb.®				
UTILITY POLES, FIRE HYDRANTS, ETC.	Not in median. Not will the main travelway of For other facilities clear zone. Normally M/W when beyond clear therwise as close as to R/W line.	Norma beyond	illy 6.5' 1 clear	zone. Insido F zone ot practical	therv	vise	Outside clear zone. Not in median. Normally 6.5' Inside R/W when beyond clear zone otherwise as close as practical to R/W line.	No rm beyor	ide ciear zone. nully 6,5' Instide nd clear zone c lase as practice	R/W witherwis	Not in median. 4' min. from fac of curb.0				
RAILROAD CROSSING DEVICES	Not on freeways. IO min. from edge of or 6 min. from edge lane to near edge of No guardrall.	or 6' 1	min.fra near	edge of	of.	auxillary	Minimum from edge of travellane to near edge of device: Greater than 40 mph − 10′ 35 − 40 mph − 8′ 30 mph or less − 6′ Minimum for auxillary lanes − 6′ No guardrali ☆	fo nei Gre 35 30	oum from edge ar edge of dev eater than 40 : - 40 mph mph or less oum for auxillo vardrall ◇ ☆	Ice: nph - Ii - - 6 irv lanes	2.5 min. from face of curb to near edge of device.				
MEDIAN WIDTHS	Freeways 60' mln 60 mph of 40' mln under 60 0ther divided highway 40' mln 55 mph of 22' mln under 55						22' Min.	19.5' I 15.5' I 19 gred For min. p provis	min. – 45 – 5 min. – 40 mph oter widths des reconstruction painted median sion for left tu oter widths des	O mph or less irable i projects width w irn is 10	19.5 min 45 mph 15.5 min 40 mph or less (greater widths desirable) For reconstruction projects the min. pulnited median width with provision for left turn is 10' (greater widths desirable)				
TREES △ Existing Or Expected	REES \(\triangle \) If or Expected for Freeways.					ar zone.			Outside clear zone.		de clear zone.		d'min, from face of curb where curb height is 6' or greater.		

Design speed to be established using realistic anticipated operating speed.

Preferred ditch cross sections are shown on pages 25, 26 & 27 of the AASHTO Guide For Selecting, Locating And Designing Traffic Barriers.

Consideration should be given to maintaining greater than the above specified clearances and/or flatter slopes where feasible unit pructicul.

Values shown above shall be used on all new construction and on reconstruction projects to the extent that economic and environmental considerations and R/W limitations will cliow. For definitions of new construction and reconstruction see Manual Of Uniform Winimum Standards For Design, Construction And Maintenance For Streets And Highways. The values shown do not apply to RRR projects. Driving lane is any traffic lane, travel or auxiliary.

An auxiliary lane is the portion of the roudway adjuining the traveled way for parking, speed changes, turning, storage for turning, weaving, truck climbing or for other purposes supplementary to through traffic movement.

Traveled way (Travelianes) is the portion of the roadway for the movement of vehicles, exclusive of shoulders a

- On projects where the 4 foot minimum offset cannot be reasonably obtained and other alternatives are deemed impractical, the minimum may be reduced to 2.5%.
- * At locations where immediately adjacent development such as buildings, etc. provide less clearance, bridge piers can be placed to provide clearance less than i6'. Adequate sight distance for crossing maneuvers at intersections should be provided.
- △ Offsets shown are for existing or newly planted trees. If existing trees are close to but less than the minimum offset indicated, other factors should be considered to determine adequacy of offset, i.e. ADT; operating speed; accident history; size, age and type of tree; protection by a barrier; etc. Newly planted trees may be placed behind barriers that are provided for other reasons. The minimum setback distance from the back of concrete barriers, wails, abutments or other rigid obstructions is 4 for newly planted trees. For W-board quarterial the minimum setback is 6 from the face of the rail for newly planted trees. Grees. Offsets shown apply to both outside and median for divided highways unless otherwise noted. When trees are placed in median adequate sight distance at intersections, turnouts and median openings, etc. shall be maintained.
 - ☐ IO minimum from driving lane when curb height is less than 6 or when curb height has been reduced by resurfacing.
 - + When offset established, care shall be taken to avoid blocking sight distance to roadside signs.
 - For divided facilities with curbed medians and undivided facilities utilizing curbed separations at crossings, 2.5' from face of curb to near eage of signal device in accordance with i rattic Design Standards Index No. 17882. For divided facilities with curbed medians and undivided edge of signal device in accordance with I rath tic Desiral Not less than 2'from edge of any shoulder pavement.

 Clear Zone Footnates:

DESIGN TO

CRITERIA HIGHWAY S

'A RELATED 'SAFETY

TRANSPORTATION

Names Names 06/8 06/8

1 of 2

Approved By

- Clear Zone Footnales:

 Destrable values are to be used for all new construction projects. These values may be reduced only where individually justified to milligate critical social, economic, and environmental impacts or to lessen excessive right-of-way costs. Destrable values are also to be used for reconstruction projects however, values less from destrable down to minimum may be used where individually justified of use to critical social, economic, and environmental impacts and/or excessive right-of-way costs economic, and environmental impacts and/or excessive right-of-way costs evoluence when existing roadside obstacles are not considered hazardous as evidenced by field review and by accident history or accident potential.
- (2) Where accident history indicates need, or where specific site investigation shows definitive accident potential clear zones for rural and urban facilities (without curb) may be adjusted on the outside of horizontal curves in accordance with Table I (Shi, 2 of 2).
- The use of barriers or other safety treatment is to be considered if clear zone requirements are not provided.
- $\stackrel{\frown}{4}$ 10' in median where median curb Type E is used on rural collectors with a design speed of 45 mph.
- (5) May be reduced to low speed condition criteria (30-40 mph) if conditions more nearly approach those for low speed.
- 6 May be reduced to ID' for collectors.
- ① 10' In medians where median curb Type E is used; 4' from face of median curb where curb height is 6'.
- 8 4' from face of median curb where curb height is 6".

EXI-4-A

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4.2 Canals

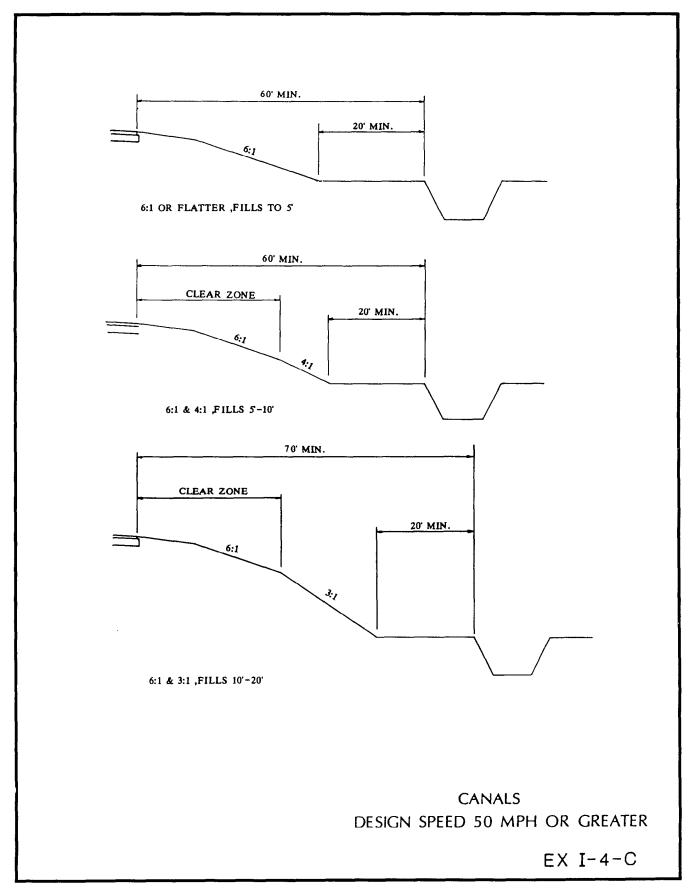
A canal is considered to be an open ditch with the canal side slope adjacent to the roadway 4:1 or steeper and/or with a seasonal water depth in excess of three feet for extended periods of time (24 hours or more). When the roadway slope and the canal side slope adjacent to the roadway is 6:1 or flatter the minimum distance to canal may be measured from the edge of the through travel lane to the "extended period of time" water surface. A berm between the roadway front slope and canal slope is not required for this condition.

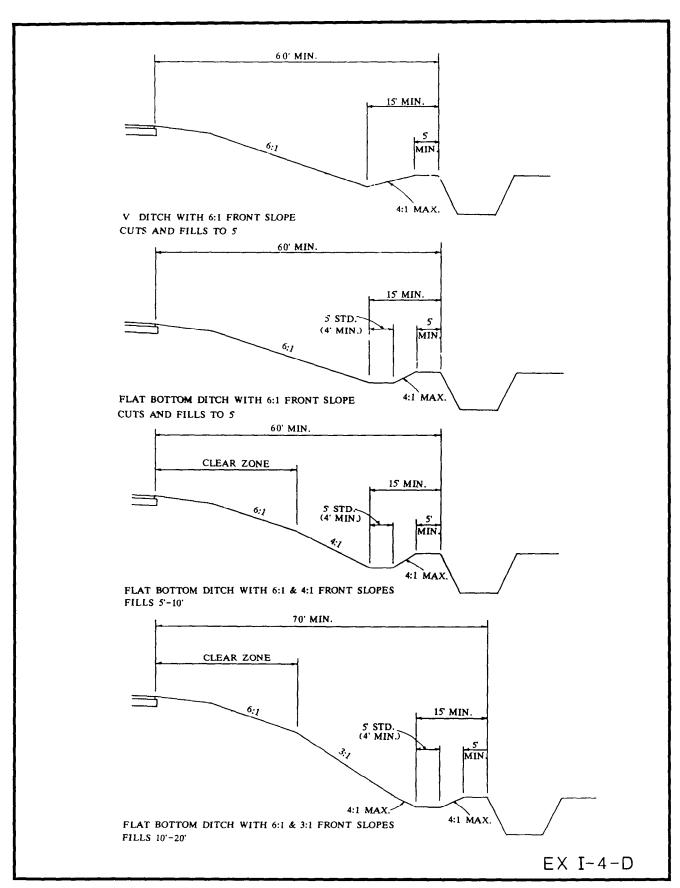
The distance from the outside edge of the through travel lane to the top of the canal side slope nearest the road will be no less than 60 feet for highways with design speeds of 50 MPH or greater. For highways with design speeds less than 50 MPH this minimum distance may be reduced to 50 feet for rural highways or 40 feet for urban (curb and gutter) highways. When new canal or roadway alignment is required, distances greater than these above should be provided, if possible, to accommodate possible future improvements to the roadway (widening, etc.).

On fill sections, a flat berm (maximum 10:1 slope) of width no less than 20 feet will be provided between the toe of the roadway front slope and the top of the canal side slope nearest the roadway. This minimum distance applies to all types of highways, both rural and urban (curb and gutter) construction. Exhibits I-4-C through I-4-F illustrate various roadway fill section configurations.

In cut sections a minimum of 15 feet will be provided between the inside edge of the roadside ditch bottom and the top of the canal side slope nearest the road.

Guardrail, or other protective devices shall be installed where it is impossible to meet the above minimum criteria.





4.3 Roadside Barriers

4.3.1 Warrants

Roadside barriers are warranted when non-traversable conditions or obstacles exist within the needed clear zone, the conditions cannot be cost effectively eliminated or corrected and a collision with the condition will be more serious than a collision with the barrier.

The following conditions are normally considered more hazardous than a roadside barrier, depending upon speed, volume and type of roadway:

- o Fill slopes steeper than 3:1. Steeper slopes may be acceptable for fill heights less than 6 feet.
- o Bridge piers, abutments and railing ends.
- o large, non-traversable culverts, pipes and headwalls.
- Non-traversable parallel ditches and canals.
- o Bodies of water other than parallel ditches and canals that the engineer determines to be hazardous.
- o Parallel retaining walls with protrusions or other potential snagging features.
- o Retaining walls at an approach angle with the edge of pavement larger than 7 degrees (8:1).
- o Non-breakaway sign or luminaire supports.
- o Trees greater than four inches in diameter.
- o Utility poles.
- o Rigid protrusions above the ground in excess of four inches.

In addition to protecting the above hazards, there may be situations that warrant barrier installation due to involvement of "bystanders," such as nearby pedestrian or cyclist facilities, schools, residents or businesses.

4.3.2 Barrier Selection

The following roadside barriers are acceptable:

- Standard blocked-out w-beam on 6 inch X 8 inch wood post (strong post).
- o Standard blocked-out w-beam on 6 inch "C" steel post (strong post).
- o Standard blocked-out w-beam on W6 X 8.5 or W6 X 9 steel post (strong post).
- Blocked-out Thrie-Beam on any of the above post systems.
- o Concrete barrier wall.

Most guardrail installations will be blocked-out w-beam on wood or steel posts. The Thrie-Beam guardrail should be considered when additional rail depth is needed because of a potential to under-ride the rail or because additional height may be needed. Concrete barrier wall may be used in locations where no barrier deflection can be tolerated.

4.3.3 End Treatments

Non-crashworthy longitudinal barrier ends can represent serious hazards if they occur within the needed clear zone. The following crashworthy end treatments for upstream terminations will be used:

Type IV End Anchorage. It is very important the standard 4 foot parabolic flare be provided as shown in Roadway Design Standard Index 400. This flare should be off a projection of the guardrail alignment immediately downstream. If the guardrail alignment is on a flare off the roadway, the standard 4 foot parabolic flare is an

additional flare. The area in front of the Type IV shall be no steeper than 10:1 with no curbs, as shown in Roadway Design Standard Index 400.

- o The maximum allowable cross slope in front of the rail is 10:1, including the area in front of the Type IV and the upstream approach to the Type IV. A structurally adequate anchorage system must be provided.
- o Other, non-crashworthy end terminal treatments will be used only outside the needed clear zone.
- o Thrie-beam and concrete barrier wall will be terminated as discussed above. Appropriate transitions to w-beam guardrail will be necessary to use the Type IV. Crash cushions may also be used to terminate thrie-beam or concrete barrier wall. The sloped concrete barrier wall terminal will only be used on roadways with an operating speed of 40 MPH or less.

The Type II end anchorage is not crashworthy and, therefore, may only be used at downstream terminations where it is not likely to be struck.

4.3.4 Transitions

Whenever standard w-beam or Thrie-beam guardrail transitions into bridge rail or concrete barrier wall, a transition section is necessary. Transitions must include sound structural connections, nested beams and additional posts for increased stiffness; as shown in Roadway Design Standard Index 400. It is preferable that transition sections be parallel to the edge of the pavement. Flares should be introduced upstream of the transition section. Care must be taken in the details of the junction of the two barrier types to avoid snag points.

4.3.5 Placement

The primary design factors associated with guardrail placement are:

- o Lateral offset from the edge of pavement
- o Terrain effects
- o Flare rate
- o Length of advancement

As a general rule, a roadside barrier should be placed as far from the traveled way as conditions permit. The minimum offset should be the shoulder width plus two feet, not to exceed 12 feet.

If guardrail must be located within the desirable offset distances, the designer should realize that there will be an adverse impact on capacity and traffic operations.

A two foot distance from the back of the barriers to the hinge of a slope break is desirable for post support. Although it is desirable to locate the barrier as far from the travel way as possible, it should not be located so close to the hazard that it is protecting that it is within the dynamic deflection distance of the barrier. The dynamic deflection of standard barriers are shown in Table 4.3.2.

Table 4.3.2

Dynamic Deflection of Barriers

(Measured from the back of the barrier post)

Barrier Type	<u>Dynamic Deflection (ft.)</u>
W-Beam, Strong Post	4
Thrie-Beam, Strong Post	2
Barrier Wall	0

Curbs should not be placed in the front of barriers. If curbs are necessary, it is recommended that the face of the curb be located at the face of the guardrail or behind it.

Barriers should not be placed on slopes steeper than 10:1. This is particularly important on the approach to the standard flare of the Type IV end anchorage.

4.3.6 Resetting Guardrail

For those projects that include the resetting of guardrail, the quantity of guardrail panels, posts and special posts that are determined to be non-salvageable shall be shown on the plans (538-1 pay item footnote) since the method of measurement for

resetting guardrail is lineal foot. These items need to be determined (by visual observation) prior to letting project to contract in order to avoid the 25% extra charge over cost and transportation when found to be non-salvalvageable during construction, as shown in the Standard Specifications. When guardrail is to be reset, concrete posts are to be considered non-salvageable.

The following is provided for guidance for determining non-salvageable items when specifying quardrail to be reset:

- o <u>Posts</u> All concrete; Improper size; Impact damaged; Deteriorated; (Since deterioration cannot be detected in most cases without removing wood posts, determination must normally be made during construction.)
- <u>Panels</u> Warped as a result of impact; (minor dents are not justification).
- o <u>Special Posts</u> All concrete; improper size; impact damaged; special posts that will not have proper height as a result of relocation or rail adjustment.
- o <u>End Treatments</u> All upstream end treatments that do not meet the criteria discussed in 4.3.3.

Concrete posts are to be removed and replaced with wood or steel only when it is necessary to reset guardrail because of other substandard features. Substandard features include such items as lateral clearance, height, blockouts, post spacing, type of rail, type of end treatment, etc., all of which must be in conformance with the latest version of the Department's guardrail Standard Index Drawings. It is the responsibility of the designer to verify that the existing guardrail does or does not meet all current standards.

A pay item footnote should also be used to notify the contractor about unusual features in resetting existing guardrail, i.e. new posts required since the existing post spacing is 12.5 feet, etc.

Downstream Type II end anchorage installations can be adjusted by cutting the anchor side and adding a turnbuckle, as shown in Roadway Design Standard Index 400.

4.4 Median Barriers

4.4.1 Warrants

Median barriers are warranted on freeway or expressway sections when the minimum median width shown in Exhibit I-4-A cannot be provided.

4.4.2 <u>Selection</u>

The same barrier types as discussed in Section 4.3.2 are available as median barriers. In general, the concrete barrier wall is preferable in narrow medians.

4.4.3 End Treatments

Median barriers can be terminated with any of the treatments discussed in 4.3.3. However, care must be taken to ensure that the non-crashworthy back of an end treatment in a median does not encroach into the needed clear zone of the opposing traffic. If that occurs, it must be protected by a barrier for the opposing traffic.

4.5 Crash Cushions

4.5.1 Warrants

Fixed point objects within the necessary clear zone, as discussed in 4.3.1, which cannot be cost effectively eliminated or corrected, and represent a more serious collision potential than a crash cushion, are warrants for the installation of a crash cushion. There are situations where crash cushions may be a more cost effective treatment than a longitudinal barrier. Crash cushions can be used to shield the ends of roadside and median barriers.

4.5.2 Selection

The following types of crash cushions are currently recommended for use:

- o Hex-Foam Sandwich System
- o Guardrail Energy Absorbing Terminal (G.R.E.A.T.)
- o Sand-filled Plastic Barrels

The AASHTO Roadside Design Guide and manufacturer's materials provide detailed information about these systems. Each system has its own unique physical and functional characteristics. The design engineer should consider the following factors when selecting a system for a particular situation:

- o Site characteristics
- Structural and safety characteristics of candidate systems
- o Initial and replacement/repair costs
- Expected frequency of collisions
- o Maintenance characteristics

Site characteristics frequently dictate the crash cushion For instance, the G.R.E.A.T. is the only system selection. appropriate for very narrow installations. If more than one system is appropriate, an economic analysis should be performed. Sand barrels are relatively low in initial cost, but usually must be completely replaced when struck, so are more appropriate in locations with a low likelihood of collision. The other systems have higher initial costs but can be repaired after collisions relatively cheaply, so are more appropriate where frequent The ability of maintenance forces to collisions are expected. perform routine maintenance and to place a crashed system back into service quickly should be a major consideration. cushions that require stocking unusual and expensive parts or that are complex to replace should not be selected.

4.5.3 Design

Crash cushion suppliers normally provide design assistance for their system. Crash cushions must decelerate both large and small automobiles from the design speed of the facility to stop. As described in the AASHTO Roadside Design Guide, crash cushions should be designed so that the velocity with which an unrestrained passenger strikes the interior of the vehicle should not exceed 40 feet per second, and the subsequent vehicle deceleration should not exceed 20q's (highest millisecond Preferred values are 30 feet per second and 15q's. These occupant risk criteria differ from earlier quidelines and are not directly comparable with acceleration forces averaged over the entire crash event. However, the acceptable levels of safety performance are approximately the same, and the various design charts prepared by the manufacturer of proprietary crash cushions may be used to select an appropriate unit. charts are used, the maximum average deceleration level should not exceed approximately 7g's.

All terrain within the likely approach of a vehicle should be relatively flat and level. An impacting vehicle should strike the unit at normal height, with the vehicle's suspension system neither collapsed nor extended. Curbs shall not be used in the approach area of a crash cushion.

Sand barrels do not have redirection capability, so they should be oriented toward the expected angle of attack (10 - 12 degrees).

Care must be taken with all systems protecting a rigid object to ensure that there is not a potential to impact the upstream corner of the object in a side impact.

The nose of all crash cushions shall be delineated with reflective material or standard object markers.

4.6 Roadside Appurtenances

4.6.1 Sign Supports

All sign supports, except overhead cantilever or truss type, shall be either breakaway or frangible as defined in the AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals and the AASHTO Roadside Design Guide. Supports not meeting the frangibility or breakaway criteria should not be installed within the clear zone. Signs not meeting these requirements which must be installed within the clear zone shall be protected by a barrier or crash cushion. The following are specific design considerations.

- o <u>Single Post Signs</u> -- The support sizes and proper location for these signs are covered in the <u>Roadway and Traffic</u> Design Standards.
- Multi-Post Signs -- Ground mounted signs not covered in the 0 Roadway and Traffic Design Standards as single post signs must be supported by two or more posts. These supports shall be designed to breakaway independently and release This release is provided by a hinge from the sign panel. point on the support located a minimum of 7 feet above the ground. The posts shall be separated by a distance of 8 feet or more or shall be considered to act together as a single post. A single post or all posts within a 7 foot path should not weigh more than 45 pounds per foot or have a total weight of more than 600 pounds. No sign attachment should be made below the hinge point. This will interfere with the breakaway action of the post.

4.6.2 Mailbox Supports

Mailbox supports shall be of an acceptable crashworthy design, as described in Roadway Design Standard Index 532.

4.6.3 Other Appurtenances

Exhibit I-4-A contains design criteria for numerous other roadside appurtenances.

4.6.4 Location Criteria

Most breakaway mechanisms are designed to be impacted at bumper height, typically about 20 inches above the ground. If impacted at a significantly higher point, the bending moment in the breakaway base may be sufficient to bind the mechanism, resulting in non-activation of the breakaway device. For this reason, it is important that breakaway supports not be located in ditches or on steep slopes where a vehicle is likely to be partially airborne at the time of impact.

Chapter 5

UTILITIES

5.1 General

Highways serve the public by carrying people and goods from place to place. Public and privately owned utilities have a public-serving function similar to that of highways. The needs of utilities (power lines, communication lines, gas and other pipelines, water mains and sewers) to go from place to place (often to or from the same points as the highway system), are recognized by the Department. Utility facilities, both above and under ground level, are permitted by the Department to be accommodated within the road rights-of-way on the state maintained highway system (non-interstate).

For interstate facilities parallel utilities within the right-of-way are not allowed. Lateral crossings are allowed by permit only (see Utilities Policy # 71000-020-001-a). The designer should make every effort to design a project that will avoid conflicts with major utilities. The selection of typical section features, horizontal alignment and location of storm sewer lines are two areas that can sometimes be adjusted without violating safety standards and design criteria. A dual storm sewer system to avoid lateral crossings may be practical on some projects to avoid utility conflicts. The increased cost of some features may be offset by reduced construction time and the associated costs.

5.2 Relocation

Relocation is the adjustment of utility facilities required by a roadway project. Examples are removing and reinstalling a utility at a new location, moving or rearranging existing utilities, changing the type of facility, improving safety and protective measures. Relocation includes constructing a replacement facility functionally equal to the existing facility, where necessary for continuous operation of the utility services, the project economy, or sequence of roadway construction.

5.3 Utility-Accommodation Guide

The Department has established certain guidelines to regulate the location and manner for installation and adjustment of utility facilities in order to ensure safe and efficient operation and maintenance of the roadway facility.

The Department's <u>Utility Accommodation Guide</u> governs on matters concerning future location, manner and methods for the installation or adjustment and maintenance of utilities on highway right-of-way.

A permit must be approved by the Department before any utility is installed on the right-of-way, whether it is for aerial or underground installations. Exceptions and requirements for permits are enumerated in the quide.

Design considerations for accommodating utilities within the highway rights-of-way are given in the AASHTO publications <u>A Guide for Accommodating Utilities within Highway Right-of-Way and A policy on Geometric Design of Highways and Streets.</u> Additional information can be found in the TRB's publication <u>Policies for Accommodation of Utilities on Highway Rights-of-Way</u>.

5.4 Verification of the Location of Major Existing Utilities

Utility delays account for 17 to 18% of the construction contract time extensions. Inaccurate location of existing utility facilities has been identified as a significant contributor to these extensions. As a result, positive horizontal and vertical verification of the location of major existing utilities is required on those urban reconstruction and add lane projects where timely completion is essential because of disruptions to traffic flow.

Major utilities are defined as those underground utilities that potentially conflict with construction activities to the extent that the existing location may interfere with pavement construction, stabilizing, curb and gutter construction, bridge foundations, sign and signal foundation, light poles, drainage structures, and/or storm sewer installation, manholes or inlet construction or those utilities that lie within the normal excavation limits for these structures. Construction personnel should be consulted for assistance in determining the excavation limits. Major utilities are also defined as water mains (6 inches or larger), all gas lines except service lines, telephone-coaxial toll lines and multi-duct lines, telephone cables and ducts (50 pair or larger), sanitary lines (all gravity flow mains), sanitary force mains (6 inches or larger) and electric power cable (all buried electric transmission cables - not service Service connections are not to be considered as major lines). utilities.

Positive verification of utility locations can be either by DOT or consultant survey or utility company verification based on previous surveys or certified as built plans. To avoid obtaining detailed verification at locations that may not be necessary, it is desirable that designs be advanced to the stage that the location of the storm sewer is known. It will be the responsibility of the District

Utility Engineer, or his designee, with assistance from the Designer and construction personnel to determine the appropriate locations of positive verification.

5.4.1 Locating Underground Utilities

To further identify the responsibilities of the Department and utility companies regarding the location of underground utilities, the following will be used along with, and in accordance with, the <u>Location Survey Manual</u>:

- Contact the utility owner, give details as to the location, length, and/or extent of job. Advise if the underground utilities are to be located horizontally only, or that they will have to be uncovered and located both horizontally and vertically. Request that personnel necessary to locate the utilities meet DOT or consultant forces at a specific time and place.
- 2. All underground utilities must be marked by the company representative with a DOT employee or consultant present, giving size, type and use. The utility company will furnish the DOT with as built plans, if available. The DOT or consultant survey forces will tie all major utilities and others deemed necessary to the centerline or baseline of survey and record all field data usually horizontally only, giving the approximate depth estimated by the utility company representative. If field verified vertical data are also available, these data will also be shown by positive tie in the field book.

- 3. During the design of the project, those major utilities requiring vertical location of the underground utilities will be identified. The utility company will be contacted to uncover (dig, excavate, and so forth) their utilities as deemed necessary by the DOT. The DOT location or consultant survey personnel will locate and record all finding, both horizontally and vertically, showing the positive horizontal and vertical ties in the field book for future reference. All storm sewer systems and sanitary sewer gravity flow lines will be shown as presently outlined in the <u>Location Survey Manual</u>.
- 4. All as built plans obtained from the utility companies are to be marked as follows:

Date
DOT Job No.
Party Chief
Utility Company
Utility Company Representative

All field books in addition to the standard heading, index, etc., will denote the following:

Date
Personnel
Utility Company
Utility Company Personnel
Utility Company Work Order No., if applicable.

Those locations where positive horizontal and vertical location is known will be designated on the plans. It is important to the contractors bidding on projects to know at which locations the information is exact and not interpolated. Volume II gives specific instructions on how to show this information on the plans.

5.5 Coordination Process

Copies of plans, with all utilities shown, should be given to the utility owner for verification at approximately 30% design completion stage. Right-of-way map or R/W key map with existing utilities shown may be used for utility contact. The District Utility Engineer normally coordinates this activity through a utility predesign conference. The utility owners are required to verify or indicate the correct location of their respective utilities within the project limits on the plans provided to them. These marked up plans should be used by the design team to show the correct locations, horizontal and vertical, of the various utilities.

The designer should make every effort to design a project that will avoid conflicts with major utilities. At 60% completion of the project, when the drainage design is well underway, the potential utility conflicts should be identified. Only major utilities potentially in conflict shall be field verified horizontally and vertically.

At approximately 80% completion of plans, the utility companies should be provided with two sets of plans showing all utilities. One marked up set is transmitted back to the designer and the other is retained by the utility company for their use.

Positive verification of utility locations may be done either by DOT, consultant or utility company. Verification can also be based on previous surveys or from certified as-built plans. It will be the responsibility of the Utility Engineer, or his designee, with assistance from the Designer and construction personnel, to determine the appropriate locations of positive verification.

Chapter 6

RATIROAD CROSSING

6.1 General

A railroad-highway crossing, like any highway-highway intersection, involves either a separation of grades or a crossing at grade. The geometrics of a highway and structure that entails the overcrossing or undercrossing of a railroad are substantially the same as those for a highway grade separation without ramps.

Selection of the warning devices to be used is a function of the geometrics of railroad-highway grade crossing, including the alignment, profile, sight distance and cross section of both the roadway and the railroad. Railroad grade crossing angles should be as near 90 degrees as practical.

Design Criteria and Standards are given in the Florida Green Book and the Department's <u>Railroad Procedures Manual</u> Volume III. Design considerations are discussed in Chapter IX of the AASHTO policy on Geometric Design.

6.2 Devices

Traffic control devices for railroad-highway grade crossings consist primarily of signs, pavement markings, flashing light signals and automatic gates. A large number of significant variables must be considered in determining the types of warning device to be installed at a railroad grade crossing. The type of highway, volume of vehicular traffic, volume of railroad traffic, speed of vehicular traffic, volume of pedestrian traffic, accident record, and geometrics of the crossing are some of the factors influencing the choice of warning devices to be provided at the railroad crossing. Standards and criteria for design placement, installment and operation of these devices are covered in the MUTCD and the Department's Railroad Procedure Manual Volume III. The Department's Roadway and Traffic Design Standards should also be consulted in the design of railroad crossings.

6.3 Surfaces

The highway traveled way at a railroad crossing should be constructed for a suitable length with all-weather surfacing. A roadway section equal to the current or proposed cross section of the approach roadway should be carried across the crossing. The crossing surface itself should have a riding quality equivalent to that of the approach roadway. When selecting the type of crossing and the material to be used in its construction, consideration should be given to the character and volume of traffic using the highway. The Department's Highway-Railroad Grade Crossing Material Selection Handbook should be consulted in selecting the material.

Roadway Design Standard Index #560 contains specifications for the construction of crossings of the following types.

Bituminous
Wood Plank Crossing
Prefabricated Sectional Treated Timber Crossings
Concrete Slab Crossings (Precast)
Concrete Slab Crossings (Prestressed)
Steel Grid Crossing
Rubber (Elastomeric) Panel Crossings
Epoxy-Rubber Mix Cast-In-Place Crossings
Linear High Density Polyethylene Modules Crossings

6.4 Clearances at Railroad/Highway Structures

An offset horizontal dimension to the abutment slope of up to 20 feet from the centerline of tracks requires no special justification. Horizontal offsets in excess of 20 feet should be justified based on individual site conditions. Vertical clearances up to 23 feet require no special justification. Greater vertical clearances can be justified based on special site conditions or the need to meet documented railroad electrification plans. The horizontal clearance to an obstruction is a minimum of 9 feet. Greater clearances may be necessary to preclude the placement of piers in drainage ditches. Roadway Design Standard Index 280 contains additional criteria for culvert clearances below railroads.

Chapter 7

SIGNING, MARKING, LIGHTING AND SIGNALS

7.1 General

Traffic control devices are necessary to help ensure highway safety by providing the orderly and predictable movement of all traffic, motorized and non-motorized, throughout the highway transportation system, and to provide such guidance and warnings as are needed to ensure the safe and informed operation of individual elements of the traffic stream. The design and layout of signs, signals, pavement marking and lighting should complement the basic highway design.

7.2 Signing and Marking

The designer responsible for a signing and marking project should be aware that the design must comply with various standards. In addition to Department Standard Specifications, the following standards should be consulted:

Manual on Uniform Traffic Control Devices (MUTCD) - The MUTCD was adopted by the Department as the uniform system of traffic control for use on the streets and highways of the State. This action was in compliance with Chapter 316.0745 of the Florida Statutes. The MUTCD is therefore the basic guide for signing and marking. The requirements of the MUTCD must be met, as a minimum, on all roads in the State.

<u>Standard Highway Signs</u>, FHWA - This manual contains detailed drawings of all standard highway signs. Each sign is identified by a unique designation. Signs not included in this manual or in the <u>Roadway and Traffic Design Standards</u> must be detailed in the plans.

Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals, AASHTO - This specification contains the strength requirements of the sign supports for the various wind loadings in the State as well as frangibility requirements. All signs supports must be in compliance with these specifications.

Roadway and Traffic Design Standards - These standards are composed of a number of standard drawings or indexes which address specific situations which occur on a large majority of construction projects.

7.2.1 <u>Design Criteria</u>

The MUTCD and the <u>Roadway and Traffic Design Standards</u> should be consulted for sign location. All signs not bridge or barrier wall mounted and installed within the clear recovery zone, must be frangible or protected by an approved barrier. Chapter 4 of this volume contains detailed instructions on safety design.

Post sizes for single column signs are covered in Hite Roadway and Hiraffic Design Standards. The supports for multi-post signs are not in that reference and must be included in the plans. The designer must provide post sizes and length for each multi-post sign. The Structures Design Office has written a program for personal computers that calculates post sizes and length for multi-post signs. This program may be used for these calculations.

Signs will be illuminated with 175 watt mercury vapor Deluxe White Iamps. The following table gives the number of luminaires for various sign widths. See <u>Roadway and Traffic Design Standard</u> Index 17505 for spacing details and mounting location.

Sign Width	To 10'	To 21' To 32'	To 43'
Luminaires	1	2 3	4

The wind loadings given below were taken from the AASHTO Standard Specification For Structural Supports For Highway Signs, Luminaires and Traffic Signals. The Counties are listed by wind loading for the appropriate sign type.

GROUND SIGNS

- 60 MPH Alachua, Baker, Bay, Bradford, Calhoun, Clay, Columbia, Escambia, Gadsden, Gilchrist, Hamilton, Holmes, Iafayette, Lake, Leon, Liberty, Jackson, Jefferson, Madison, Marion, Okaloosa, Putnam, Santa Rosa, Sumter, Suwannee, Union, Walton, Washington
- 70 MPH Citrus, DeSoto, Dixie, Duval, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Levy, Nassau, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Seminole, St. Johns, Taylor, Wakulla
- 80 MPH Brevard, Charlotte, Collier, Indian River, Lee, Manatee, Martin, Palm Beach, Sarasota, St. Lucie, Volusia
- 90 MPH Broward, Dade, Monroe

OVERHEAD SIGNS

- 80 MPH Baker, Columbia, Gadsden, Hamilton, Holmes, Jackson, Jefferson, Lafayette, Leon, Madison, Suwannee
- 90 MPH Alachua, Bay, Bradford, Calhoun, Charlotte, Citrus, Clay, Collier, DeSoto, Dixie, Duval, Escambia, Flagler, Franklin, Glades, Gilchrist, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Lake, Lee, Levy, Liberty, Manatee, Marion, Nassau, Okaloosa, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Putnam, Santa Rosa, Sarasota, St Johns, Sumter, Taylor, Union, Wakulla, Walton, Washingt
- 100 MPH Brevard, Indian River, Martin, Seminole, St. Lucie, Volusia
- 110 MPH Broward, Dade, Monroe, Palm Beach

7.2.2 Project Coordination

Coordination with other offices and other agencies is a very important aspect of project design. The offices discussed in this section are not intended to be an all inclusive list with which the designer should coordinate, but are those that are typically involved in a signing and marking project.

Roadway Design - The designer of a signing and marking project receives the base sheets for design from the roadway designer, who can also provide any required cross sections. If the signing project is not an active roadway design project, base sheets may be obtained from existing plans or aerial photographs.

<u>Utilities</u> - The District Utilities Engineer provides the coordination between the designer and the various utilities involved in the project. The Utilities Section can also identify potential conflicts with overhead and underground utilities or verify those which have previously been identified. The District Utilities Engineer should be contacted as early in the design phase as possible.

7.3 <u>Lighting</u>

The designer responsible for a highway lighting project should be aware that the design must comply with various standards. In addition to the Department's Standard Specifications, the following standards should be consulted:

An Information Guide for Roadway Lighting, AASHTO — This is the basic guide for highway lighting. It includes information on warranting conditions and design criteria.

Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals, AASHTO -- This specification contains the strength requirements of the poles and bracket arms for the various wind loadings in Florida as well as the frangibility requirements. All Luminaire supports, poles and bracket arms must be in compliance with these specifications.

Roadway and Traffic Design Standards — These indexes are composed of a number of standard drawings or indexes which address specific situations which occur on a large majority of construction projects.

7.3.1 <u>Design Criteria</u>

The AASHTO <u>Guide for Roadway Lighting</u> permits either the illuminance technique or the luminance technique to be used in the design of highway lighting. The luminance technique requires a more complex design process and a knowledge of the reflective characteristics of the pavement surface used. These reflective characteristics change as the pavement ages and with variations in weather conditions. The Department has elected to

use the illuminance technique for lighting design. The design values for light levels given by the AASHTO <u>Guide for Roadway Lighting</u> are maintained values. The light levels given in this criteria have been adjusted and are listed as average initial horizontal footcandles (H.F.C.). This, in effect, sets the maintenance factor to be used in the calculation process to a value of 1.

Mounting height (M.H.) for conventional lighting is the vertical distance from the roadway to the light source, regardless of lateral placement of the pole. Pole setback is the horizontal distance from the edge of the travel lane to the pole.

7.3.2. <u>Conventional Lighting Criteria</u>

o Illumination Level:

Urban Arterials 1.5 average initial H.F.C.
All Other Roadways 1.0 average initial H.F.C.
Weigh Stations 2.0 average initial H.F.C.
Rest Areas 1.0 average initial H.F.C.

Note: These values should be considered as standard, but should be increased if necessary to maintain an acceptable uniformity ratio. The Maximum value should be one and one-half these values. Rest Area sections with pedestrian activity should be increased to 1.5.

o Uniformity:

4:1 or less Avg/Min
10.1 or less Max/Min

o Light Sources:

High Pressure Sodium 400 Watt or Less

o Mounting Height Restrictions:

400 Watt HPS 40 FT. M.H. Min 250 Watt HPS 30 FT. M.H. Min 150 Watt HPS 25 FT. M.H. Min

7.3.3 <u>High Mast Lighting Criteria</u>

o Illumination Level: 0.7-0.9 average initial H.F.C.

over the area

o Uniformity:

3:1 or Less Avg/Min (on the roadway)
10:1 or Less Max/Min (on the roadway)

o Light Sources:

High Pressure Sodium 1000 Watt, 400 Watt

o Mounting Height: 80 to 150 FT. (as design needs

dictate)

7.3.4. Pole Design Criteria

Roadway and Traffic Design Standards Index 700 specify minimum setbacks for conventional lighting. A 20 feet minimum should be used where possible. Poles should be located at least 4 feet behind the face of guard rail and from the face of curbs. High mast lighting poles should be located as far from the travelled way as possible and in no case within 40 feet of the edge of travelled way unless the pole is protected by barrier wall or guardrail. High mast lighting poles should not be located in gore areas within the runout length as defined in the AASHTO Roadside Design Guide. Engineering judgement should be used when locating high mast poles adjacent to bridges and high fills. All conventional height poles shall be frangible unless bridge or barrier wall mounted.

Frangible pole installations shall not be used in areas of heavy pedestrian traffic where the hazard of a falling pole is a greater hazard to others than it is to the motorist. See <u>Roadway and Traffic Design Standards</u> for frangibility requirements.

The installation of lighting in certain locations (e.g. adjacent to residential areas) may require the luminaires to be shielded. This is especially true for high mast poles.

Poles on bridges over open bodies of water or on causeway sections should be considered for dampers. These poles are subject to sustained winds of a critical velocity which may induce vibrations in the pole.

7.3.5 <u>Underdeck Lighting Criteria</u>

- o <u>Iuminaires Pendant hung or pier cap mounted.</u>
- o <u>Light Source</u> High Pressure Sodium 150 Watt or 250 Watt
- o <u>Illumination Level</u> The Light levels should be equal to the adjacent lighting.
- o <u>Location</u> The location of pendant hung luminaires mounted between the bridge beams should not be such that the beams interfere with the spread of light. Normally the luminaires are mounted with 50 percent of the lamp below beams.

Pier cap mounted luminaires should be used when piers are less than 15 feet from the edge of the travel lane. They should be mounted on the bridge piers at 15 feet to 17 feet mounting height.

7.3.6 Foundations Criteria

Foundations for high mast poles are not standard and designs must be provided for each project. The Department has determined that drill shaft footings are the most economical. Drill shaft footings should be used unless special conditions require other types. Drill shafts are typically 4' in diameter as a minimum and have a minimum of 6" of cover on the reinforcing. Boring data should be requested to provide a basis for the design.

7.3.7 Wind Loading Criteria

The wind loadings given below were taken from the AASHTO <u>Standard</u> <u>Specification For Structural Supports For Highway Signs, Luminaires and Traffic Signals</u>. The counties for each wind loading are listed for the appropriate mounting height.

M.H. of 50 Feet and Under

70 MPH Alachua, Baker, Bradford, Calhoun, Clay, Columbia, Gadsden, Gilchrist, Hamilton, Holmes, Lafayette, Lake, Leon, Liberty, Jackson, Jefferson, Madison, Marion, Putnam, Sumter, Suwannee, Taylor, Union, Wakulla, Washington.

80 MPH Bay, Charlotte, Citrus, DeSoto, Dixie, Duval, Escambia, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Lee, Levy, Manatee, Nassau, Okaloosa, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Santa Rosa, Sarasota, Seminole, St. Johns, Volusia, Walton.

90 MPH Brevard, Collier, Indian River, Martin, St. Lucie

100 MPH Broward, Dade, Monroe, Palm Beach

M.H. over 50 Feet

80 MPH Baker, Columbia, Gadsden, Hamilton, Holmes, Jackson, Jefferson, Lafayette, Leon, Madison, Suwannee

Alachua, Bay, Bradford, Calhoun, Charlotte, Citrus, Clay, Collier, DeSoto, Dixie, Duval, Escambia, Flagler, Franklin, Glade, Gilchrist, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Lake, Lee, Levy, Liberty, Manatee, Marion, Nassau, Okaloosa,Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Putnam, Santa Rosa,Sarasota, St. Johns, Sumter, Taylor, Union, Wakulla, Walton, Washington

100 MPH Brevard, Indian River, Martin, Seminole, St. Lucie, Volusia

110 MPH Broward, Dade, Monroe, Palm Beach

7.3.8 Lighting Project Coordination

Coordination with other offices and other agencies is a very important aspect of project design. The offices discussed in this section are not intended to be an all inclusive list with which the designer should coordinate, instead it includes offices that are normally involved in projects.

o <u>Roadway Design</u> - Normally the designer of a lighting project receives the base sheets for lighting design from the roadway designer. The roadway designer can also provide any required cross sections. If the lighting project is not an active roadway design project, base sheets may be obtained from existing plans or aerial photographs. If copies of existing plans are used for base sheets, the drainage maps are usually a good choice.

o <u>Utilities</u> - The District Utilities Engineer provides the coordination between the designer and the various utilities involved in the project. This usually is limited to agreements with the power company for electrical service. The Utilities Section can also identify potential conflicts with overhead and underground utilities or verify those which have previously been identified.

The Utilities Engineer should be contacted as son as pole locations are set and the electrical load has been determined. The designer should indicate a preferred location for the electrical service location.

- o <u>Soils</u> Conventional height poles require the standard base shown in the <u>Roadway and Traffic Design Standards</u>, and only require foundation designs in special cases. High mast poles, on the other hand, require foundation designs for each location. Soil Borings are required for the design of the foundations. The District Soils Engineer should be requested to provide soils data as soon as high mast pole locations are determined.
- o <u>Drainage</u> The location of high mast poles is a minimum of 40 feet from the edge of the travel way. When the locations are established, they should be checked with the Drainage Section to determine if high water level is a problem. High mast poles are often located in the center of interchange loops. These same areas may be used as drainage retention areas. Coordination with the Drainage Section will alleviate this type problem.

Chapter 14-64 of the Department Rules established the maintenance responsibility of lighting systems not on limited access or toll facilities as a local government function.

Normally the District Traffic Operations Engineer in conjunction with the District Utilities Engineer obtain the required maintenance agreements. The designer should coordinate with these offices to ensure that this activity is either underway or scheduled.

Any lighting project, especially high mast, adjacent to or in the vicinity of an airport, may be a potential problem. Any lighting project within 3 miles of an airport should be discussed with the Office of Public Transportation, Aviation Office to determine if a problem exists.

7.4 Traffic Signals

The designer responsible for a traffic signal project should be aware that the design must comply with various standards. In addition to the Department's Standard Specifications, the following standards should be consulted:

Manual on Uniform Traffic Control Devices (MUTCD), FHWA - The MUTCD was adopted by the Department as the Uniform System of traffic control for use on the streets and highways of the State. The action was in compliance with Chapter 316.0745 of the Florida Statutes. The MUTCD is therefore the basic guide for traffic signals. The requirements of the MUTCD must be met, as a minimum, on all roads in the State.

Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals AASHTO - This specification contains the strength requirements of the signal supports for the various wind loadings in the State All signal supports must be in compliance with these specifications.

Roadway and Traffic Design Standards - These standards are composed of a number of standard drawings or indexes which address specific situations which occur on a large majority of constructions.

7.4.1 Design Criteria

The MUTCD, as noted above, has been adopted as the uniform system of traffic control for use on the streets and highways of the state. The MUTCD is the basic guide for traffic signal design; therefore, the traffic signal designer should be familiar with this document. The criteria below supplements the MUTCD.

7.4.2. Certification and Specialty Items

Traffic signal equipment installed in Florida is required to be certified by the Department. The Office of Traffic Engineering in the Central Office is charged with the responsibility of certifying traffic control equipment. The designer of a traffic signal project, if requiring new equipment types or types not normally used, should contact Traffic Engineering in Tallahassee to determine the certification status of the equipment Non-certified equipment cannot be used.

Standard specifications have not been developed for all signal equipment. Some items are project dependent and the development of standard specifications is difficult. Specifications for these items must be developed on a project by project basis and included in the contract as a special provision. Some of these specialty items are included on the Departments approved products list. For these items, detailed specifications are not required. The Office of Traffic Engineering should be consulted on these items.

7.4.3 Stop Line Location

A stop line which is not properly located invites violation by the motorist. The MUTCD specifies the minimum and maximum distances from the signal head to the stop line for adequate visibility. The traffic signal designer must insure that this requirement is met.

Instead of relocating the signal heads, the stop lines at many intersections have been moved from their proper location to comply with these requirements. The tendency for the motorist is not to stop at the new stop line location, but rather to creep beyond the stop line. This could in some cases result in valid calls being dropped, thereby increasing delay and decreasing the overall efficiency of the intersection.

The first step in the design process should be to locate crosswalks and stop lines properly. Then the signal head location should be determined to meet the MUTCD requirements. This may require changing the mounting configuration. A box span, for example, may be required where a diagonal span would normally be installed.

7.4.4 Controller Timings

The development of controller timings is a basic part of traffic signal design. A recent ruling from the Board of Professional Engineers stated that the development of timings is considered engineering and therefore requires the signature and seal of a professional engineer.

All traffic signal designs prepared for or by the Department shall include initial timings of all controllers. This is also true for signals to be included in local systems. If the timings in the plans are not implemented, it will be the responsibility of the agency providing the timings to insure they were prepared under the supervision of a professional engineer.

7.4.5. Left Turn Treatments

The guidelines given below should be followed when determining signal treatments for left turns.

o Single Turn Lane

Restrictive/Permissive Phasing - A five-section cluster should be used for this location. The head should be installed over the lane line between the left turn lane and through lane. The five-section cluster can serve as one of the two indications required for the through traffic. Restrictive Phasing - A separate signal head for the left turn lane with red, yellow and green arrow indications should be positioned over the center of the left turn lane.

o <u>Dual Turn Lanes</u>

Only restrictive phasing should be used. Permissive movements should not be allowed for dual turn lanes. A single three section head with red, yellow, and green arrow indications should be centered over each turn lane. These heads are in addition to the dual indications required for the thru movement.

o Separated Turn and Thru Lanes

Turn lanes that are separated from the thru lanes more than 10 feet by a raised or painted island shall not be operated in the permissive mode.

o Single Lane Approach on Stem of "T"

Two three-section heads are required as minimum. All indications must be circular in this situation.

o Two Approach Lanes on Stem of "T"

Option #1: The approach may display two three-section heads with circular indications on all sections.

Option #2: The approach may display a five-section cluster in conjunction with a three-section head. If the lanes are exclusive left and right turn lanes, then the five-section cluster should be placed over the center of the lane line and the three-section head over the major movement lane. If one of the lanes is a shared left and right lane, then the five-section cluster should be placed over the center of this lane and the three-section head over the center of the other lane.

Option #3: The approach may display two three-section heads for the major movement and a single three-section head for the secondary movement

o Three Approach lanes on Stem of "T"

Option #1: The approach may display two three section heads for the major movement and one for the secondary movement (Exclusive left and right turn lanes).

Option #2: The approach may display a five-section cluster in conjunction with three-section head (exclusive left and right turn lanes). The five-section cluster should be placed over the center of the lane line separating the left turn lane(s) from the right turn lane(s). The three-section head should be placed over the other lane line to provide dual indication for the major movement.

Option #3: When the middle lane is a shared left and right turn lane, then a five-section cluster should be placed over the center of this lane and a three section head placed over each of the other two lanes. Each head must contain green and yellow arrow indications in this situation.

NOTE:

- For all cases, the approach shall display "dual indications". This means that there must be at least two heads with identical indictions on the major approach. For example, if a green arrow is displayed on one head of the major movement or approach then a green arrow must be displayed on the second head.
- The same signal display option should be used throughout an urban area to provide consistency in display to the motorist.

3. The use of advance and/or overhead lane use signs should be used as a supplement to pavement arrows on stems of signalized "T" intersections.

7.4.6. Signal Preemption

The engineer responsible for the design of a traffic signal project should, as a matter of routine, check each intersection to determine if the need for signal preemption is present.

Intersections located within 200 feet of moveable span bridges or railroad crossings should be considered. Those located at distances greater than 200 feet should be considered if the queues frequently extend to the moveable span or crossing.

Intersections near fire stations require individual study. This is necessary to determine the interaction between the fire station vehicles and the intersection operation. This information must be known before the preemption sequence can be developed.

7.4.7 Intersection Design - Lane Configuration

The engineer responsible for the traffic signal design may be asked to verify the number and configuration of traffic lanes required for an intersection to function properly when signalized.

The results are dependent upon the traffic volumes used in the analysis. The traffic used for this calculation shall be the design hourly volume based on the 30th highest hour (k factor) and not a peak to daily (P/D) ration based on a 24 hour count. The k factor volumes account for traffic variations through the year, and, in most case, are higher than P/D volumes.

The K, D, and T factors covert the two-way AADT volumes to a one-way Design Hour Volume (DHV). This is appropriate for the total approach movements. The AM and PM peak turning movement counts on each approach should be addressed individually. Current turning movement counts should be taken to determine the percentage of turns for each approach. These percentages should then be applied to the DHV for each approach volume to determine the turning volumes which should be used for the turn lane design calculations. These values should be compared to the movement counts supplied by Planning and the greater of the two values used for the design of turn lanes. The District Planning Office should be contacted to determine if recent counts are available and also if any use changes are planned which would require adjustments to the turn percentages found in the current counts.

Storage lanes for left turns can affect the capacity and safety of intersections. The storage length of a left turn lane is a critical design element. The queue of left turn vehicles in a storage lane of inadequate length may extend into the through lanes. The result is loss of capacity for the through lanes. The queue of through vehicles may also extend beyond the entrance of a short left turn storage lane blocking access to the storage lane. Either case results in a less efficient operation of the intersection and may cause last minute lane changes, thereby increasing the possibility of conflicts.

The important factors which determine the length needed for a left turn storage lane are:

- o The design year volume for the peak hour (see discussion above).
- o An estimate for the number of cycles per hour.

 NOTE: If the cycle length doubles, the length of the storage for the same traffic also doubles.
- o The type of signal phasing and timing which will control the left turn lanes.

There are several techniques used to determine necessary storage length. The following are suggested guidelines for left turn lanes.

- o Where protected left turn phasing is provided, an exclusive turn lane should be provided.
- o Left turn lanes should be provided when turn volumes exceed 100 vph and may be considered for lesser volumes if space permits.
- o For signalized intersections, the following formula may be used, assuming an average vehicle length of 25 feet.

$$L = (20) (DHV) (25)$$

Where

L = design length for left turn storage in feet

DHV = left turn volume during design peak hour, in vph

N = number of cycles per hour for peak hour, use N = 30 as default.

- o Where left turn volumes exceed 300 vph, a double left turn should be considered.
- o When right of way has already been purchased, and the designer has to choose between a long wide grass median or a long left turn lane, the storage length for the left turn should be as long as practical without hindering other access.

Right turn lanes are provided for many of the same reasons as left turn lanes. Right turns, are, however, generally made more efficiently than left turns. Right turn storage lanes should be considered when right turn volume exceeds 300 vph and the adjacent through volume also exceeds 300 vphpl.

7.4.8. Signal Loops

Traffic signal loops are detailed in <u>Roadway and Traffic Design</u>
<u>Standard</u> 17781. These loops are standard and will be appropriate for most locations.

The traffic signals for each intersection should be individually designed. The requirement for type and placement of loops is a part of this design. The above standard allows for some variation in size and placement of the standard loops. These modifications are intended to be used only when required by the design of a particular location.

7.4.9 Wind Loading

The wind loadings given below were taken from the <u>Standard Specifications For Structural Supports For Highway Signs, Luminaires and Traffic Signals</u>, AASHTO. The Counties for each wind loading as as listed.

- 70 MPH Alachua, Baker, Bradford, Calhoun, Clay, Columbia, Gadsden, Gilchrist, Hamilton, Holmes, Lafayette, Lake, Leon, Liberty, Jackson, Jefferson, Madison, Marion, Putnam, Sumter, Suwannee, Taylor, Union, Wakulla, Washington
- 80 MPH Bay, Charlotte, Citrus, DeSoto, Dixie, Duval, Escambia, Flagler, Franklin, Glades, Gulf, Hardee, Hendry, Hernando, Highlands, Hillsborough, Lee, Levy, Manatee, Nassau, Okaloosa, Okeechobee, Orange, Osceola, Pasco, Pinellas, Polk, Santa Rosa, Sarasota, Seminole, St. Johns, Volusia, Walton

90 MPH Brevard, Collier, Indian River, Martin, St. Lucie

100 MPH Broward, Dade, Monroe, Palm Beach

Chapter 8

Bicycles and Pedestrian Facilities

8.1 General

Pedestrians and bicycle riders may often wish to travel between the same locations as other vehicular traffic. When such a situation arises, the designer should consider the effects on the safety and operation of the roadway system. A special effort should be made to provide the greatest degree of safety within the economic constraints that must always be considered.

Additional special bicycle and pedestrian facilities should be provided where the use of travel lane or wide paved shoulders is deemed unacceptable or unsafe for the pedestrian or bicycle rider. It is the Department's policy to provide for bicycle and pedestrian needs within one mile of urban streets.

Pedestrian facilities include sidewalks, crosswalks, traffic control features, special walkways, curb cuts and ramps for the handicapped.

8.2 Sidewalks

The design of sidewalks is affected by pedestrian volume, traffic volume, average pedestrian age, walking rate, required level of service, location, etc. The Florida Green Book and the AASHTO Green Book present the various factors that influence the design of sidewalks and other pedestrian facilities.

All urban projects that have sufficient right-of-way to provide adequate lane, median, sidewalk and border widths should be thoroughly evaluated to provide a reasonably safe and cost effective design. A distance of as little as 2' between the back of sidewalk and right-of-way can be effective in minimizing construction easements by using a 1' berm and 2:1 slope or a 4:1 slope between the sidewalk and right-of-way. Differences in elevation up to 6" can be accommodated in this manner. Caution must be exercised in using this treatment at connections to driveways since this distance and slope will not assure adequate vehicular connections (see Index 515, sheet 2).

Sidewalks should be constructed in conjunction with all new construction, major reconstruction and lane addition curb and gutter projects. As a general practice, sidewalks should be constructed along both sides of arterial streets that are not provided with shoulders, even though pedestrian traffic may be light. Exceptions may be made to the construction of sidewalks on both sides of the street when the roadway parallels a railroad or drainage canal and pedestrians would not be expected and in some cases on bridges. If sidewalks are constructed on the approaches to bridges, they should be continued across the structure.

The standard width for sidewalks is 5 feet when separated from the curb by a buffer strip. A 4 feet minimum may be used in areas with few pedestrians. When sidewalks are constructed adjacent to the curb, the minimum width should be 6 feet. Additional width of sidewalk may be provided when traffic volumes, truck volumes, pedestrian volumes or vehicular speeds are high. Proximity to schools should also be considered for additional width. Department's Bicycle Facilities Planning and Design Manual states that it is important to recognize that the development of wide sidewalks does not necessarily add to the safety of sidewalk bicycle Wide sidewalks encourage higher-speed bicycle use and can increase the potential for conflicts with motor vehicles at intersections, as well as with pedestrians and fixed objects. the AASHTO Guide for Bicycle Facilities and the "Florida Green Book" state that bicycle riding on sidewalks can be expected in residential areas with young children who are too inexperienced to ride in the This type of bicycle use is generally accepted, but it is not appropriate to sign a sidewalk as a bicycle path. appropriate to provide both wide curb lanes for the benefit of the experienced bicyclist and wide sidewalks that may encourage high speed bicycle use. Five foot sidewalks are to be provided unless greater widths are justified for pedestrian use. A method for determining pedestrian facility needs is given in the Highway Capacity Manual.

8.3 Handicap Access

Pedestrian facilities such as walkways and sidewalks must be designed to accommodate the physically handicapped persons whose mobility is dependent on wheelchairs and other devices. Street intersections with steep-faced curbs are one form of obstacle that can be alleviated while still providing reasonable mobility for the handicapped, and without sacrificing the mobility of others.

In areas with sidewalks the ramps must be incorporated in the sidewalks. The basic ramp type and design application depends on the geometrical characteristics of the intersection. Standard Index No. 304 of the "Florida Roadway and Traffic Design Standards" sets forth the requirements and standards of curb cut ramps for use in Florida.

All Department facilities (roadway, parking lots, rest areas, buildings, etc.) must be designed in compliance with current handicap Florida statutes, rules and regulations. Design must also meet minimum requirements of the American National Standards Institute (ANSI 117.1) for accommodation of the handicapped.

8.4 Bicycle Facilities

The bicycle has become an important element for consideration in the highway design process. In recent years the emphasis in bicycle system planning has changed from attempts to provide completely separate facilities for bicyclists to the growing recognition that bicyclists are legitimate users of the roadway. Recent studies have shown that in many cases shared roadway facilities afford greater safety for the bicyclists than sidewalk facilities because of the increased visibility and maneuverability. Certain measures such as

- o Paved shoulders
- o Wide outside traffic lanes if no shoulders
- o Bicycle-safe drainage grates
- o Adjusting manhole covers to grade
- o Maintaining a smooth, clean riding surface

can considerably enhance a route's safety and capacity for bicycle traffic without impacting the service for motor vehicles on the roadway.

Planning and design consideration for bicycle facilities are given in the AASHTO Green Book and the AASHTO <u>Guide for Development of New</u> Bicycle Facilities.

The Department's current policy is to give special emphasis to the needs of bicyclists in and within one mile of urban areas. This policy will generally provide for the construction of wide curb lanes, bicycle lanes or paved shoulders for the needs of bicyclists in conjunction with other planned roadway improvements. The lack of adequate right of way and the cost associated with its acquisition in built up areas may not allow provision of the additional width for bicyclists on all projects. Roadway improvements in the smaller urban areas (5,000 to 50,000 population) and the more rural areas

should be reviewed on a case-by-case basis depending on anticipated bicycle travel and the need for wider pavement or paved shoulders based on other safety and operational benefits. Anticipated bicycle travel should be considered of sufficient volume when the roadway section is identified for bicycle improvements in the Transportation Improvement Program, the State Transportation Plan (Bicycle Elements) or other approved Community Comprehensive Bicycle Transportation Plans.

The Department's <u>Bicycle Facilities Plannining and Design Manual</u> should be referenced for the design of bicycle facilities. Chapter 2 of this volume discusses shoulder width criteria when bicycle use is anticipated.

Chapter 9

Landscaping

9.1 General

The complete highway is one wherein the elements of design, construction and maintenance have been integrated to provide a facility that possesses utility, safety, beauty and economy. The highway should be considered as an element of the total environment, not apart from it or in conflict with it. All highway-oriented disciplines should collaborate at each stage of highway corridor selection, location, and design to obtain the maximum beneficial potential of the highway, its roadsides, and its environment.

The highway roadside is an integral unit of a total highway facility. The term "roadside" generally refers to the area between the outer edge of the roadway and the right-of-way boundary. It could include extensive areas in a wide median of a divided highway. Roadside development is the treatment given to the roadside to conserve, enhance, and effectively display the natural beauty of the landscape through which the highway passes. It should provide safety, utility, economy, and highway-related recreation facilities by means of proper location, design, construction, and maintenance.

Because the potential for conflict between the highway and environmental values is greatest in urban areas, it is essential that special attention be given to the multiple use-joint development possibilities in areas over, under, and adjacent to the highway to ensure that land and space above the highway provides the greatest benefit to the greatest number.

Landscape development should be in keeping with the character of the highway and its environment. Programs include the following general areas of improvement:

- o Preservation of existing vegetation.
- o Transplanting of existing vegetation where feasible.
- o Planting of new vegetation.
- o Selective clearing and thinning
- o Regeneration of natural plant species and material.

The objectives in planting or the retention and preservation of natural growth on roadsides are closely related. In essence, they are to provide vegetation that will be an aid to esthetics and safety, aid in lowering construction and maintenance costs, and create interest, usefulness, and beauty for the pleasure and satisfaction of the traveling public.

Landscaping of urban highways and streets assumes additional importance by mitigating many of the nuisances associated with urban traffic. Landscaping should be arranged to permit sufficiently wide, clear, and safe pedestrian walkways. Combinations of turf, shrubs, and trees are desirable in border areas along the roadway. However, care should be exercised to ensure that requirements for sight distances and clearance to obstructions are observed, especially at intersections.

Further information concerning landscape development and erosion control is presented in AASHTO's <u>A Guide for Highway Landscape and Environmental Design</u>. The Department's <u>Roadway and Traffic Design Standards Booklet</u> sets forth specific criteria and standards for erosion control and roadside landscaping. The Department's <u>Landscaping Guidelines</u> (document No. 650-050-001b) provides the general criteria for use in the development of landscaping plans for roadway projects.

Chapter 10

WORK ZONE TRAFFIC CONTROL

10.1 <u>General</u>

The need to improve the capacity of, and to rehabilitate Florida's highways, has greatly increased the frequency of highway construction taking place immediately adjacent to or under traffic. The travelling public, as well as construction and inspection personnel, are exposed to conflicts that may become hazardous. In addition to the safety issue, the potential delays to the public, as traffic is interrupted by construction, can be significant. As a result, the Department places a great deal of emphasis upon ensuring that traffic can be accommodated through construction zones with minimum delay and exposure to unsafe conditions.

10.2 References

The following references contain the basic criteria for work zone traffic control in Florida:

The <u>Manual on Uniform Traffic Control Devices for Streets and Highways</u>, (MUTCD), Federal Highway Administration. Part VI of the MUTCD deals specifically with work zone traffic control. Other parts of the MUTCD may also be useful in designing a traffic control plan.

<u>Traffic Control Devices Handbook</u>, (TCDH) Part VI, Federal Highway Administration.

Policy on Geometric Design of Highways and Streets, AASHTO

Roadside Design Guide, AASHIO, Chapter 9.

Roadway and Traffic Design Standard Index Series 600, 415 and 700.

Federal-aid Highway Program Manual (FHPM) 6-4-2-12.

10.3 Comprehensive Work Zone Traffic Control Planning

Consideration of traffic control must begin at the Project Development and Environmental (PD&E) study stage. Impacts on traffic, traffic handling options, constructability, and design features and constraints, as they affect traffic, are to be evaluated for each alternate alignment studied. The preliminary and final engineering reports must specifically address work zone traffic control.

Traffic control considerations must begin in the early stages of design, using the work zone traffic control material from the PD&E study as the basis. As the design progresses, the following should be considered:

<u>Design features and constraints.</u> Length of the project, lane configuration, and grade differentials between existing and proposed, interchanges and intersections, pavement materials, storm sewers, utilities and bridge features are some of the design element decisions that might be influenced by work zone traffic control considerations.

<u>Contract specifications</u>. Provisions such as time restrictions on construction activities; incentive-disincentive clauses; daily, weekly and seasonal restrictions and special materials may be necessary. Public relations activities such as media releases, television and radio spots, hand bills, and highway advisory radio may be specified.

Other actions. Actions may need to be taken by the Department prior to or during construction that may not be a contract requirement. Examples are dealing with the media and local businesses, provisions for mass transit options to commuters, service patrols, improvements to alternate routes, coordination with other projects and maintenance activities, and special inspection requirements.

<u>Public input.</u> On very large and complicated projects, it may be necessary to involve the public through informal public meetings to be held early in the design of a project. Close coordination with city and county officials may be necessary. Citizen and business advisory committees may be established as sources of input.

<u>Utility work.</u> If contract utility work is anticipated in conjunction with or during the highway construction, the Traffic Control Plan (TCP) must account for and adequately protect all work activities. The phasing of construction activities must be compatible with the utility work. Utilities, whose work affects traffic, are required to have a TCP by FHWA's FHPM 6-6-3-2. This requires early and effective coordination with utilities.

10.4 Traffic Control Plans (TCP)

A TCP is a set of specific plan sheets, references to standard (typical) layouts, and/or notes on roadway plans describing how traffic will be controlled through a work zone. All projects and work on highways, roads and streets shall have a traffic control plan, as required by Florida Statute and Federal regulations. All work shall be executed under the established plan and Department approved procedures. The TCP is the result of considerations and investigations made in the development of a comprehensive plan for accommodating traffic through the construction zone. These considerations include the design itself, contract specifications, and plan sheets.

TCP sheets detail the proper delineation of traffic through the work zone during all construction phases. The complexity of the TCP varies with the complexity of the traffic problems associated with a project. Many situations can be covered adequately with references to specific sections from the <u>Manual on Uniform Traffic Control Devices</u> (MUTCD), the <u>Traffic Control Devices Handbook</u> (TCDH), or <u>Roadway and Traffic Design Standard Series 600</u>. Specific TCP sheets shall be required in the plans set whenever project conditions are not specifically addressed in a typical layout from the manuals noted above. This is usually the case for complex projects, and therefore references to Standard Indexes, as well as specific TCP sheets, will both likely be necessary.

A quality traffic control plan should include the following information for the mainline and any affected cross roads, side streets, and ramps:

- (1) the location of all advance warming signs and lighting units
- (2) temporary pavement markings, (including RPM's) for detours, transitions or other special situations
- (3) location of temporary barriers and attenuators

- (4) temporary drainage devices
- (5) channelizing devices at special locations
- (6) locations for special devices such as variable message signs (VMS), arrow boards, and temporary signals
- (7) VMS messages for each phase
- (8) signal timing for each phase, including method of temporary actuation if needed. (Check with Traffic Operations Engineer)
- (9) location and geometry for transitions and detours
- (10) typical sections for each phase of work on all projects, except simple resurfacing projects, in order to show lane widths, offsets, barrier locations and other features influencing traffic control
- (11) the proposed regulatory speed(s) for each phase
- (12) reference to appropriate Standard Indexes or MUTCD drawings whenever applicable.
- (13) appropriate quantities, pay items and pay item notes
- (14) resolve any conflicts between permanent signing and markings and work zone signing and markings

Chapter 19, Volume II, <u>Plans Preparation and Assembly</u>, explains the required information for specific TCP sheets.

Traffic control is a joint responsibility of design (both roadway and bridge), construction and traffic operations personnel. Coordination is necessary by all three parties in the development of TCPs. Both traffic operations and construction personnel must routinely review TCPs in the early stages of development (30 - 60% plans) to ensure that the plan is sound and constructible and bid items are complete and quantities reasonable.

Consideration must also be given to adjoining, intersecting or sequential work zones. This can be a particular problem with maintenance operations, bridge or roadway projects under different contracts, operations of other jurisdictions or utilities. When such work must take place, the operations must be coordinated and taken into account in the TCP so that the motorist encounters one, consistently designed, work zone.

TCP's for project designs "on the shelf" must be updated prior to contract letting.

10.5 TCP Development

The following step-by-step process should be followed by designers when preparing traffic control plans.

STEP #1 Understand the Project

- o Field reviews by designers should be required
- o Review the scope
- o Examine the plans (30% 60%)
- o Look at plan-profiles and cross-sections for general understanding.
- o Review PD&E study for any constraints
- o Consider bicycle/pedestrian needs during construction
- o For complex projects consider developing a TCP study and possible TSM strategies such as public awareness campaigns, alternate route improvements, service patrols, etc.

STEP #2 Develop Project Specific Objectives

What are your objectives? Examples might be:

- o close road if adequate detour exists
- o maintaining 2-way traffic at all times.
- o maintaining existing roadway capacity during peaks
- o maintaining business/resident access
- o provide bike/pedestrian access
- o minimize wetland impacts

STEP #3 Develop a Construction Phasing Concept

- o Examine existing facility versus what is to be built.

 This is a major task on jobs other than resurfacing.
- o Coordinate with bridge designers

- o Color or mark the plan and profile sheets to show existing roadway versus new construction. Then, check station by station, the plan sheet against cross-section sheets. Make notes on plan sheets as to dropoffs or other problems. Use profile grade lines or centerlines for reference points.
- o List out tasks to be completed, such as:

construct new WB Roadway construct new EB Roadway construct frontage roads construct bridge/flyover

NOTE: The designer may need input from construction personnel or even contractors representatives in determining construction phases

o Make notes on plan sheets or notepad as to "decisions" that you make along the way.

STEP_#4 Brainstorm TCP Alternatives

Develop some rough alternatives, considering what could be used to accomplish the work, such as constructing temporary pavement and/or temporary detours, using auxiliary lanes, placing 2-way traffic on one side of divided facility, using detour routes, etc. Also, southside as opposed to northside on an east-west roadway. Don't worry that an alternate doesn't meet <u>all</u> objectives.

Designers should check condition of any proposed detour routes. If off state system, may need agreement with locals.

<u>STEP #5</u> Examine/Analyze Alternatives Which Meet Objectives (for each phase).

Next, consider how you could achieve the proposed alternatives and meet the stated objectives.

Examine pros and cons of various alternatives.

Consider how much work and expense is involved for each alternative.

Consider detour/transition locations, signal operations during construction, how to handle alternate modes of transportation-buses, bikes, pedestrians, service vehicles, etc.

STEP #6 Develop Detailed TCP

Select the most feasible alternative for each phase. Add details such as:

- o detour/transition geometrics and locations.
- o if lane closures are needed, use the lane closure technique discussed in 10.15.7 to determine time frame for closures.
- o advanced signing scheme and locations, revisions needed to existing signs - including guide signs, and proposed signs for all work activities - lane closures, detours, etc., on mainline, sideroads, x-roads and ramps
- need for portable traffic signals, variable message signs, and barriers,
- o how existing operations will be maintained side streets, businesses, residents, bikes, pedestrians, buses - bus stops, etc.,
- o revisions to signal phasing and/or timing during each TCP phase,
- o regulatory speed desired for each phase,
- o all pay items and quantities needed for TCP.

- o how existing Auxiliary lanes will be used and any restriction necessary during construction.
- o typical sections for each phase
- o outline key strategies to be used
 - (a) service patrol
 - (b) police
 - (c) public service announcements
 - (d) Highway Advisory Radio
 - (e) night work
- o need for Alternate route improvements

10.6 Coordination

Work zone traffic control can be a complex undertaking that requires the coordination of a number of agencies and other interested parties. Planning and coordination must begin early in a project design.

Traffic control plans shall be developed and reviewed with assistance from appropriate offices such as construction, maintenance traffic operations, FHWA, community awareness teams, freeway coordinator management teams and local agencies, business and interested groups. Initial reviews should be made by construction and traffic operations no later than the 60% plans stage with subsequent reviews of 90% plans. Input from local agencies should be obtained early in the process, such as during the PD&E study and the 30% plans stage.

Adjoining work zones may not have sufficient spacing for standard placement of signs and other traffic control devices within their traffic control zones. These situations can occur when separate contracts adjoin each other (separate bridge and roadway contracts are a typical example), utility work performed separately from roadway work or when maintenance activities are performed adjacent to a construction project. Where such restraints or conflicts occur, or are likely to occur, the designer should try to resolve the conflicts in order to prevent misunderstanding on the part of the travelling public:

10.6.1 FHWA Review

The type and format of TCPs on Federal-aid projects must be coordinated with FHWA early during design. TCPs for federal aid projects are to be submitted to FHWA for review not later than at the 60% plan stage.

10.6.2 TCP Revisions After Letting

Any TCP changes by the contractor or the Department that significantly alter the original intent should be processed as plan revisions and approval obtained in advance from the FHWA for These revisions must be prepared under federal-aid projects. the supervision of a registered professional engineer and the revisions must be signed and sealed. However, it should be noted that various phases of construction may be accomplished concurrently, or the order of work (phasing) revised as approved by the engineer, without submitting signed and sealed revisions if such changes do not cause conflicts or add new traffic control requirements. The contractor may also choose to use other methods which are considered to be standard engineering practices, such as the Standard Indexes, without submitting a signed and sealed revision. Any change to the engineering of the TCP which affects geometry, drainage, or traffic flow must be signed and sealed.

10.7 Work Zone Traffic Control Training

10.7.1 Background

Work zone traffic control is an important function affecting the safety of the traveling public, contractor personnel and equipment, and department employees. Every reasonable effort should be made to eliminate or reduce involvement in accidents within work zones. Proper traffic control training is vital to achieving this objective.

The Department's Maintenance of Traffic Committee consists of representatives from the Roadway Design, Construction, Safety, Maintenance, Traffic Engineering, and value Engineering Offices. Its purpose is to develop, review or revise procedures, standards and specifications regarding work zone traffic control to maximize efficiency and enhance safety of motorists, pedestrians, and workers in these zones.

10.7.2 Training Requirements

The Department's Maintenance of Traffic Committee has prescribe work zone traffic control training requirements for Department employees and shall furnish training course information and requirements to each District's Human Resource Development Manager.

Every employee whose activities affect maintenance and construction work zone safety, from upper-level management through construction and maintenance field personnel, shall complete appropriate training as prescribed above and as required by Department procedure number 750-030-006-a.

District Design, Construction, and Maintenance Engineers shall ensure that only Department employees who have been certified under provisions of this procedure shall be responsible for traffic control plan design, implementation, or inspection and to supervise the design, selection, placement, or maintenance of traffic control schemes and devices in work zones.

10.8 Traffic Control Devices

Traffic control devices/methods that are available for use include:

- Signs (warning, regulatory and guide)
- Lighting units (arrow panels, barricade and sign lights, illumination devices, temporary signals and variable message signs)
- Channelizing devices (cones, tubular markers, plastic drums, vertical panels, Types I, II and III barricades)
- Markings (pavement markings, raised pavement markings, delineators, and removal of conflicting markings)
- Safety appurtenances (portable concrete barriers, guardrail and crash cushions) See AASHTO Roadside Design Guide (Chapter 9).
- Flaggers
- Police
- Guardrail attached to barrels for work zones \leq 45 mph See AASHTO Roadside Design Guide.

The MUTCD contains detailed instructions on the use of traffic control devices. Special design considerations applicable to Florida are discussed in the following sections.

10.9 Signs

10.9.1 Advance Warning Signs

The TCP should identify the advance construction warning signs, including legends and location., These include signs such as "Road Construction Ahead", and "Road Construction One Mile". The TCP should provide the advanced warning signs, legends and locations for all proposed operations which require signing. These include detours, lane closures, and flagging operations. The sequence for advance signing should be from general to more specific. As an example: Road Construction Ahead (general), Left Lane Closed Ahead (more specific), Merge Right (specific).

10.9.2 Length of Construction Sign

The length of construction sign (G20-1) bearing the legend "ROAD CONSTRUCTION NEXT ____ MILES" is required for all projects of more than 2 miles in length. The sign shall be located at begin construction points. Mileages should be to nearest 0.1 mile.

10.9.3 Sign Covering

Signs (temporary and permanent) that warn of conditions shall be covered or removed when the condition is not present, such as might occur in work zones with daytime only operations. Traffic control signs that require covering shall be fully covered with a durable opaque sheet materials. Plastic film and woven fabrics including burlap will not be permitted. Covering of only the legend or symbol will not be permitted. Reflective coverings will not be permitted. Hinged signs designed to cover when folded and sign blanks are permitted. Signs to be covered or removed will be identified in the TCP, along with acceptable procedures.

10.9.4 Existing Signs

Existing regulatory signs that conflict with the TCP shall be removed. Existing guide signs should be modified as necessary. It is good practice to revise existing guide signs by using black on orange panels to show changes made necessary by the construction operations.

If permanent guide signs are to be removed during construction, provisions should be made for temporary guide signing. The temporary sign should be black on orange with the legend designed in accordance with MUTCD requirements for permanent guide signing whenever possible.

10.9.5 Construction Identification Signs

Construction identification signs shall be shown on the TCP for projects in excess of \$2 million dollars. The contract dollar amount on the sign shall be rounded to the nearest \$1,000. The sign shall include the project objective and the completion year. Project objective shall be one-line descriptions such as "IANE ADDITIONS," "ROADWAY RESURFACING," or "BRIDGE REPLACEMENT." For further details on construction I.D. signs, refer to the Indexes and to Department procedure 750-010-015-a.

10.10 Lighting Units

10.10.1 Warning Lights

Warning lights shall be in accordance with Section 6E-5 of the MUTCD except for the limitations below.

Flashing

Type A low intensity flashing warning lights should be mounted on barricades, drums, or vertical panels and are intended to continually warn drivers that they are approaching or proceeding in a hazardous area. Flashing lights shall not be used to delineate the intended path of travel, and shall not be placed with spacings that will form a continuous line to the driver's eye. The Type A light will be used to warn of isolated obstructions that are located adjacent to or in the intended travelway, and, to mark the approach to closed or detoured travel lanes. Type A lights shall not be used in conjunction with the first and second advance warning signs.

Type B high intensity flashing warning lights shall be mounted on the first advanced warning sign and on the first and second advanced warning sign where two or more signs are used. This applies to all approaches to any work zone. The first and second advance warning signs shall be supplemented with an 18 inch by 18 inch warning flag.

Steady-Burn

Type C steady-burn lights are to be mounted on barricades, drums, concrete barrier walls or vertical panels and used in combination with those devices to delineate the travel way on lane closures, lane changes, detour curves and other similar conditions. Steady-burn lights are intended to be placed in a line to delineate the traveled way through the work zone and around obstructions in the transition, buffer, work and termination areas of the traffic control zone. Steady burn lights often serve the dual purpose of delineation and hazard warning.

10.10.2 Advance Warning Arrow Panels

Arrow panels shall be used to supplement other devices for all lane closures on high-speed and high-traffic density multi-lane roadways. The use of arrow panels should be considered for all other multi-lane closures. These devices are also useful for short-term operations, such as during work zone installation and removal. Arrow panels can also be used to provide additional advance warning and directional information where traffic must be shifted laterally along the roadway, as well as provide positive guidance about a roadway path diversion that might not otherwise be expected. Refer to MUTCD 6E-7 for further information.

Arrow panel locations shall be shown on the TCP, along with any necessary notes concerning the use of this device.

10.10.3 <u>Variable Message Signs</u>

Variable message signs may be used to supplement a traffic control zone. As a supplemental device, it cannot be used to replace any required sign or other device. These devices can be useful in providing information to the motorist about construction schedules, alternate routes, expected delays, and detours. Variable message signs should be considered for use in complex, high-density work zones. Messages must be simple, with a minimum number of words and lines and should require no more than three displays of no more than three lines each. The TCP should include the location and messages to be displayed.

The message displayed should be visible and legible to the motorist at a minimum distance of 900 feet on approach to the signs. All messages should be cycled so that two message cycles are displayed to a driver while approaching the sign from 900 feet at 55 mph.

The VMS units may be used:

- To supplement conventional traffic control devices in construction work areas and should be placed approximately 500' to 800' in advance of potential traffic problems, or
- 1-2 miles in advance of complex traffic control schemes which require new and/or unusual traffic patterns for the motorists.

Message Selection

Programmed messages should provide appropriate messages for the conditions likely to be encountered. The following items must be carefully considered in the development of a message:

- (1) Message elements not necessarily in order
 - (a) problem statement (where?)
 - (b) effect statement (what?)
 - (c) attention statement (who?)
 - (d) action statement (do?)

(2) Message format

- (a) will vary depending on content
- (b) "where" or "what" will generally lead
- (c) "who" and "do" follow in that order
- (d) "who" often understood from "where"

(3) Message length

- (a) limited by reading time (e.g., 900' at 55 mph provides 11 seconds of reading time)
- (b) limited by driver's processing capability
- (c) 4 word maximum, one part message
- (d) 8 word desirable max if two or three sequence message is used
- (e) shorter messages desirable to permit multiple readings
- (f) if two or three sequence messages is programmed, allow for off-time between messages.

(4) Display format

- (a) discrete with entire message displayed at once is most desirable
- (b) sequential is OK, 2 part preferred, 3 part maximum
- (c) run-on moving displays prohibited
- (d) one abbreviation per panel display desirable, two abbreviations are maximum. Route designation is considered as one abbreviation and one word.

EXAMPLE:

	Display 1	Display 2	Display 3
(a)	RAMP TO	USE	
	SR 26 E	ALT	
	CLOSED	ROUTE	
(b)	I - 95	FOLLOW	
	ROAD	DETOUR	
	CLOSED	ROUTE	
(c)	I-495 A T	2 R I G H T	EXPECT
	E X I T 30	LANES	DELAYS
		CLOSED	

Typical Conditions

Consistent with the factors described above, VMS messages should be considered under the following conditions:

- (1) Road closures
- (2) Ramp closures
- (3) Delays one hour or longer created by
 - (a) congestion
 - (b) accidents
 - (c) lane closures
 - (d) two-way traffic on divided highway
 - (e) multiple lane closures
 - (f) unexpected shifts in alignment

10.10.4 Traffic Signals

Frequently portable or temporary traffic signals will be a preferred alternative to a flagger Also, existing signal operations may need to be revised to accommodate the construction operations. The TCP should identify the specific alterations (physical location and timing) necessary for existing signals and the location and timing of portable signals. Signal displays and location must meet MUTCD requirements.

10.11 Channelizing Devices

10.11.1 Type III Barricades

12 foot Type III barricades have proven too cumbersome for work site applications. Two 6 foot Type III barricades should be used to block off or close a roadway. Whenever two 6 foot barricades are used together, only one warning light is required on the 12 foot of barricade.

10.11.2 Separation Devices

Placing two lane two-way operations (traffic) on one roadway of a normally divided highway (TIIWO) should be a last resort and should be done with special care. An analysis similar to that presented on pp 6-57 thru 6-61 of the TCDH, should be performed and documented when contemplating the need for TIIWO

"Separation" is defined in FHPM 6-4-2-12 and requires the use of devices such as those presented on pp 6-57 and 58 of the TCDH. These include temporary barrier, drums, cones, tubular markers, or vertical panels. Separation devices are required for TLTWO unless: (1) it is used on an urban street where speeds are low; or (2) drivers entering the TLTWO can see the transition back to normal one-way operation on each roadway. Striping, RPM's and signing alone, are not allowed when separation devices are required.

Experience has also shown that stand alone tubular markers for long term operations are difficult to maintain and therefore their effectiveness is questionable. The use of a temporary asphalt separator in conjunction with the tubular markers has been very effective (see Index 614).

10.12 Pavement Markings

10.12.1 Removing Pavement Markings

Existing pavement markings that conflict with temporary work zone traffic patterns must be obliterated where operations will exceed one work period. Painting over existing pavement markings is not permitted.

10.12.2 Reflectorized Raised Pavement Marker (RPM)

Temporary RPMs are required on the lane lines of all transitions, crossovers and detours and to delineate temporary gore areas within the work zone. The spacing shall be 40 feet on tangent sections and 20 feet on transitions, curves and crossovers.

10.12.3 Temporary Pavement Markings

Section 102-3.3 of the Standard Specifications, provides a pay item for "Non-Removable Pavement Markings" (Item #102-92-3 for skip and 102-92-4 for solid) which includes Non-Removable or Removable Pavement Markings depending upon the traffic pattern changes as noted in the Specifications. There is also a separate pay item for Removable Pavement Markings (Item #102-91-1 for skip and 102-91-2 for solid) that can be used in addition to the Temporary Pavement Marking Pay Item at the designers option. If both non-removable and removable pavement markings are anticipated, then both pay items should be included in the plans. This will make contract administration and enforcement much easier for construction personnel.

The designer should also consider using a miscellaneous asphalt pavement pay item for covering unneeded markings, especially in areas such as detours or crossovers.

10.13 <u>Safety Appurtenances for Work Zones</u>

10.13.1 Traffic Barriers

Work zone traffic barriers are designed either as permanent barriers or as temporary barriers that can be easily relocated. They have four specific functions: to protect traffic from entering work areas, such as excavations or material storage sites; to provide positive protection for workers; to separate two-way traffic; and to protect construction such as false work for bridges and other exposed objects. The designer should anticipate when and where barriers will be needed and include this information and the quantities on the plans.

10.13.2 Portable Concrete Safety Shape (Temporary Barrier Walls)

Portable concrete safety shape barriers, also known as portable concrete barriers (PCB's), are widely used in work zones to protect motorists as well as workers. However, improper use of these barriers can provide a "false sense of security" for both the motorist and the worker. Therefore, care must be taken in their design, installation and maintenance. Installation instructions and flare rates are given in Standard Index 415.

To perform properly and redirect vehicles, the PCB system must be capable of withstanding severe impacts. The PCB's weakest point is its connector which includes the physical connection and mating faces of adjoining barriers.

Acceptable connections are noted on Standard Index 415.

10.13.3 End Treatments

The desirable treatments for exposed ends of barriers are:

- connecting to an existing barrier (smooth, structural connections are required Refer to Index 410 and 415) or
- attaching a crashworthy terminal such as a crash cushion or
- flaring away to the edge of the clear zone (See Chap. 6)

10.13.4 Modifications of Existing Barriers

When 2-way traffic is placed on a facility that is normally one-way, the existing permanent or temporary barriers will be modified as necessary to ensure their proper crashworthiness during the temporary situation. This will include eliminating non-crashworthy end-treatments, snag points or other protrusions normally angled away or hidden from approaching vehicles.

10.13.5 Crash Cushions

Crash cushions in work zones may be used in the same manner as at permanent highway installations. Crash cushions are used to protect the motorists from the exposed ends of barriers within the clear zone, fixed objects and other hazards. Two types of stationary crash cushions are commonly used; the sand filled plastic barrel (Index 415) system; and the GREAT CZ system. Selection of a system should be the result of an analysis of site condition (i.e. space and need), first cost and replacement cost after hits. The GREAT system is designed to protest a hazard and redirect vehicles, whereas sand barrels only provide hazard protection. Therefore, the designer must determine what is needed in order to provide the appropriate device. The designer should anticipate the need for these devices and provide appropriate quantities on the plans.

10.13.6 Truck-Mounted Attenuator (TMA)

In many short-term, mobile, and moving work zones, trucks can be used as blocking vehicles to protect workers. Large trucks are effective in preventing vehicle encroachment into the work site, however, serious injury to occupants of the impacting vehicle and truck can result.

Crash cushions called truck-mounted attenuator (TMA) can be attached to the rear of these protective vehicles to reduce the severity of rear-end crashes. TMA may either be trailer or truck-mounted. If the designer sees the need for TMAs for a particular work zone activity, it should be noted on the plans and included in the pay items.

TMAs are used for three classes of protective vehicles in work zones:

- Shadow Vehicle: a moving truck spaced a short distance from a moving operation, giving physical protection to workers from traffic approaching from the rear.
- 2. Barrier Vehicle: a truck parked upstream from a stationary operation and usually unoccupied.
- 3. Advance Warning Truck: a truck parked a considerable distance upstream of a moving or stationary operation displaying an arrowpanel and other signs as appropriate.

Shadow trucks and barrier vehicles may be equipped with a TMA. Advance sign trucks should use TMAs if they encroach on the traveled way. Protective vehicles usually are equipped with arrow panels, variable message signs or flashing amber lights.

A truck with a TMA must be positioned properly with respect to the work. If the TMA is too close to the work when hit, it may strike the worker; if too far back, traffic may go around it and hit the workers. The manufacturer's recommendations should be followed for deployment and use. A "rule of thumb" for spacing is less than 2 times the speed limit in feet (e.g., 2 X 55 mph = 110 ft.) and at least 30 feet.

10.14 Flaggers

10.14.1 <u>General</u>

Flaggers shall be used only when other traffic control devices cannot adequately ensure safe and smooth traffic operations. Alternate devices such as temporary signals should be considered. Appropriate advanced warning signs shall be used in conformance with the MUTCD, and shall be removed when flaggers are not in use.

10.14.2 <u>Location of Flaggers</u>

Location of flaggers for known flagging operations shall be shown on the TCP, along with all notes necessary to fully explain conditions and requirements for their use. Refer to the MUTCD for further information on flagging.

10.14.3 Police Agencies

Police agencies (FHP, Sherrifs, or City Police) should be contacted for assistance in speed enforcement in work zones. Police escorts can also be used to assist dump trucks and other construction equipment in re-entering the traffic stream on high speed congested facilities.

10.15 Layouts

Roadway Standard Index 601 through 650 are layouts of work zone traffic control for typical conditions. These indexes should be referenced only if project conditions are nearly the same as the typical layout. Otherwise, specific plan sheets or details must be prepared. Some conditions that will require specific plan sheets include:

- Work not covered by a typical layout.
- Nighttime work requiring special lighting, oversized or additional devices.
- Ramps and intersections that interrupt the standard layout.
- Sight distance restrictions such as horizontal or vertical curves.
- Lane or shoulder configurations that do not match the standards.
- Special considerations during installation, intermediate traffic shifts and removal.
- Complex projects, including add-lane projects, that involve many phases, traffic shifts, entrances and exits.

When designing layouts, the following shall be considered:

10.15.1 Taper Lengths

Minimum taper lengths shall be calculated by the following formulas:

- o $L = S \times W$ for speeds of 45 MPH or more.
- o $L = WS^2/60$ for speeds of 40 MPH or less.

Where "L" is the length of the taper, "W" is the width of the closed lane and "S" is the posted regulatory speed for the work zone, in MPH. Both L and W are measured in feet. The following table gives the formulas for the lengths of the various taper types:

Table 10.15.1
Taper Length Criteria for Work Zones

Type of Taper	Taper Length
UPSTREAM TAPERS	
Merging Taper	L Minimum
Shifting Taper	1/2 L Minimum
Shoulder Taper	1/3 L Minimum
Two-way Traffic Taper	100 feet Maximum
DOWNSTREAM TAPERS	100 feet per lane
(use is optional)	

10.15.2 <u>Intersecting Road Signing and Signals</u>

Signing for the control of traffic entering and leaving work zones by way of intersecting highways, roads and streets shall be adequate to make drivers aware of work zone conditions. Under no condition will intersecting leg signing be less than a "ROAD CONSTRUCTION AHEAD" sign for approaching vehicles and a "END CONSTRUCTION" sign for departure vehicles unless the intersecting street consists of a shell, sand, or dirt surface, in which case it shall be left the decretion of the engineer as to the need for this type signing.

Existing traffic signal operations that require modification in order to carry out work zone traffic control shall be as approved by the District Traffic Operations Engineer. If lane shifts occur, signal heads may have to be adjusted to maintain proper position. The DTOE should also determine the need for temporary loops for traffic actuated signals. The TCP should include all necessary signal adjustments.

10.15.3 Sight Distance To Delineation Devices

Merging (lane closure) tapers should be obvious to drivers. If restricted sight distance is a problem (e.g., a sharp vertical or horizontal curve approaching the closed lane), the taper should begin well in advance of the view obstruction. The beginning of tapers should not be hidden behind curves.

10.15.4 <u>Pedestrians and Bicyclists</u>

When pedestrians and/or bicyclists are accommodated on the existing facility, provisions must be included in the TCP to accommodate them during construction. Pedestrian accommodations through the work zone must include provisions for the handicapped.

10.15.5 Superelevation

Horizontal curves constructed in conjunction with temporary work zone detours, transitions, and crossovers should have the required superelevation. Under conditions where superelevation is not used, the minimum radii that can be applied are listed in the Table 10.15.2. Superelevation must be included with the design whenever the minimum radii cannot be achieved.

TABLE 10.15.2

MINIMUM RADII FOR NORMAL CROSS SLOPES

	MINIMUM
SPEED	RADIUS
(MPH)_	(FEET)
45	1080
40	830
35	620
30	450

10.15.6 Lane Widths

Existing lane widths of through roadways should be maintained through work zone travel ways wherever practical. The minimum widths for work zone travel lanes shall be 10' for all roadways other than Interstate. On Interstate highways the minimum width for work zone travel lanes shall be 11' except at least one 12' lane in each direction shall be provided.

10.15.7 <u>Lane Closure Analysis</u>

Traffic delay as a result of lane closures should be less than five minutes. An hourly volume and capacity analysis should be performed to determine necessary hourly restrictions on closures in order to prevent excessive delays. Iane closure analysis worksheets should be included in the Special Provisions. The contractor should be permitted by plan note to refine the volumes to meet the actual conditions at the time of construction.

Exhibit I-10-A includes the Lane Closure Analysis Worksheets and two sample analyses. The designer should obtain the data detailed on Page 1 of 10 from the District Planning -Transportation Statistics Engineer, or the District Traffic Operations Engineer, and should analyze alternate detour routes before beginning the worksheets. A single worksheet per project may not produce realistic results if there is significant variance in the Distribution Factor between the AM, Noon, and PM peaks, or significant variance in the G/C ratio of signalized intersections. The number of worksheets prepared is a design judgement, but some guidance can be gleaned from the two attached samples. In the two lane sample, note that a vertical shift in either the open road or signalized line on the graph will have minimal impact on lane closure times, but in the four lane sample, a vertical shift in the signalized line on the graph will have maximum impact on lane closure times.

LANE CLOSURES

Symbols and Definitions

- ADT = Current 2 Way Average Daily Traffic. Ref: Planning
- P/D = Peak Traffic to Daily Traffic Ratio = the highest number in the ADT % Column of the 24 Hour Count on page 5.
 Range: 7-10%. Ref: Planning and samples on pages 6-9
- 3. D = Directional Distribution of Peak Hour Traffic on multilaned roads. Does not apply to a 2 Lane roadway converted to 2 Way, 1 Lane. Range: 50-65%. Ref: Planning
- 4. PMF = Peak Month Factor to ADT Ratio. Since many counties have significant variance in monthly traffic, and since the actual date of Lane Closure would be difficult to estimate, Designer should use the Peak Month Factor. Range: 100-125%. Ref: Planning and sample on page 4.
- 5. RTF = Remaining Traffic Factor is the % of traffic that cannot be diverted onto other facilities during Lane Closure. Range 0% for all diverted to 100% for none diverted. Ref: Designer Judgment
- 6. G/C Ratio = Green Time to Cycle Time Ratio to be applied when Lane Closure is thru, or within 600' of a signalized intersection.

 Range: 50-75%. Ref: Traffic Operations
- 7. V = Peak Hour Traffic Volume. Ref: Calculations on page 3.
- 8. C = Capacity of a 2L, 4L, or 6L Roadway with one lane closed, and the remaining lane(s) unrestricted by lateral obstructions. Capacity of a 4L or 6L Roadway is based on Lane Closure in only one direction.

 Ref: Table on page 3.
- 9. RC = Restricting Capacity of the above facilities by Site Specific limitations detailed in the MOT plans which apply to Travel Lane Width (TLW), Lateral Clearance (LC), and the Work Zone Factor (WZF). Ref: Calculation on page 3.
- 11. WZF = Work Zone Factor is directly proportional to the Work Zone Length (WZL) on a 2 Lane Roadway, and reduces the Capacity by restricting movement to a single lane while opposing traffic queues.

 Ref: MOT Plans and Table on page 4.

EX-I-10-A 1 of 10

LANE CLOSURE WORKSHEET STATE PROJECT NO.______ FAP NO:_____ WPI NO.: COUNTY: DESIGNER: _____ NO. EXISTING LANE: SCOPE OF WORK: ______ Calculate the peak hour traffic volume (V) V= ADT_____ x P/D____ x D____ x PMF ____ x RTF___ = ____ LANE CLOSURE CAPACITY TABLE Capacity(C) of an Existing 2 Lane-Converted to 2 Way,1 Lane = 1400 VPH Capacity(C) of an Existing 4 Lane-Converted to 1 Way 1 Lane = 1800 VPH Capacity(C) of an Existing 6 Lane-Converted to 1 Way,2 Lane = 3600 VPH Factors restricting Capacity: TLW LC WZL _____ G/C____ Calculate the Restricted Capacity (RC) at the Lane Closure Site by multiplying the appropriate 2L, 4L, or 6L Capacity (C) from the Table above by the Obstruction Factor (OF), and the Work Zone Factor (WZF). If the Lane Closure is thru, or within 600' of a signalized intersection, multiply the RC by the G/C Ratio. RC (Open Road) = C_____x OF____ x WZF___ = ____ RC (Signalized) = RC (Open Road) ____ X G/C___ = ____ If $V \leq RC$, there is no restriction on Lane Closure If V > RC, Calculate the hourly percentage of ADT at which Lane Closure will be permitted. RC (Open Road)_____ = _____ = ______% ADT______ x D____ x PMF_____ x RTF________ Signalized % = Open Road %_____ x G/C____ = ____ Plot 24 hour traffic to determine when Lane Closure permitted. NOTE: For Existing 2 Lane Roadways, D = 1.00. Work Zone Factor (WZF) applies only to 2 Lane Roadways. For RTF < 1.00, briefly describe alternate route: EX-I-10-A 2 of 10

LANE CLOSURES

CAPACITY ADJUSTMENT FACTORS

PMF SAMPLE

	Tropic County Monthly Factors					
January	1.12	July	0.88			
February	1.20	August	0.85			
March	1.18	September	0.88			
April	1.12	October	0.94			
May	1.05	November	1.00			
June	0.95	December	1.06			

NOTE: February is the peak month in Tropic County-therefore the PMF in the 2L and 4L samples is 1.20

OBSTRUCTION FACTORS (OF)

Lotavel	Travel Lane Width (TLW)					
Lateral Clearance (LC)	12'	11'	10'	9'		
≥6' 4' 2' 0'	1.00 0.98 0.94 0.86	0.96 0.94 0.90 0.82	0.90 0.87 0.83 0.75	0.80 0.77 0.72 0.65		

WORK ZONE FACTORS (WZF)

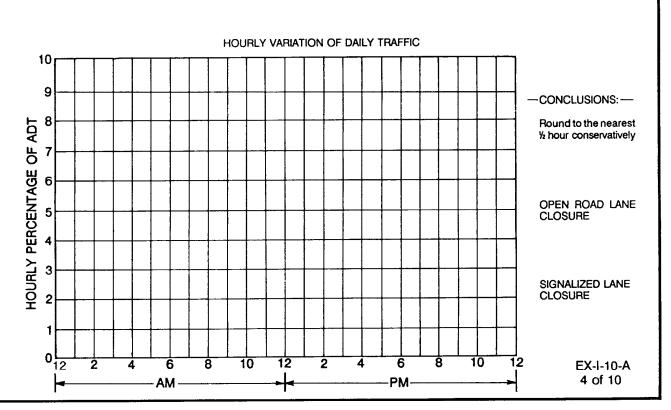
WZL	WZF	WZL	WZF	WZL	WZF
200' 400' 600' 800' 1000' 1200' 1400' 1600' 1800' 2000'	0.98 0.97 0.95 0.93 0.92 0.90 0.88 0.86 0.85 0.83	2200' 2400' 2600' 2800' 3000' 3200' 3400' 3600' 3800' 4000'	0.81 0.80 0.78 0.76 0.74 0.73 0.71 0.69 0.68 0.66	4200' 4400' 4600' 4800' 5000' 5200' 5400' 5600' 5800' 6000'	0.64 0.63 0.61 0.59 0.57 0.56 0.54 0.52 0.51

Work Zone Length (WZL) for 2 Lane Roadways = Distance between opposing traffic queues \triangleright **TRANSITION BUFFER** WORK **TERMINATION AREA -**ADVANCE WARNING AREA -AREA - moves traffic out of its SPACE -AREA lets traffic resume tells traffic what to expect ahead provides normal driving normal path protection for EX-I-10-A traffic and 3 of 10 workers

LANE CLOSURES

24 HR COUNTS

	AM		P₩	1	
TIME	VOLUME	ADT%	VOLUME	ADT%	
12 - 1					
1 - 2					DATE:
2 - 3					
3 - 4					
4 - 5					DESIGNER:
5 - 6					
6 - 7					
7 - 8					PROJECT NO.:
8 - 9					
9 - 10					
10 - 11					LOCATION:
11 - 12			-		
		TOTAL			



LANE CLOSURE WORKSHEET
STATE PROJECT NO. 12345 - 6789 FAP NO: NA
WPI NO.: 1234567 COUNTY: TROPIC DESIGNER: YATES
NO. EXISTING LANE: Z SCOPE OF WORK: WIDEN & RESURFACE
Calculate the peak hour traffic volume (V) $V = ADT 15,000 \times P/D_0.083 \times D_A/A \times PMF_1.20 \times RTF_0.75 = 1120$
LANE CLOSURE CAPACITY TABLE
Capacity(C) of an Existing 2 Lane-Converted to 2 Way,1 Lane=1400 VPH Capacity(C) of an Existing 4 Lane-Converted to 1 Way,1 Lane=1800 VPH Capacity(C) of an Existing 6 Lane-Converted to 1 Way,2 Lane=3600 VPH
Factors restricting Capacity:
TLW 10' LC 4' WZL 2100' G/C 0.64
Calculate the Restricted Capacity (RC) at the Lane Closure Site by multiplying the appropriate 2L, 4L, or 6L Capacity (C) from the Table above by the Obstruction Factor (OF), and the Work Zone Factor (WZF). If the Lane Closure is thru, or within 600' of a signalized intersection, multiply the RC by the G/C Ratio.
RC (Open Road) = C /400 x OF 0.87 x WZF 0.82 = 999
RC (Signalized) = RC (Open Road) 999 X G/C 0.64 = 639
If V < RC, there is no restriction on Lane Closure If V > RC, Calculate the hourly percentage of ADT at which Lane Closure will be permitted.
RC (Open Road) 999
% = 7.40 % ADT 15,000 x D 1.00 x PMF 1.20 x RTF 0.75
Signalized % = Open Road % 7.40 x G/C 0.64 = 4.74
Plot 24 hour traffic to determine when Lane Closure permitted.
NOTE: For Existing 2 Lane Roadways, D = 1.00. Work Zone Factor (WZF) applies only to 2 Lane Roadways. For RTF < 1.00, briefly describe alternate route: 25% of existing
traffic diverted West on Bullard Blud. North on
Newhall Nene, then East on Xanders X-way
EVILOR

EX-I-10-A 5 of 10

LANE CLOSURES 24 HR COUNTS AM PM TIME **VOLUME** ADT% VOLUME ADT% 160 960 12 - 1 DATE: 1 - 2 Feb-1988 2 - 3 3 - 4 4 - 5 DESIGNER: YATES 5 - 6 6 - 7 PROJECT NO .: 7 - 8 12345-6789 8 - 9 9 - 10 BUCK Lake Rd. 10 - 11 960 11 - 12 100 TOTAL HOURLY VARIATION OF DAILY TRAFFIC 10 9 -CONCLUSIONS: -HOURLY PERCENTAGE OF ADT OPEM ROAD Round to the nearest 1/2 hour conservatively 4.1 OPEN ROAD LANE CLOSURE STGNALIZED 6:30 p.m. - 4:00 p.m SIGNALIZED LANE CLOSURE 8:00 p.m. - 7:00 a.m. 0<u>L</u> 12 10 6 10 12 EX-I-10-A -PM-- AM -6 of 10

		LANE CLOS	SURE WOF	RKSHEET		
STATE PROJECT N	10. 123	45-678	39	FA	P NO: NA	·
WPI NO.: 12345	67 COUNT	Y: TROPIC	DESIGNE	R: <i>GIDD</i>	ENS	
NO. EXISTING LAN	E: 4	SCOPE OF WO	PRK:_R	ESURFE	CE	
Calculate the peak V= ADT <u>30,000</u>	hour traffic vo _x P/D_ <i>0.03</i>	olume (V) 33 x D <i>O.</i> :	55_x	PMF <u>/.20</u>	x RTF <i>1.00</i>)_= <u>/643</u>
		LANE CLOSU	RE CAPAC	CITY TABLE		
Capacity(C) of an E Converted to 1 Wa Capacity(C) of an E	y,1 Lane=180	0 VPH			acity(C) of an E	xisting 4 Lane-
Factors restricting						
TLW//	LC	6'	WZL	NA for 41	G/C_ <i>O</i> ,	74
Calculate the Restri Capacity (C) from the Lane Closure is the	ne Table abov	e by the Obstruc	tion Facto	r (OF), and the W	ork Zone Facto	r (WZF). If the
RC (Open Road) =	c <u>/800</u>	2x OF_	0.96	x WZF	1.00	= <u>/728</u>
RC (Signalized) = 1	RC (Open Roa	ad) /728	X G/	c 0.74	= /2	19
If $V \leq RC$, there is If $V > RC$, Calculat Closure will	no restriction e the hourly p I be permitted	ercentage of AD	e OT at which	n Lane		
	en Road) <u>//</u>	728			072	
% = ADT <u>30,0</u>	00 x D 0.	55 x PMF_	1,20	_ x RTF_ /, Ox	8,73 2_	%
Signalized % = Op	en Road %	8.73		_x G/C <i>O</i> ,	74	= 6.46
Plot 24 hour traffic	to determine	when Lane Clos	ure permit	ted.		
	ctor (WZF) ap	ways, D = 1.00. plies only to 2 L ribe alternate ro	ane Roadv			
						EX-I-10-A

7 of 10

LANE CLOSURES 24 HR COUNTS AM PM VOLUME ADT% TIME **VOLUME ADT%** 1920 6,4 320 1.1 12 - 1 180 5.5 1660 1 - 2 DATE: 0.2 5.4 60 1620 2 - 3 FEB. 1988 0.2 7.2 50 2160 3 - 4 0.2 60 2380 DESIGNER: 4 - 5 260 2480 0.9 GIDDENS 5 - 6 1050 1860 6 - 7 2270 1360 PROJECT NO .: 7 - 8 1820 6.1 3,5 1060 12345-6789 8 - 9 850 1740 5,8 2,8 9 - 10 1650 5,5 730 2,4 LOCATION: 10 - 11 1920 540 1,8 BUCK LAKERD. 11 - 12 30,000 100.0 STA 100-STA 200 TOTAL HOURLY VARIATION OF DAILY TRAFFIC 10 9 CONCLUSIONS: -AD Round to the nearest 1/2 hour conservatively HOURLY PERCENTAGE OF OPEN ROAD LANE CLOSURE NO RESTRICTION SIGNALIZED LANE CLOSURE 9:00 AM - 3:30PM 7:00 PM - 7:30 AM 0<u>L</u> 12 12 EX-I-10-A 8 of 10 AM. -PM-

OUTBOUND LANE CLOSURES VOLUME 1399 1201 1201 1201 1201 1364 697 697 697 510 HOURLY VARIATION OF DAILY TRAFFIC 24 HR COUNTS HOUR OF DAY SAMPLE 4-LANE SITE = SR 60 @ US 301 EAST OF TAMPA - HILLSBOROUGH CO 298 144 96 151 136 156 167 1094 1094 1378 COMPOSITE LANE CLOSURES 2836 2594 2644 2644 2823 3060 3080 2362 1775 1212 890 662 42,232 /OLUME HOURLY VARIATION OF DAILY TRAFFIC 24 HR COUNTS HOUR OF DAY 232 294 295 294 299 299 2559 2403 2603 2694 7 · 8 8 · 9 9 · 10 4 - 5 9 - 9 2 - 9 1437 1387 1358 1261 1261 1261 1289 901 594 518 380 252 252 NBOUND LANE CLOSURES HOURLY VARIATION OF DAILY TRAFFIC 24 HR COUNTS HOUR OF DAY 4 - 5 8.9 HOURLY PERCENTAGE OF ADT EX-I-10-A 9 of 10

LANE CLOSURE WORKSHEET SUMMARY

LANE SAMPLE WITH SIGNIFICANT AM-PM PEAKS

SAMPLES = INBOUND (WB), COMPOSITE (EB & WB), OUTBOUND (EB)

SITE = SR 60 @ US 301 EAST OF TAMPA, HILLSBOROUGH CO.

COMPONENT	INBOUND	COMPOSITE	OUTBOUND	
ADT	21,760	42,232	20,472	
P/D	0.103	0.073	0.092	
D	1.00	0.60	1.00	
PMF	1,17	1.17	1.17	SEE NEXT PAGE
RTF	1.00	1.00	1.00	
V	2622	2164	2203	
TLW	12'	12'	12'	
LC	o'	0'	0'	
С	1800	1800	1800	
OF	0.86	0.86	0.86	
RC(OPEN ROAD)	1548	1548	1548	
G/C	0.50	0.50	0.50	
RC(SIGNAL)	774	774	774	
% OPEN ROAD	6.10	5.20	6.50	
% SIGNAL	3.05	2.60	3,25	
LANE CLOSURE	7:00 A.M	7:00 A.M	11:30 A.M.	-
(OPEN ROAD)	4:00 P.M.	7:30 P.M.	7:30 P.M.	CONCLUSIONS
LANE CLOSURE	6:00 A.M-	6:00 A.M	7:30 A.M	1
(SIGNAL)	9:00 P.M.	10:30 P.M.	- 10:30 P.M.	-
•	' i	'		EX-I-10-A 10 of 10

10.15.8 Detours

A detour is a deviation from the normal roadway and is initiated when traffic is directed to leave the roadway. Activities such as traffic shifting and/or splitting accomplished within the confines of the roadway do not involve detours, but a crossover involves a detour because traffic is directed to depart the directional roadway.

There are two types of detours: on-site and off-site. At an on-site detour traffic is diverted onto a temporary roadway generally constructed within or adjacent to the right-of-way or onto a frontage road. At an off-site detour traffic is diverted onto another highway in order to bypass the work zone.

For off-site detours, the detour signing is usually done under the direction of the traffic engineer who has authority over the The detour should be signed clearly so drivers can roadway. traverse the entire detour and return to the original roadway. Detour signing is not required for minor shifts to the direct or There will be situations that are between minor shifts and re-routing. In these cases engineering judgement must be applied to ensure that traffic is given clear and adequate When shifts and detours are required the designer direction. should analyze the effects of the action on the capacity of the roadway and take the necessary steps to minimize adverse The structural capacity of the shoulder or detour pavement should also be considered and additional structure provided if necessary.

The designer has the option of paying for detours by using the "special detour" lump sum pay item or by using the individual pay items for each component (i.e. asphalt pavement, base, signs, markings, etc.) of the detour. The special detour pay item is to be used only when the work and quantities included for pay under the item are tabulated and noted in the plans.

Even though shifts may meet the definition of a detour and a "special detour" may be called for contract pay purposes, it may not be appropriate to sign the shift as a detour. Signing should match the perceptions and desired responses of the driving public as per the MUTCD rather than administrative definitions.

TCPs should include sufficient detail for detour geometry. Detours should be designed with some shoulders (2' min) whenever practical. The radius of curvature and taper lengths should be shown. Detours should be designed and operated as close to the normal speed as possible. When speed reductions are necessary, the reduction should be in 10 mph increments. The recommended minimum radius of curvature for detours is shown in Table 10.15.2.

10.15.9 Above Ground Hazards

An above ground hazard is any object, material, or equipment, which does not meet the Department's safety criteria for clear zones - i.e., anything that is greater than 4 inches in height and is firm and unyielding or doesn't meet breakaway requirements.

Construction hazards located within the travel way or from the outside edge of pavement through the clear zone for the highway are to be considered work areas and treated as required by the appropriate warning devices during the contractor's work hours or eliminate the hazard. During non-working hours, all objects, materials, or equipment that constitutes a hazard, must be stored/placed outside the clear zone or be protected by a barrier wall.

For above ground hazards within a work zone, the clear zone required should be based on the regulatory speed posted during construction.

10.15.10 Drop-offs in Work Zones

Acceptable warning and barrier devices for traffic control at drop-offs in work areas are detailed on Sheet 4 of 6 of <u>Standard</u> Index 600.

The designer should anticipate dropoffs which are likely to occur during construction and provide the appropriate devices thru pay items and quantities. For those projects where barrier wall would be needed and yet it is not practical - such as highly developed urban areas where numerous driveways exist - the designer should consider adding plan notes which restrict the length of the contractors operations in order to reduce the dropoff at acceptable level, prior to the end of the days operations.

10.15.11 Narrow Bridges

Simultaneously working on both sides of a bridge (bridge widening, etc.) may be hazardous due to the narrow width of some bridges. Considerations should be given to specifying that work be done only on one side at a time. In some situations, the installation of barrier wall on both shoulders totally eliminate any shoulder or refuge area. The designer should consider whether or not this restriction of the effective bridge width is acceptable and consistent with the desired operational ability of the facility.

10.16 Speed Zoning

10.16.1 Regulatory Speeds in Work Zones

The goal of traffic control plans for construction, maintenance, and utility operations is to route traffic through such areas in a manner comparable to normal highway conditions. Changes to the existing posted speed limits should only be made after consideration of actual or anticipated field conditions, including: vehicular volumes, congestion, TCP phasing, lane restrictions, type of construction, closeness of traffic to workers, equipment, flagger usage, pedestrians, geometrics, and physical conditions. By virtue of F.S. 316.187, all regulatory speeds must be established on the basis of a traffic and engineering investigation. The justification for establishing work zone regulatory speeds different from normal speed limits must be included in the project file.

When developing a TCP, the considerations noted above must be addressed in determining the appropriate regulatory speeds. The TCP and the project file will suffice as the traffic and engineering investigations. TCPs for all projects must show specific regulatory speeds for each phase of work. This can either be the existing posted speed or a reduced speed. The speed shall be noted in the TCP. Guidelines for determining the appropriate speed reduction are given in Table 10.16.1.

If field conditions warrant speed reductions greater than those shown in the Traffic Control Plan (TCP), then the contractor may submit to the project engineer for approval by the Department, a signed and sealed study to justify the need for further reducing the posted speed or the engineer may request the District Traffic Operations Engineer (DTOE) to investigate the need. It will not be necessary for the DTOE to issue regulations for regulatory speeds in work zones due to the revised provisions of F.S. 316.0745(2)(b). However, all other regulatory signs, work zone or permanent, require issuance of a regulation by the DTOE.

Table 10.16.1

REGULATORY SPEED REDUCTIONS FOR USE IN CONSTRUCTION AND MAINTENANCE OPERATIONS

er situations ions for r workers.	*Unless drop-offs or other situations create hazardous conditions for motorists, pedestrians or workers.	all requirements of trol Devices (MUTCD).	Regulatory Speed signs shall meet all requirements of the Manual on Uniform Traffic Control Devices (MUTCD).	NOIE: Regulaton the Manua
10 MPH	MAY BE USED	Greater than one daylight period		of pavement.
N/A	SHOULD NOT BE USED*	One daylight period or less	Utility Work Guardrail Maintenance Shoulder Work	Activities which encroach the area from the edge of the pavement to 2' from the edge
10 MPH	MAY BE USED	Greater than one daylight period	Structures Reworking Ditches Sign Installation and Maintenance Shoulder Work	pavement.
N/A	SHOULD NOT BE USED*	One daylight period or or less	Utility Work Culvert Extensions Side Slope Work Guardrail Maintenance Landscaping Work	Activities which encroach the area closer than 15' but not closer than 2' to
N/A	SHOULD NOT BE USED*	Any Time Period	Landscaping Work Utility Work Fencing Work Cleaning Drainage Structures Reworking Ditches	Activities are more than 15' from the edge of pavement.
SUGGESTED AMOUNT OF SPEED REDUCTION	REDUCTIONS TO EXISTING REGULATORY SPEEDS SI	DURATION OF WORK	TYPICAL APPLICATIONS	CONDITIONS

Table 10.16.1 (Continued)

REGULATORY SPEED REDUCTIONS FOR USE IN CONSTRUCTION AND MAINTENANCE OPERATIONS

CONDITIONS	TYPICAL	DURATION OF WORK	REDUCTIONS TO EXISTING REGULATORY SPEEDS	SUGGESTED AMOUNT OF
				SPEED REDUCTION
Activities which	Pavement marking	One hour or less	SHOULD NOT BE USED*	N/A
between the	Pavement Repair	Greater than one hour	MAY BE USED	10-20 MPH
centerline and	Utility Work			
the edge of	Bridge Repair			
pavement.	Widening			
(Tanke CTOPATES)				
Activities which	Shoulder and Slope	One hour or less	SHOULD NOT BE USED*	N/A
require inter-	Utility Work			
mittent or	Guardrail Maintenance			
moving operation	Landscape Work	Greater than one hour	SHOULD NOT BE USED*	N/A
on the shoulder.	Delineator Installation			
	Widening			
Activities	Bridge Construction	Any time period	MAY BE USED	10-20 MPH
requiring a	Subgrade Restoration	1		
temporary detour	Culvert Repair			
to be constructed**	to be constructed** Roadway Construction			
Activities which	Pav't Marking	Any time period	MAY BE USED	10-20 MPH
encroach the area	Pav't Resurfacing	1		
beyond either the	Use of Temporary			
centerline of a	Barrier Wall			
roadway or lane	Installation of			
line of a multi-	Drainage Laterals			
lane highway				
NOTE: Regulatory	Regulatory Speed signs shall meet all		*Unless drop-off or other situations create hazardous	te hazardous

conditions for motorists, pedestrians or workers **Detour and transition geometrics which meet the existing regulatory speed should be provided whenever possible. requirements of the Manual on Uniform Traffic Control Devices (MUTCD).

Regulatory speed signs in rural areas (Interstate and Non-Interstate) are to be preceded by a "REDUCE SPEED AHEAD" sign positioned as follows:

Interstate (Rural) - 1000 feet in advance Non-Interstate (Rural) - 500 feet in advance

All urban areas do not require an advance sign, however, it may be included at the designer's option.

The "Regulatory Speed" and "Reduce Speed Ahead" signs are to be paid for under a pay Item, 102-96 Temporary Regulatory Signs per each per day.

If the existing regulatory speed is to be used, consideration should be given to supplementing the existing signs when the construction work zone is between existing regulatory speed signs. For projects greater than one mile in length for rural areas (Non-Interstate) and on Rural or Urban Interstate, regulatory signs are to be placed at no more than one mile intervals. For urban situations (Non-Interstate) regulatory speed signs are to be placed at a maximum of 1000 feet apart.

The 85 percentile speed used to establish normal regulatory speeds does not apply for construction zones. Changes to the existing speed should be made on actual or anticipated field conditions such as: vehicular volumes, congestion, TCP phasing, lane restrictions, type of construction, closeness of traffic to workers, equipment, flagger usage, pedestrians, geometrics and physical conditions. This is interpreted to mean, using engineering judgement, the responsible designer is to establish the regulatory speed and so note in the plans and project file, after considering the above conditions and the degree to which warning devices are required. In general, the regulatory speed should be established to route vehicles safely through the work

zone as close to normal conditions as possible. The regulatory speed should not be reduced more than 20 MPH below the posted speed. This reduction is to be done in 10 MPH per 500 feet increments.

To ensure credibility with motorists and enforcement agencies, reductions in the existing posted speed shall be removed as soon as the conditions requiring the reduced speed no longer exist. On longer projects, such as interstate resurfacing, the speed reductions should be located in proximity to the activities which merit a reduced speed and not "blanketed" for the entire project.

When the regulatory speed is changed in a work zone, the permanent speed limit signs are to be removed during the period when the work zone regulatory speed zones are in effect.

Once the work zone regulatory speeds are removed, the regulatory speed existing prior to construction will automatically go back into effect unless a new regulation is issued by the DTOE to revise the previous speed.

10.16.2 Advisory Speeds in Work Zones

Advisory Speed plates should not normally be specified in the plans, unless they are being used merely to reinforce the regulatory speed signs. Advisory Speed plates will be used at the option of the field engineer for temporary use while processing a request to change the regulatory speed specified in the plans when deemed necessary. Advisory speed plates cannot be used alone, but must be placed below the construction warning sign for which the advisory speed is required.

CHAPTER 11

PAY ITEMS, QUANTITIES AND SPECIFICATIONS

11.1 Items and Quantities and Specifications

The Federal-Aid Highway Program Manual (FHPM), Vol. 6, Chap. 4, Sec 1, Subsec. 6 states that federal-aid construction contracts shall provide that the Contractor is to furnish all necessary materials and that the Contractor shall be permitted to select the source from which materials are to be obtained. Exceptions may be made when there is a definite finding by the Department and concurred by the FHWA Division Administrator that it is in the public interest to require use of state furnished material or use of material from a designated source.

Payment for materials used in the construction project is made for each pay item included in the project. Calculations and tabulations that substantiate the quantities for each pay item for both federal-aid and non federal-aid projects, should be considered as part of the plans package. These computations should be on standard forms as described in the Department's Computation Manual. of <u>Estimates Manual</u> presents the Department's standard method of calculating design quantities for construction pay items. nonstandard methods used in computing quantities should be clearly and completely documented in the Computation Book by showing calculations and the basis of estimates used. Items calculated using the standard basis of estimate may sometimes require clarification especially if several intermediate computations are necessary to arrive at the total quantity. These computations should also be documented in the Computation Book.

The Department's current practice is to provide for final payment under the plan quantity concept for a large number of commonly used items. Items under this concept require that the estimated quantities be calculated and documented as accurately as possible. Exceptions to this will be allowed for asphalt and base items for resurfacing, widening and intersection improvement projects. These items may be evaluated for payment under final measurement on a project by project basis. Projects for which the Districts desire to have these items paid under final measurement shall include in the plans transmittal package to Tallahassee, a letter to the Engineer of Specifications, listing the specific items desired to be paid by final measurement. The items for which payment may be allowed under final measurement are the payment and base square yard items. Tonnage items are not included in the plan quantity concept.

Allowing final measurement on these items does not release the designer from calculating the quantities as accurately as possible. Field reviews and design surveys are still required, when necessary, to define or establish scope and/or essential topography. Planimetering of areas from plans for resurfacing and widening is allowed, as long as the scale is such that the area can be calculated as accurately as possible.

For projects that have partial federal-aid, adequate distinction should be clearly made between participating (included in federal aid) and nonparticipating (not included in federal-aid) items. All nonparticipating items or partial quantities should be identified on the plans and on the Summary of Quantities sheet as has been done in the past. The method of presenting this information must be of sufficient detail for project personnel to readily distinguish between participating and nonparticipating work, including its physical location in the project. Project personnel must be able to properly account for the necessary separation of quantities. These separated quantities should also be properly identified as to

participating and nonparticipating work when entered into CES. few cases certain lump sum items such as mobilization, maintenance of traffic, etc. may be at least partially Federal Aid nonparticipating depending upon the nature of other nonparticipating items which must Where it is determined that certain lump sum items should be partially Federal Aid nonparticipating, the percentage assignment of nonparticipating should be negotiated with the FHWA. Upon mutual agreement the summary of quantities should so note the appropriate percentage by footnote to the summary, and describe the area of the project to which this note applies. This percentage should also be reflected when entering data into CES. Where joint project agreements are involved between the Department and the City or County, appropriate participation information regarding this particular phase of the work should be so noted. These items should be determined during early stages of project development. Appropriate final adjustments should be included in the plans package and CES entries appropriately modified, if such items arise during the PS & E phase.

All projects (including resurfacing, widening, traffic operations, safety, etc.) shall utilize the individual pay items as listed in the <u>Master Pay Item List</u>.

Utility contract plans which have a 6000 job number, and are let in the contract as roadway work, need to have special attention given to the establishment of pay items and loading CES. All items of work related to the utility work shall be included in the CES under 6000 project numbers. Lump sum items such as Mobilization and Maintenance of traffic should be loaded for both the roadway work (3000 series project number) and utility work (6000 series project number). Traffic Control Plan (TCP) items that are paid by per each or per each per day (barricades, signs, etc.) are to be included under the

6000 project when the work is clearly done separately from the roadway construction work. If the utility work is done concurrently with the roadway work, individual TCP items are not required under the 6000. However, the TCP Lump Sum item must be included in all cases under both the 3000 and 6000 projects when TCP work is anticipated.

For those projects which have minor joint project agreement work, (such as adjusting manholes, valves, etc.), the lump sum items of mobilization and maintenance of traffic do not have to be included under the 6000 series projects.

11.2 New Pay Items

New construction material and new uses of existing construction materials require new pay item definitions. The Department has established the following procedure to establish new pay items:

The originator of a new pay item should submit a draft of the pay items specification, any manufacturer's brochures, estimated material and labor cost, a completed 781-25R form (Exhibit I-11-A) and any other relevant data to the central roadway design office. This Any inquiries arising during package will be thoroughly reviewed. the review will be addressed to the originator through the Central office design coordinator. Upon successful completion of review, the package will be forwarded to the specifications office for further review, approval and pay item number assignment, after which it is transmitted to the estimates office for review, approval and entry into the data base system. After the pay item number is entered into the estimates office's database system, the design coordinator will be notified of the approval and copies of the new pay item will be distributed.

PAY ITEM REQUEST

	PAY ITEM NO		- UNIT	OF MEASURE
FULL ITEM NAME				
	MATION: (Attach Addition	al Sheet if No	ecessary)	
Spec Yr COST	DATA:		,	
ORIGINATOR	DISTRICT	_DATE/_	/ PHONE	
DESIGN COORDINATOR_			APPROVED	DISAPPROVED
	ESIGN SECTION			
DESIGN CHECKLIST DATA	A			
NEW CHECKLIST ITEM_				
	DATE/			
SPECIFICATIONS OFFICE				
SPECIFICATION BOOK	SPECIAL PROVI	SIONS	□ APPROVED	DISAPPROVED
DEVELOPMENT	☐ ATTACHMENT		SUPPLEMENTA	AL SPECIFICATION
SIGNATURE		DATE_/	/_ PHONE	
ESTIMATES (Engineering	Support)		APPROVED	DISAPPROVED
SIGNATURE	ALVIEW TO A CONTROL OF THE PROPERTY OF THE PRO	_DATE/_	_/PHONE	
* DISAPPROVAL REASON	l;			
	*			

NOTE: ORIGINAL TO BE PROCESSED THROUGH ESTIMATES. COPY TO BE RETAINED BY COORDINATOR AND SPECIFICATIONS.

JOB NUMBER:

USE CURRENT STATE PROJECT NUMBER FOR WHICH NEW ITEM NUMBER WILL FIRST BE USED.

PAGE ITEM NUMBER:

THIS NUMBER WILL BE ASSIGNED BY THE STATE SPECIFICATIONS ENGINEER IN THE CENTRAL OFFICE.

UNIT OF MEASURE:

A MAXIMUM OF TWO SPACES USED TO ABBREVIATE THE METHOD OF MEASURE FOR THIS ITEM. (SEE STANDARD INDEXES FOR STANDARD ABBREVIATIONS)

FULL ITEM NAME:

DESCRIBE THE NEW PAY ITEM SO THAT THIS "NAME" BEST RELATES TO FUTURE USERS WHAT THIS ITEM IS USED FOR

DETAILED PAY ITEM INFORMATION:

PROVIDE DRAFT SPECIFICATIONS WHICH FULLY DESCRIBES THE NEW PAY ITEM FORMATTED TO THE CURRENT SPECIFICATION BOOK. THE SPECIFICATIONS OFFICE WILL PUT THE DRAFT INTO THE FINAL FORM BUT THEY MUST HAVE THE USERS IDEAS ON THE DESCRIPTION, METHOD OF MEASUREMENT, HOW THIS ITEM WILL BE USED AND SUGGESTED METHOD OF PAYMENT.

COST DATA:

PROVIDE A BREAKDOWN OF THE COST PER UNIT OF MEASURMENT WITH NUMBER OF SKILLS REQUIRED AND EQUIPMENT: SHOULD SUPPLIER OR MANUFACTURER BE KNOWN PLEASE INCLUDE NAME & PHONE.

ORIGINATOR SIGNATURE:

WHEN THE ORIGINATOR HAS PREPARED THE DRAFT SPECIFICATIONS, COST DATA AND ANY OTHER AVAILABLE DATA HE/SHE WILL SIGN THIS FORM SIGNIFYING THEIR APPROVAL OF THE ATTACHED MATERIAL.

DISTRICT:

THE DISTRICT NUMBER OF THE ORIGINATOR.

DATE AND PHONE NUMBER: SEE NOTE BELOW.

DESIGN COORDINATOR:

THE CENTRAL OFFICE DESIGN COORDINATOR WILL RECEIVE THE PAY ITEM PACKAGE REVIEW FOR COMPLETENESS. EVALUATE THE REQUEST FOR STATEWIDE UNIFORMITY AND AFTER FINAL APPROVAL DISTRIBUTE PAY ITEM PACKAGE TO DISTRICT DESIGN ENGINEERS, ORIGINATOR AND DISTRICT ESTIMATOR FOR THEIR FUTURE REFERENCE.

DESIGN SECTION:

THE DESIGN SECTION APPROVING THE USE OF ABOVE PAY ITEM, (EXAMPLE ROADWAY, STRUCTURES, BRIDGE REFURB, LIGHTING, ETC.) DATE AND PHONE NUMBER: SEE NOTE BELOW.

CES DESIGN CHECKLIST INFORMATION:

THE DESIGN COORDINATOR WILL ASSIGN THE STANDARD STATEMENT NUMBERS APPROPRIATE FOR THIS PARTICULAR ITEM, AND ADD ANY NEW STATEMENTS THAT ARE APPROPRIATE.

DESIGN COORDINATOR SIGNATURE:

WHEN THE DESIGN COORDINATOR HAS REVIEWED, EVALUATED AND CHOSEN CES CHECKLIST STATEMENTS HE/SHE WILL SIGN THIS FORM SIGNIFYING THEIR APPROVAL OF THE ATTACHED MATERIAL.

SPECIFICATIONS:

SPECIFICATIONS OFFICE WILL REVIEW AND EVALUATE REQUEST AND DESIGNATE SECTION LOCATION. SECTION LOCATION IS INDICATED BY THE 2ND, 3RD & 4TH DIGIT IN THE FIRST GROUP OF THE PAY ITEM NUMBER.

TYPE OF SPECIFICATION:

THIS OFFICE WILL ALSO DESIGNATE SPECIFIC LOCATION OF THE REQUEST IN SPECIFICATIONS PACKAGE (SPECIAL PROVISIONS, SUPPLEMENTAL SPECIFICATIONS, ETC.)

SIGNATURE:

SIGNATURE OF THE AUTHORIZED PERSON IN SPECIFICATIONS OFFICE.

DATE AND PHONE NUMBER: SEE NOTE BELOW.

ENGINEERING SUPPORT:

SIGNATURE:

SIGNATURE OF AUTHORIZED PERSON FOR ESTIMATES, ENGINEERING SUPPORT OFFICE.

DATE AND PHONE NUMBER: SEE NOTE BELOW.

NOTE:

DATE IS THE MONTH, DAY AND YEAR REQUEST IS LEAVING THAT OFFICE, PHONE NUMBER - THE PHONE NUMBER OF THE AUTHORIZED PERSON TO CONTACT IF ADDITONAL INFORMATION SHOULD BE REQUIRED.

Chapter 12

RIGHT-OF-WAY

12.1 General

Right-of-way requirements must be carefully checked during the design process. Roadway cross sections should be reviewed to determine if the limits of construction lie outside the established right-of-way, additional right-of-way must be requested, or cut and fill slopes must be revised so that all proposed construction will be within the right-of-way. Retaining walls may be necessary to avoid damaging key parcel remainders.

Close cooperation and coordination of activities is to be maintained with the Right-of-way Office in the selection of corridors for location of the improvement and establishment of reasonable right-of-way limits based on property ownerships, economics, alignment, grades, land use, typical sections, etc. As the preliminary layout progresses, the District Right-of-way Surveyor should be coordinated with, on-site reviews held, and rough appraisals obtained where necessary in order to assure a reasonable and practical facility that can be economically, aesthetically and operationally justified.

The Right-of-way Office should be involved in the development of the approximate right-of-way requirements as early as possible in the plans preparation stage so that title searches and acquisitions may be initiated on a timely basis.

If the requirements are altered, the Right-of-way Office must be notified immediately in writing. The right-of-way check of completed plans is made by representatives of the Design Office and the Right-of-way Office.

The right-of-way shown on the plans must be in exact agreement with that shown on the right-of-way map. If a connection can be provided outside right-of-way which does not significantly reduce the utility or value of the parcel, then a restoration agreement should be sought by our Right-of-way Department. if the agreement cannot be secured, a wall should be constructed. If a connection outside right-of-way will, in the opinion of the Right-of-way Department, significantly impact the utility or value of the parcel, then a construction easement should be obtained to provide a mechanism or opportunity to compensate the owner for the impacts. Every effort should be made in the latter situation to avoid this impact by adjusting the design since it will add a right-of-way phase to the project and possibly delay the project.

On projects which otherwise require right-of-way, an attempt should be made to obtain the restoration agreement so that a connection can be made. If the owner refuses to grant the restoration agreement, and no other reasonable access is available, and the appraised value and administrative cost to obtain an easement is less than a wall, or if a connection will significantly impact the utility of the property, the construction easement should be acquired.

Lateral ditch cross sections should be examined, and right-of-way requirements for the ditches established. A reasonable distance will be required between top of slope and right-of-way line, on one side at least, if maintenance equipment cannot be operated in the ditch.

After right-of-way requirements are determined, these data are referred to the Right-of-way Office for completion of the right-of-way maps.

Chapter 13

PROJECT DEVELOPMENT

13.1 General

This chapter explains the overall process of highway project development, from the Five Year Work Program to the project letting stage. A brief outline of the scope requirements at the various significant phases of the process will also be discussed.

13.2 Process

13.2.1 Five Year Work Program

The development of a Five Year Work Program begins with the identification of short and long range statewide transportation needs. Planning documents are developed with inputs from Florida DOT, regional planning groups and local governments. Data concerning the condition of existing highways are also used to establish priorities among the proposed projects. Once the basic needs are established, a financial plan identifying the sources and amounts of available funding is developed. Funding allocations are made based on each district's transportation needs, population, lane-miles, gasoline sales and other appropriate factors. Each district then refines their list of proposed projects to be consistent with the amount of funding available.

13.2.2 PD&E Progess

The project development phase for projects included in the Five Year Work Program and requiring PD&E includes environmental studies, the determination of project alignment and completion of preliminary design. Coordination with the Federal Highway Administration (FHWA), the State Departments of Environmental Regulation and Natural Resources, and public input are important elements of this phase. The PD&E process is described in detail in the PD&E Manual.

A detailed engineering report is normally produced in the PD&E stage which documents some of the key criteria to be used in the design of the particular facility. Typical sections, interchange and intersection locations and configurations, drainage features, conceptual bridge design, highway lighting justification, and right-of-way requirements are some of the elements that are sometimes finalized during the PD&E phase of the project. For some projects, value engineering reviews are conducted during the PD&E study. Traffic control through work zones should also be considered.

Environmental investigations made during the course of a PD&E study are documented in the Environmental Impact document. Environmental documents frequently commit the State to certain restrictions, features and mitigation measures that must be included in the final design. The designer should carefully review the environmental documents before beginning the design. Sometimes restrictions, features and/or mitigation measures stipulated in the environmental document are such that the final design becomes unworkable. This will require a change in the environmental document. This document also lists the permits required to be obtained prior to construction of the project.

Although the district offices take the lead role in project development, the central office performs a quality assessment review of selected projects to ensure compliance with department standards and procedures. The project development phase usually takes from 12 to 36 months to complete.

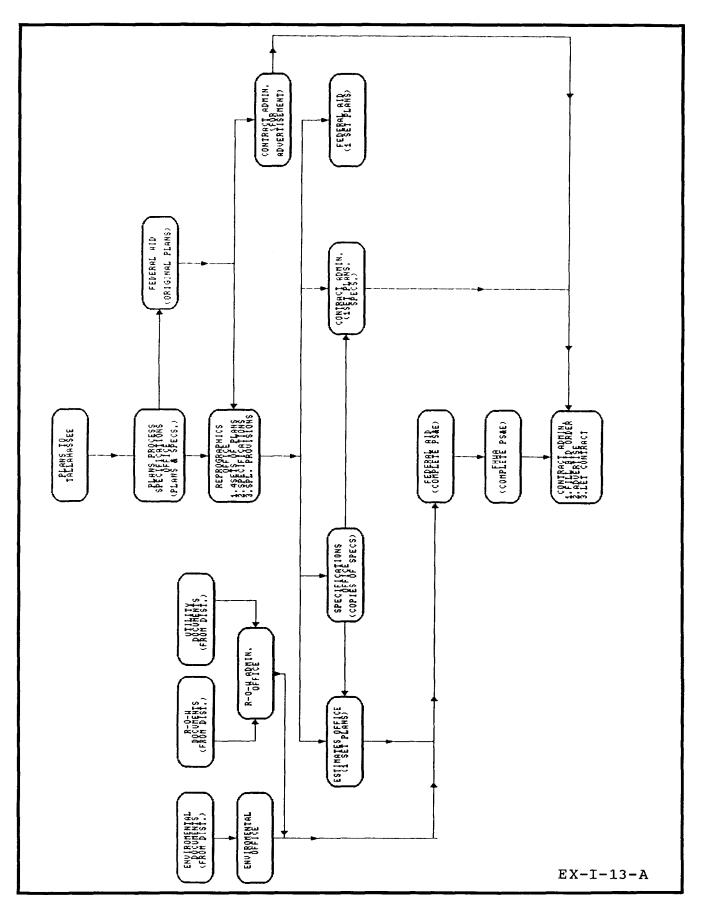
13.2.3 Final Design

The next major phase in the project life cycle includes engineering design and right-of-way acquisition. During this phase of the project, the final and detailed engineering design is completed and plans, specification and contract documents prepared for construction. For most projects, right-of-way acquisition is planned and carried out during this phase of the project. All required permits are also obtained. The various elements of the final design phase are explained in detail in subsequent chapters of this manual.

Engineering and right-of-way activities normally require from 12 to 36 months to complete, depending upon the size and scope of the project.

13.2.4 Project Letting

Project letting is the next phase after final design. Once the design is complete and approved, the contract is let. To prepare a project for letting the DOT prepares specifications, plans packages, project cost estimate, and administers various procurement activities leading up to and including contract execution. These activities are performed primarily in the DOT's central office and require approximately 3 months to complete. Exhibit I-13-A illustrates the process leading up to the letting.



I-13-4.0

13.3 3R Design

Resurfacing, Restoration and Rehabilitation (3R) work is defined as work undertaken to extend the service life of an existing highway and enhance highway safety. This includes the placement of additional surface materials and other work necessary to return an existing roadway to a condition of structural and functional adequacy. It may include resurfacing, pavement structural and joint repair, minor lane and shoulder widening, the removal of parking and restriping as through lanes, removal of median curb to provide a turning lane or 2-way left turn lane, shoulder pavement, alterations to vertical and horizontal curvature, superelevation upgrading, bridge widening, modifications to bridge rails, intersection improvements, the addition of sidewalks and bikeways, utility relocation, removal or shielding of roadside obstacles, modification of side slopes and ditches, drainage modifications, upgrading of at-grade railroad crossings, aesthetic improvements, landscaping, lighting, and signing, signals and pavement markings. Pavement repairs on short segments, and patching and repair of minor pavement failures are considered by FHWA to be routine maintenance and are ineligible for 3R projects. To qualify for Federal funding, a resurfacing structural overlay must be a minimum thickness of 3/4" based on a structural analysis and the 3R project is required to adequately meet existing and probable future traffic needs in a manner conducive to safety, durability and economy of maintenance, within acceptable levels of community and environmental impact. The 3R project must be designed and constructed in a manner that will enhance highway safety and accomplish the foregoing objectives according to the particular needs.

Design criteria and standards to be used for the 3R projects are given in Florida's Design Standards for Resurfacing, Restoration, and Rehabilitation (R-R-R) of Streets and Highways Manual. These standards and criteria are not applicable to interstate projects.

13.4 <u>In-House (DOT) Design</u>

For in-house (DOT) design projects, all activities related to the project are performed by the various sections of the department. The complete design and preparation of the contract documents is accomplished by a team of roadway design personnel lead by the DOT Project Manager/Coordinator. Guidelines for in-house project flow are available in FDOT's Project Management Guidelines.

13.5 Consultant Design

Some design projects are contracted to consultants for design. A DOT project manager/coordinator is assigned to coordinate the project with the consultant and the various sections of the department. At specified stages in the project, the consultant is required to submit his plans to the Department for review. The DOT Project Coordinator/Manager distributes the plans as needed to the various sections for their review and comments. However, it is the consultant's responsibility to throughly check the plans for quality, accuracy and completeness before submitting to the Department. The DOT reviews the plans for compliance with scope, project requirements and progress.

Coordination with utility companies, permitting agencies, railroad companies, and adjacent projects is an important aspect of the design project. Responsibility for this coordination is usually specified in the consultant's scope of services.

At the completion of the design the complete plans package - consisting of the design plans, computations book, engineer's cost estimate, special provisions and other related data - is submitted to the department by the consultant. At this stage, the project is production ready. The consultant's scope of services will define the required number of plan sets and distribution.

13.6 Project Scheduling

As mentioned earlier in this chapter, engineering and right-of-way activities of the final design phase normally take 12 to 36 months. Usually the project letting date is decided well in advance and, hence, other project activities are scheduled accordingly.

A design project undergoes four phase reviews, at 30%, 60%, 90%, and 100% completion. Some small projects (less than \$2,000,000) may undergo only two or three reviews depending upon the scope of work or the district's discretion. District's may also require reviews in addition to the phase reviews.

The 30% plans review, also known as phase I review, is the first milestone in the design of the project. The plans developed at this stage, with all existing utilities, topography, drainage, and other relevant data shown are used for preliminary utility and railroad coordination and review of preliminary proposed roadway geometrics. For complex projects, the preliminary or conceptual traffic control plan is also reviewed at this stage. If bridge structures are included in the project, then this phase also marks the review of the preliminary structural drawings including bridge data sheets and soil borings. Phase I completion of the project triggers other activities such as final drainage design, soil survey, utility contact conference, permit activities, right-of-way maps, and foundation investigation activities for structures.

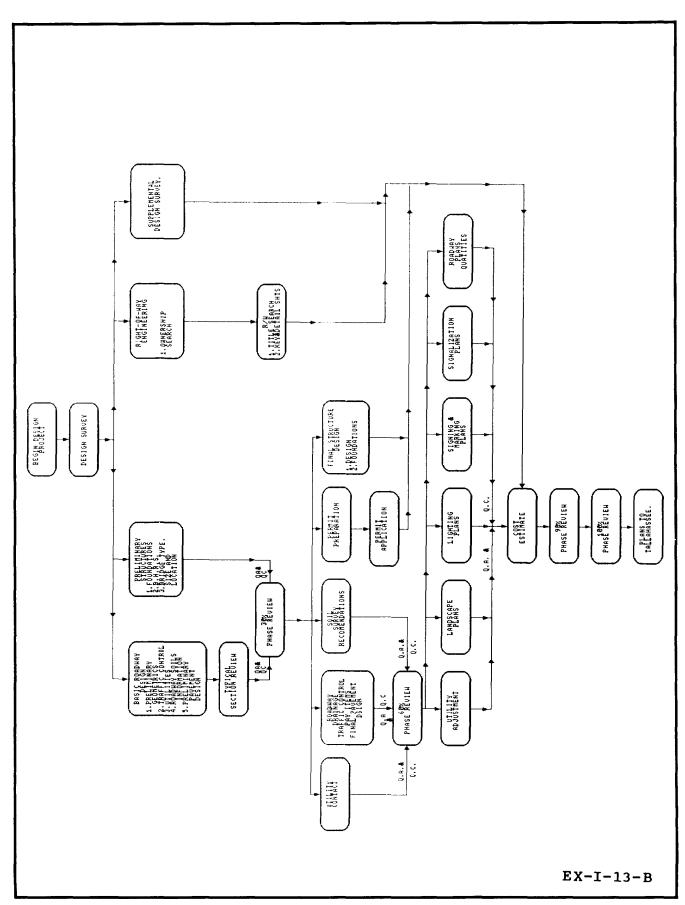
The next milestone, in a design project is the 60% complete, or Phase II review. At the completion of this phase, the geometrics should be final and the drainage design and soil survey are complete. New activities which start at this point are signalization, signing and marking, and roadway lighting design. Other ongoing activities are permits, bridge design, utility contact conference and adjustments, engineer's cost estimate, and right-of-way acquisition.

At Phase III, or 90% complete plans, most activities including quantities, specifications and contract documents are complete, except, in some cases, the permit process and R/W acquisition.

Once the 100% plans are reviewed and approved, and all specifications and contract documents, utility agreements, R/W certifications and required permits are completed, the project is at the PID (Plans in District) stage. At this stage, the transmittal letter can be completed and the plans package transmitted to the central office in Tallahassee. Exhibit EX-I-13-C is an example of the transmittal letter utilized to transmit final plans to Tallahassee.

For a detailed description of plans requirements for each phase, please refer to Chapter 2, Volume II of this manual.

Exhibit I-13-B shows the various activities of the design project.



DATE

то	State Design Engineer				
FROM	District Director of Production				
COPIES TO					
SUBJECT		PI NO. FATE JOB NO. .A. NO.			
	DESCRIPTION: GOES WITH (Other projects in	same contract)			
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1.Road	dway Plans				
	mary of Pay Items				
	ity Plans				
	ning and Pavement Marking Plans		□		
_	nal Plans			밀	_
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7. 5tru	uctures: Bridge Plans Special Structures		님		
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10.0the	er (Please specify)				
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	gn Approval (Location/Design)		-		
	ement Design Calculations		_	00000	
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QAS	Evaluation Form [two (2) copies]		_		
	we Engineering Review				
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* Prel	iminary Engineering Certification		-		

page 1 of 4

EX-I-13-C

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Z.Plan reviews cor	Major projects (>\$2M) Min. 4 re		ä	ä
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CONCO	For consultant designs, by consu		Ħ	
	For consultant designs, plans re		_	_
	with scope, project intent and		ant. 🔲	
Summaries	and checklists placed in project file			
3.100% Bluelines	• • •			
Comments	incorporated in plans			
4.Re: Safety Proj	ect - Design concept reviewed by FHWA			
5.Project require	s: "SPECIAL PROVISIONS FOR CONSTRUCTION M	ETHODS"		000000
	"SPECIAL COMMUNITY AWARENESS ACTIVITIES	S"		
6.Plans signed by	a Registered Professional Engineer			
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A signed	and sealed "Record Set" retained			
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All agreements	are consistent with Special Provisions of	Contract (Initial)		
	Page 2 o	f 4		Utility Engr

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Co.						
JTILITY ADJUSTMENT SCHE						
JTILITY CERTIFICATION TO						
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Railroad](Easements/De	eds)			CLEAR		CLEAR DAT
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Page 3 of 4

EX-I-13-C

		SCHEDULED	COPIES TO
INTENANCE AGREEMENTS		CLEAR DATE	F.A.
*****************		=======================================	\$222222222
Lighting			
Traffic Signals (RR signal pre-emption,	, where required)		
Roadway (where OFF-SYSTEM)			
		SCHEDULED	COPIES TO
ND AGREEMENTS		CLEAR DATE	c.o.
===============		Z===Z\$ZZ==	\$== 12
W/Locals (as required)			
NSTRUCTION ENGINEERING INSPECTION (CEI)			
In-House			
Consultant			
Date: Shortlisted	. Scheduled Execution		
SCHEDULED AWARD OF HIGHWAY CONTRACT :			
Provide recommendation for resolution	if Hwy. contract award	occurs	
before CEI contract Execution (Be spec			
,			
Does this project require any items to	he furnished by the co	ntractor which whom completion	uill be
, - , .	•	•	
retained by the D.O.T. for use outside			
Detour Bridge, Barrier Walls, Computers	s, Equipment, etc.)		
READY FOR PROCESSING TO AN ADVERTISEMENT Name:			
Nama	sia	Project Manager/Designer	
Name:	519	District Director - Productio	
Distribution	NOTE: (1) The following constitutes	
		a COMPLETE PS&E Package suit	able for
Design/Project File		processing to FHWA.	
Production Management		PLANS	
Federal Aid/Project File Etc.		SPECIFICATIONS, INCLUDING:	SPECIAL PROVISIONS, PERMIT COPIES
Ltc.			
		FOTIMATE	UTIL./RR ADJ. SCHEDULES
		ESTIMATE	
		CONTRACT PROPOSAL	
		DIST. CONST. "STATEMENT OF	APPROVAL" OF M. OF T. PLA
		R/W CERTIFICATION	
		UTILITY/RR CERTIFICATION	
		ENVIRONMENTAL RE-EVALUATION	
		MAINT ASSESSED 440 ASSOS	DOTATES
		MAINT. AGREEMENTS (AS APPRO	rkinic)
		PRELIMINARY ENGINEERING CER	·
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Revised: 06/1/89

to FHWA for Authorization.

CHAPTER 14

DATA COLLECTION

14.1 General

Data required for the design of a project are available from various sources in different form. Prior to the start of design of the project, all available relevant data should be collected. The remainder of this chapter explains the various types of data required for the design process and their source.

14.2 Project Data

Project data are the data specific to the project and identify the project.

14.2.1 Project Description

A complete description of the type of project, or facility, to be designed is available in the scope of services. The project length, location, county and milepost, type of facility and proposed modification or construction and total time to completion of the design are key items pertaining to the project description.

14.2.2 Project Number

Each DOT project is assigned a project number and a work program identification (WPI) number. The project number indicates the county of the location of the project, the type of facility and the type of project - i.e., PD&E, or R/W, or construction.

For federal-aid projects, the project has a DOT project number and federal-aid project number.

The project and WPI numbers are shown on the keysheet of the project plans. (See exhibit II-3-A).

14.2.3 Project Cost

The construction cost is estimated during the five-year program phase of the project, and this information is included in the program data documents. As costs are refined the Contract Estimating System (CES) should be updated to reflect the most current estimate. As a minimum, this should be done at each design phase.

14.2.4 Production Schedule

As mentioned in Chapter 13, the project goes through various phases during the design process. Each of these phases marks a production milestone which is of particular importance to the overall schedule of the project. Certain activities in the production schedule are critical and cannot be delayed without jeopardizing the production schedule. Usually, the production schedule is set using two main constraints - the letting date and the total design project time.

14.3 <u>Design Data</u>

Technical data required for the design of a roadway project can be available from various sources in different forms. Some of those major sources and forms are discussed below:

14.3.1 <u>Surveys</u>

- a) Design Survey: A design survey documents the horizontal and vertical alignment, along a baseline or centerline of a project. This alignment is established with reference to fixed horizontal and vertical control points. Locations of other features both natural and man-made are also documented and tied to the baseline or centerline. Horizontal and vertical locations of existing aboveground and underground utilities are also given in the design survey, as described in Chapter 5. Requirements and details of conducting a design survey are given in the DOT Location Survey Manual.
- b) Topographical Survey. This is one of the most important of field surveys. This survey documents all the existing features of the project site such as buildings by type, size and location with respect to baseline of survey, locations of streets and sidewalks, locations and limits of grassed or paved areas, wood lines, fence locations, lakes or ponds, changes in type of cultivation, drainage structures, and breaks in terrain. The Location Survey Manual describes procedures to be used in detail. All information from the topographical survey is input into the CADD file to provide the base data for the project.
- c) Aerial Survey: Another method of obtaining project site existing features information is by aerial photographic survey. The information obtained by aerial photography survey is digitized and used as base data for the project. Good aerial

photo control is very important in order to get accurate aerial mapping. Procedures for establishing horizontal and vertical control points for aerial photo mapping are given in the Department's <u>Location Survey Manual</u>.

Digitized aerial mapping proves to be of great value in the preparation of drainage maps as it effectively records the development of the project vicinity at the time of design as well as the existing ground features.

- d) Drainage Survey: All information pertaining to existing drainage conditions at the project site can be obtained from the drainage survey documents. A good drainage survey records pertinent historical facts related to the drainage of the area, reflects land use trends and any other data that may help in solving drainage problems of the project. Details of survey information required for drainage design are given in Chapter 4 of Volume 2A of the DOT Drainage Manual.
- e) Right-of-Way Survey: Project right-of-way is one of the important factors of the project and should be documented precisely. The R/W survey documents the existing R/W of the project facility. It also documents all public land corners, monuments, property lines, property owners, and complete dimensions of the properties.
- f) Soil Survey: Soils information is required for the design of foundations for structures, pavement design and drainage design. A soils report for the project documents the type and extent of different types of soils encountered within the limits of the project. Physical properties and classifications of the soils together with the soils engineer's recommendations are documented in the soils and foundation report.

14.3.2 Traffic Data

Traffic volumes and/or counts to be used for the design of a new facility or for the improvement of an existing one are obtained from the District Planning and/or Traffic Operations office.

Traffic data are obtained by actual traffic counts on existing roads, roadside interviews and polls of potential users. The ADT (Average Daily Traffic) volume is projected over the expected life of the facility. The total volume and the percent trucks are factors used in determining geometric design criteria, projected truck volume and truck weight data. Traffic counts, classification data, weight-in-motion data and W-4 Tables are used to produce 18 kip equivalent single axle loadings (ESAL), used for pavement design.

14.3.3 Pavement Design

The pavement design should be completed as early as possible. For some projects, it is completed during the PD&E phase and for other projects it is one of the tasks to be completed during the design phase. The pavement type, composition and thickness, are determined using traffic data, projected ESALS, expected life-span of the facility, serviceability, and existing soils information. Details on pavement design can be found in DOT's pavement design manuals.

14.3.4 Environmental Documents

During the PD&E phase of the projects studies are made to estimate the impact of the project on the environment. Factors such as noise generation and its abatement, business and residential relocations, wildlife habitat impact, and wetlands mitigation are documented in the Environmental document.

Information relating to the expected permits required for the construction of the project, and other related data are also contained in the environmental document. For small projects, a formal EIS may not be available, but all environmental data is usually available in the form of environmental reports. The environmental document may commit the State to certain design features. The designer should carefully review these documents to determine what design commitments have been made. Significant changes to the scope of the project may require reevaluation of the environmental factors.

14.3.5 Original Plans

Original plans of existing facilities requiring reconstruction prove to be a very good source of information. Information pertaining to existing alignment - both horizontal and vertical - and R/W can be helpful in the design of the new facility.

14.3.6 Accident Data

Accident data are provided by the District Safety Engineer. These data should be requested for all projects, but are especially useful for the design of 3R, safety and intersection projects and justification of signalized intersections.

14.4 List of Requests and Contacts

During the design process, the design engineer will require various items of information from different sections or departments. The following is a list of some of those items and their source:

A) Planning and Programs

- 1.) Request flexible pavement design (18 KIPS)
- Request traffic data (motorized)
- 3.) Request projected traffic data for intersections
- 5.) Request updates of traffic data (as needed)
- 6.) Railroad contact (30%)
- 7.) Railroad contact (90%)
- 8.) Plans transmittal letter data (railroad)

B) Location

1.) Request survey

C) Traffic Plans

- 1.) Request turns and counts for intersection design
- 3.) Notification that project includes milling
- 4.) Signing and pavement marking plans (30%)
- 5.) Signing and pavement marking plans (60%)
- 6.) Signing and pavement marking plans (90%)
- 7.) Traffic signal plans (30%)
- 8.) Traffic signal plans (60%)
- 9.) Traffic signal plans (90%)
- 10.) Lighting plans (30%)
- 11.) Lighting plans (60%)
- 12.) Significant review of accident data
- 13.) Safety review of design plans
- 14.) Response to safety review of design plans
- 15.) Operational and capacity review of design plans

D) Soils and Foundations

- 1.) Request pavement design LBR
- 2.) Request roadway soil survey
- 3.) Submit cross-sections for plotting of soils data
- 4.) Request foundation investigations
- 5.) Request pavement composition and milling recommendations
- 6.) Request dynaflect testing.
- 7.) PH and soils resistivity for culvert material selection.

E) Drainage

- 1.) Pavement design comments
- 2.) Request grade and high water review
- 3.) Request drainage design
- 4.) Request final drainage review
- 5.) Culvert material selection
- 6.) Permit review

F) Maintenance

- 1.) Pavement design comments
- 2.) 30% Plans review
- 3.) 60% Plans review
- 4.) 90% Plans review
- 5.) Response to 30% plans review
- 6.) Response to 60% plans review
- 7.) Response to 90% plans review

G) Construction

- 1.) Pavement design comments
- 2.) 30% Plans review
- 3.) 60% Plans review
- 4.) 90% Plans review
- 5.) Response to 30% plans review
- 6.) Response to 60% plans review
- 7.) Response to 90% plans review
- 8.) Submit traffic control plan request
- 9.) Transmit marked-up utility adjustment plans

H) R/W Engineering

- 1.) Submit title search request
- 2.) Request existing right-of-way maps.
- 3.) Transmit right-of-way requirements
- 4.) Final right-of-way check
- 5.) Plans transmittal letter data

I) Utilities

- 1.) Preliminary (First) contact (30%)
- 2.) Horizontal and vertical verification of utilities.
- 3.) Pre-Design conference and contact (60%)
- 4.) Final contact (90%)
- 5.) Plans transmittal letter data (utilities)
- 6.) Number of sets of final prints for utility companies

J) Estimates and Specifications

- 1.) Preliminary estimate and time (IRE)
- 2.) Preliminary estimate and time (30%)
- 3.) Preliminary estimate and time (60%)
- 4.) Preliminary estimate and time (90%)
- 5.) Complete estimate and contract time (100%)

K) Right-Of-Way Department

- 1.) Project schedule updates as needed
- 2.) R/W estimates as needed
- 3.) Pre-Proposal appraisal conference
- 4.) Field questions from R/W agents as needed
- 5.) Plans transmittal letter data
- 6.) Hazardous waste determination

N) FHWA

- 1.) 30% Plans review
- 2.) 60% Plans review
- 3.) 90% Plans review

- 4.) 100% Plans Review
- 5.) Submit for typical section approval
- 6.) Submit for pavement design approval
- 7.) Response to 30% plans review
- 8.) Response to 60% plans review
- 9.) Response to 90% plans review
- 10.) Submit exception request letters
- O) Value Engineering (2,000,000+)
 - 1.) 30% review
 - 2.) 60% review

Exhibits EX-I-14-A thru I are examples of some request forms utilized in obtaining the various information items required for design.

DATE:	
TO:	Highway Statistics Engineer
FROM:	
COPIES TO:	
SUBJECT:	State Road No
	W.P.I. No
	State Project No
	F.A.P. No
	County
	Description
	*Description
	Limits of Job
	Budget Construction Year
	Year Open to Traffic
	No. of Lanes, Existing
	No. of Lanes, Proposed
	Type of Facility
	(e.g. major, intermediate, or minor arterial,
	freeway, expressway, city street, rural road, ramp)
	System Name
	(Interstate, FAU, FAS, etc.)
	Pavement Type
	Type of Work
	Please forward this Design Section, the following information
	for the subject project:
	A. <u>Traffic Parameters</u> requested are:
	1. ADT's
	2. K, D & T Factors
	a. T Factor Breakdown
	1. % of ADT
	B. Equivalent 18 KIP Loadings
	C. Years
	1 Current Year
	2 Construction Year
	3 Mid-year (10 years from opening)
	4 Design year (20 years from opening)
	EX-I-14-A

DAT	E:
ı	O: Soils Engineer
FRO	M:
COPIES T	0:
SUBJEC	T: State Road No
	W.P.I. No
	State Project
	F.A.P. No
	County
	Description
Transmit	ted herewith are prints of Plan and Profile Sheets and Cross
Section	Sheets on the subject project, indicating proposed alignment,
gradient	and flow line of proposed structures.
(For Wid	dening and/or Resurfacing, Key Map and Typical Section Sheets
only).	
This is	for your use in obtaining the following:
()	Soils Survey
()	Percolation Test
()	Water Samples
()	Thickness and Type of Asphalt
()	Thickness and Type of Base
()	Thickness and L.B.R. Value of Existing Subgrade
()	Design L.B.R.
()	Amount and Type of Leveling Recommended
()	Milling Recommendation
()	Soil Boring (For Structure)
()	Pavement Composition
Plea	ase furnish this information by
i e	tional information is required, please feel free to contact
this of	fiœ.
Fredomin	~ EX-I-14-B

SURVEY REQUEST PROJECT NO. _____ B.I. NO. _____ PROJECT DESCRIPTION_____ LIMITS SURVEY REQUEST: 1. ALIGNMENT 2. TOPO 3. R/W 4. CROSS SECTION 5. OUTFALLS 6. SIDE STREETS 7. UTILITIES 8. OTHER EX-I-14-C

1 of 2

Charges can be made to Project No.	
Comments:	
Requested by	Date
	EV_T_14_C
	EX-I-14-C 2 of 2

DATE:

TO: Materials Engineer

FROM:

COPIES TO:

SUBJECT: State Road No....

W.P.I. No.....

State Project No ..

F.A.P. No.....

County.....

Description.....

Enclosed for your use are Key Map Cross Sections and marked prints of Plan and Profile Sheets for boring locations on the subject project.

If additional information is required, please feel free to contact this office.

Enclosures

EX-I-14-D

DATE: TO: FROM: COPIES TO:	Environmental Permit Coordinator
SUBJECT	PERMIT SKETCH
	State Road No W.P.I. No State Project No F.A.P. No County Description Transmitted herewith are the original permit sketches for the above referenced project. The following information is included to enable you to apply for the necessary permits: 1. Production date
	6. Brief description of project:
	If additional information is required, please feel free to contact this office.
Enclosures	

EX-1-14-E

DATE: TO: FROM: COPIES TO:	District Railroad Coordinator
SUBJECT:	State Road No
	W.P.I. No
	State Project No
	F.A.P. No
	County
	Description
	Submitted this date is Sheet of the signalization plans for your handling with the railroad.
	The following work is proposed within railroad right-of-way.
	If additional information is required, please feel free to contact this office.
Enclosure	

EX-I-14-F

DATE:	
TO:	Utilities Engineer
FROM:	
COPIES TO:	
SUBJECT	UTILITIES TRANSMITTAL
	State Road No
	W.P.I. No
	State Project No
	F.A.P. No
	County
	Description
	Transmitted herewith are sets for your use in the
	coordination of utilities for the subject project.
	If additional information is required, please feel free
	to contact this office.
Enclosures	

EX-I-14-G

DATE:	
TO:	Construction Engineer
FROM:	
COPIES TO:	
SUBJECT:	State Road No
	W.P.I. No
	State Project No
	F.A.P. No
	County
	Description
	Transmitted herewith is the Record Computation Booklet
	for the above referenced project.
	Please note the following comments:
	1. This project is scheduled for a
	production.
	2. The district Construction Engineer recommends a contract time of calendar days for this project.
i	3. The record prints are on file in the District Office.
	If additional information is required, please feel free to
	contact this office.
Enclosure	

BUREAU OF TOPOGRAPHY FLORIDA DEPARTMENT OF TRANSPORTATION

SUPPLEMENTAL WORK REQUEST FORM FOR CADD AND CROSS SECTION PROJECTS

(ATTACH TO WORK REQUEST FORM PD 100) #87-1417

				s	TATE JOB NO	D.:		
REQUESTED B	Y:		DATE:					
DELIVER TO:								
DIGITIZED TOPOGRAPHY			1" = 20'	1" = 40'	1" = 50'	OTHER		
SCALE OR SC DESIRED:	ALES							
DEGINED.								
			LEFT	LIMITS	RIG	HT LIMITS		
LIMITS OF					THOUSE ENGINE			
COMPILATIO	N:							
<u>AS HORIZ. AL</u>	IGNMENT BEEN	N CODED I	N ROADS D	ESIGN FILE?	YES	NO		
•	IGNMENT BEEN				YES	NO		
AME OF PERS		E OF ROAL	OS FILE:		YES			
IAME OF PERS	SON IN CHARGE	e of Road OSS Secti	OS FILE: ONS DESIR		YES	NO		
IAME OF PERS	SON IN CHARGE	e of Road OSS Secti	OS FILE: ONS DESIR	ED:	YES	NO		
IAME OF PERS	SON IN CHARGE AMMETRIC CRO RLINE INTERVAL	e of Road OSS Secti	OS FILE: ONS DESIR	ED:	YES	NO		
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Chapter 15

Scheduled Submittals

15.1 General

All major projects have phase reviews at 30%, 60%, 90% complete and a final plans review at 100% prior to transmittal to the Central office in Tallahassee. Minor projects, resurfacing and other projects with a construction cost of less than \$2,000,000 will have as a minimum the 60% and the 100% complete reviews.

For the departments in-house projects, the review should be done by a squad different from the one doing the design. As mentioned elsewhere in this manual, for consultant prepared projects the consultants are held responsible for their work, including plans review. The designer (department and consultant) shall review the project by following and completing the checklist shown in the "Design Training Aid Manual." A completed copy of the appropriate checklist and other review coments shall be submitted with the plans for a formal review by the department.

15.2 <u>Design Plans Phase Review Submittal</u>

On minor projects, less than \$2,000,000, reviews will be held at the 60-100% complete stages. One of these reviews must be an on-site review. These reviews will include personnel from Design, Construction, Maintenance, FHWA (for Federal Aid Projects) and any other department representatives who can provide review input. It is desirable to have an on-site visit by the designer before design starts and the more formal on-site review at 90% to ensure the design is appropriate and that no physical features have been altered or added.

On major projects, those greater than \$2,000,000, reviews must be held at the 30-60-90% stages as well as the 100% final check. Two on-site reviews will be required. Generally these are held at 30% and 90% complete with personnel from the offices noted under minor project reviews. Also, an on-site visit before design by the senior designer is desirable.

For phase reviews which are not held on-site, construction, maintenance and FWHA (for federal aid projects) offices must be sent copies of the plans and requested to furnish comments.

An additional update review, including an on-site review, will be required on all projects that have been delayed and shelved for more than nine months since the final 100% review.

The District Safety Engineer should be given an opportunity to be included in these reviews. The use of ancient history is required on all reconstruction, intersection improvements and 3R type projects to ensure that all accident problems have been addressed.

The District Project Manager/Coordinator is responsible for ensuring that prints are distributed to the appropriate personnel and that the above reviews are held on each project. 1/2 size prints may be used for reviews, at the District's option.

For consultant projects each review will require the submittal of five sets of bluelines and one set of reproducibles of the plans. Districts which would like to specify a different number of prints, or use 1/2 size, must do so in the consultants scope of services.

For detailed requirements of each phase review submittal, please refer to Chapter 2 of Volume II of this manual.

5.2.1 Plans Disposition

As discussed in Chapter 13 of this Volume, each phase of the plans preparation triggers other activities in the process of the project's design life cycle.

The 30% complete plans are used for the initial contact with the utility companies. The alignment, horizontal and vertical, and typical sections are checked for compliance with design criteria, project and site requirements, compatibility with adjacent projects and drainage requirements.

The 60% plans are used for the signing and marking design, roadway lighting design, signalization design, traffic control sheets, other component plans preparation and permit package preparation. Usually at this stage of the project a utility predesign conference also occurs. CES data input is recommended at the completion of 60% plans.

At 90% complete phase all plans are usually complete including the cost estimate. After the 100% complete phase a record set of plans are signed, sealed and dated, and the originals signed and placed in PID Status to be transmitted to the Central Office in Tallahassee.

The disposition of the plans after they are transmitted to the Central Office is shown in Exhibit EX-I-13-A in Chapter 13.

15.3 Other Submittals and Requests

Other documents, besides the roadway plans, of a project also go through reviews at various stages of the design as listed below:

15.3.1 Structures

Structures design begins as the foundation investigation is complete and on a schedule which permits preliminary bridge plans and the 30% roadway plans to be completed simultaneously. In the case of a stream or river crossing, the drainage design must also be complete prior to beginning structures design. All structures design work is coordinated through District Structures Engineer and/or the State Structural Engineer's Office in the Central Office. A typical section of the facility crossing should be determined prior to beginning structures design along with horizontal and vertical clearances required to establish the profile grade.

Generally, the completion and review of bridge plans is completed in three phases as listed below.

Preliminary - plan and elevation Near Final (90%) Final (100%)

In order to prevent problems and to "Do it right the first time," the following procedures should be used:

Typical Section Package - The typical section package should include a section showing width of bridge that is to be provided for all urban bridges. A section is necessary for rural projects when the design is different from the standard widths, i.e., bicycle facilities, sidewalks,

fishing walks, etc. For standard rural sections, the bridge width should be indicated by a note on the roadway typical: Example: "Clear Bridge Width = 44'; 24' roadway with 10' shoulders." The Structural Design Office will not proceed with bridge design until the typical section has been approved.

Request for Structural Design - (Bridges and Retaining Walls) All requests for structural design should include plan and profile sheets showing horizontal and vertical alignment and cross sections within 500 feet on each side of the bridge or ends of retaining walls. Horizontal curvature that is on or near the end of the bridge or retaining wall must be shown.

Superelevation transition (runoff) details must be included with the transmittal if any part or all of the transition is on the bridge or wall. The approved typical section should be included with the transmittal.

Provisions for access to property near the end of bridges and adjustments to avoid costly right-of-way takings should be resolved prior to submittal. An attempt should be made to avoid horizontal and vertical curvature on the bridge without sacrificing safety. Superelevation transition lengths may be appropriately adjusted.

Coordination of Final Plans - The District will request prints of bridge plans prior to submittal of the final roadway plans to Tallahassee to ensure roadway and bridge plans are consistent, i.e., widths, superelevation transitions, vertical and horizontal alignment. The Central Office Area Design Engineers will be available to assist in coordination between District Roadway Design and Central Office Structural Design when plan development is being performed by the Central Office but will not be able to verify final plan submittals.

In addition to bridge plans, structures plans may include retaining walls, noise barrier walls, box culverts, pedestrian overpasses, and special structural appurtenances.

For projects where bridges <u>and</u> retaining walls are involved, preliminary and final wall plans (usually along with bridge plans) should be sent to the State Structural Engineer's Office and then forwarded to the FWHA Bridge Section for further review.

For projects where retaining walls are required along with roadway plans only (no bridge in the project), DOT District Offices shall be responsible for development of wall plans, proper reviews and submittal of preliminary and final plans to FHWA.

Where proprietary walls are involved, an invitation to participate in alternate designs is submitted to DOT approved wall companies. This will be the responsibility of the consultant or the DOT Office responsible for development of wall plans.

After appropriate structural and geotechnical review, the wall plans are submitted directly to the FWHA at the 30% and 90% review phases. The Florida DOT District Office is responsible for the structural and geotechnical review prior to sending these plans to the FHWA. The submittal of preliminary plans (30%) to FHWA should occur at or near the time that the invitation to participate is submitted to the wall companies.

Where the District Office cannot carry out the structural review or verify the review as proper by a consultant, such review may be requested from the Structural Engineer's Office. The wall companies only claim responsibility for their system, the wall and soil mass engaged - thus the geotechnical review must include analysis of the boundary soil conditions.

15.3.2 Surveys

Most field surveys will be completed before the start of the design since the survey information is needed for the design. The Roadway soils survey is normally completed between the 30% and 60% Roadway plans review.

All bridge data sheets and channel alignment should be completed during this phase.

15.3.3 Typical Sections

For most projects, the typical section is approved during the PD & E phase. For Final design projects that require approval of typical sections, the typical section package should be submitted before the 30% review to allow ample time for approval. Typical sections shall be submitted on 8 1/2 x 14 sheets, approved, signed and sealed by the responsible engineer. Space must be provided for concurrence by the DOT and, for federal-aid projects, by FHWA. Exhibits EX-I-15-A thru EX-I-15-D are Samples of Typical Section Data Sheets Utilized for Typical Section approval. Exhibits EX-I-15-A and C are to be used when prepared by a consultant for the Department. Exhibits EX-I-15-B and D are to be used when prepared in-house.

When preparing typical section packages, only typical sections for the main roadway and/or bridge is necessary. Minor variations to these do not need typical sections. All side street tie-ins, etc., can be handled as part of the routine plan review process.

TYPICAL SECTION DATA SHEET

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15.3.4 Pavement Design

For projects that require pavement design in the design phase, the pavement design package should be submitted as early as possible. For some projects, a preliminary pavement design is approved at or before the 30% review with the final pavement design being approved at the 60% completion stage.

Pavement design packages sent to the FHWA for approval should only be for the major items of the main roadway. Minor variations can be handled as part of the normal plans review process.

15.3.5 Permits

The required permits will be determined during Project development and Environmental Study phase. The District permitting office is responsible for coordinating and processing the permits and must be contacted early in the project to determine the level of detail needed in the permit packages. The DOT Project Manager/Coordinator must communicate this information to the appropriate office preparing the permit packages. In general, permit approval is a lengthy process and permit applications must be submitted early enough so that the construction schedule will not be delayed due to permits being processed.

The permitting agencies and the estimated processing time frame should be listed in the overall project schedule developed by the Project Manager/Coordinator.

Agencies with permitting authority are listed below:

Florida Department of Environmental Regulation

U.S. Coast Guard

Local Environmental Agencies

U.S. Army Corps of Engineers

Florida Department of Natural Resources - Bureau of Beaches and Shores.

Agencies with an interest but no permitting authority are listed below:

Florida Department of Natural Resources - Bureau of Beaches and Shores

Florida Game and Freshwater Fish Commission - Office of Environmental Services

Florida Department of Veterans and Community Affairs -Bureau of Land and Water Management

U.S. Fish and Wildlife Services

National Marine Fisheries Service - Environmental Assessment Branch

Regional Planning Councils

Local Governments and Transportation Authorities

U.S. Environmental Protection Agency

15.3.6 Right of Way Engineering

During right-of-way preparation there are typically four phases as described below. All map preparation procedures shall be in accordance with the DOT R/W Survey and Mapping Manual.

Preliminary (30%) -	invol	ves	project		alignment,		curve
	data, section o		cor	ners,	cross	road	
	alignments.						

Intermediate (60%) - revisions from preliminary submittal, existing right-of-way, subdivisions, interchanges, proposed right-of-way with takings dimensioned.

Near Final (90%) - revisions from intermediate submittal, easements, tabulation of ownerships, right-of-way requirements for drainage easements.

Final (100%) - revisions from near final submittal, complete right-of-way requirements in accordance with the DOT Procedure 575-010-001 thru 007.

Right-of-way maps are prepared in accordance with the procedures as described in the Department's <u>Right-of-Way Surveying and Mapping Manual</u>. Information is added to the right-of-way maps as it becomes available and close coordination is required between the design and right-of-way sections. R/W requirements should be well identified on roadway plans as early as possible. Final requirements must be identified by 60% completion of roadway plans and transmitted to R/W office. Miscellaneous requirements for such items as drainage structures and corner clips adjustments may be made at later phases.

Title search should be ordered as soon as the right-of-way limits are reasonably established. It is necessary, particularly on urban projects, to obtain ownership search to assist in the establishment of the right-of-way limits. The ownership search may be conducted by the District Right-of-Way Surveying and Mapping office or the design consultant.

Access road and drainage (outfalls) requirements are generally the last requirements to be established in the R/W phase and must not be overlooked. They should be established in the PD&E phase when possible and adjustments made as necessary. Access road studies (justification) are required on limited access and federal aid projects and will necessitate appraisals. The appraisals and justification are developed by Department personnel or by fee appraisers. This activity takes time and therefore should be requested as soon as possible. Appraisals have an age limit; therefore, they must be compatible with the acquisition schedule.

The instruments are prepared by the Department, although the legal descriptions may be prepared by the design consultant.

It is important to note that, on federal-aid projects, the formal appraisal activity cannot begin until the final right-of-way map is approved and the function is programmed with the FHWA. The right-of-way requirements must be made available for R/W map preparation at the earliest possible time to facilitate the timely acquisition of title searches.

Occupancy permit requirements must be defined by the 100% plans completion stage so they can be obtained by the right-of-way office.

15.3.7 Value Engineering

Any FDOT project with a construction cost in excess of \$2,000,000 may be selected for a value engineering review. The Value Engineering Coordinator will screen and select projects based on cost saving potential. The Project Manager and/or Designer should expect recommendations from the value engineering team that will need to be incorporated into the design. However, at this stage of project development, basic concepts established during the PD&E phase will not be altered except in extraordinary circumstances.

Once a project has been selected for a value engineering review, it is the Project Manager's/Designer's responsibility to ensure the review is conducted in a time frame that allows maximum opportunity for value improvement considerations and still maintain proper project progress. Value engineering reviews shall be conducted in accordance with the Value Engineering Design Review Procedure No. 625-030-002a. As such, value engineering reviews should occur no later than 30% plans completion. Accepted design changes can be accomplished with minimal impact of project schedule if value engineering recommendations are provided early in the design phase.

VALUE ENGINEERING TEAM

The Project Manager, as the leader of the design effort, should be available to the value engineering team to provide input concerning project decisions and data. When the project is being developed by a consultant, a representative of the consultant should be available to present to the team, assumptions and data for project decisions. The Project Manager should maintain close communications with the value engineering team leader as the study progresses, continually providing input for consideration by the team.

VALUE ENGINEERING FINDINGS

After completion of the value engineering team study, a draft summary report will be prepared. The Project Manager should make a thorough review of the findings with the team's input. Questions that should be asked are:

- o Do the recommendations achieve the objectives of the project?
- o Have all design and existing conditions been considered?
- o Are all changes suggested economically feasible?
- o Will the project schedule be affected by the changes? Are the delays justified by the savings resulting from the changes?
- o Are the changes consistent with agreements with the local community or citizen groups?
- o Will additional public meetings be required?
- o Are any design controls altered? Are they acceptable?
- o Do the recommendations provide a long term advantage or short term solutions?
- o Will the recommended changes improve the project?

The purpose of this session is to afford the Project Manager with a "first" review of the team's recommendations, and to evaluate with the team, their potential implementation. The Project Manager, being the most knowledgeable person of the project specifics and objectives, should use this opportunity to ensure that recommended changes are consistent with project intent.

If the Project Manager and the value engineering team are in agreement, the value engineering summary study should be finalized and distributed to all persons concerned with the project, along with written concurrence of implementation by the

Project Manager. In the event that the Project Manager and value engineering team cannot reach an agreement, a presentation to management will be scheduled. At this meeting, the value engineering team will present their recommendations. The Project Manager will present his evaluation of the value engineering recommendations.

At the conclusion of the management presentation, a decision should be made to accept all, or a portion of, the value engineering recommendations. Having this decision, the Project Manager should inform, in writing, all persons involved in the project of the recommendations accepted for the continuation of the work.

Chapter 16 PLANS REVIEW

16.1 General

Plans prepared for a design project shall be reviewed periodically to ensure accuracy, proper engineering criteria application, completion and quality. A checklist should be followed for the review process to assure a complete review. Various checklists and their application are described in <u>Design Training Aids</u> manual, Design Checklists.

16.2 Design Plans Phase Review Checklist

As mentioned in Section 15.2 of this volume, major projects shall undergo four formal reviews at the 30%, 60%, 90% and the 100% completion stages of the project. The design office shall conduct these reviews by following the checklists as shown in The Design Training Aids manual. A checkmark shall be made by the reviewer at the appropriate item on the checklist. Upon completion of the checklist, it should be signed and dated by the designer and a copy of the completed checklist shall be submitted to the DOT District Office with the formal review plans set. The above procedure shall be followed for all phase reviews of minor projects also.

The Project Manager/Coordinator is responsible for ensuring that the computerized design checklists are obtained by the design organization. Consultants that have on-line access to the CES may pull the checklists directly.

16.3 3R Projects Checklist

3R (Restoration, Rehabilitation and Resurfacing) projects have different requirements than design projects, hence the checklist for phase review of 3R projects is different. For a complete discussion of 3R project requirements, please refer to the current 3R manual.

16.4 Field Reviews

As mentioned in Chapter 15 of this volume, all projects will have at least two field reviews. For major projects, a minimum of three field reviews, including the pre-design field review, are desirable. An additional update field review shall be required for all projects delayed and shelved for more than nine months since the final 100% review.

Chapter 17

QUALITY ASSURANCE

17.1 General

All offices under the State Transportation Engineer, have developed a Quality Assurance Plan. These plans can be found in the Transportation Engineering Quality Assurance Plan, 1989. The Roadway Design plan is designed to ensure districts are providing quality designs and control; to point out areas that need improvement; to ensure the effectiveness and appropriateness of established policies, procedures and standards; and to share between districts, outstanding quality successes and concerns before they become areas of noncompliance.

The plan designates design functions to be monitored on an annual basis, latitude is granted to the reviewer for the depth of each review, based on the individual districts observed performance or need.

While the plan covers the main functional roadway design areas, the designers and reviewers are reminded that quality is the result of doing many individual activities in conformance with requirements, no matter how large or small their contribution (such as drafting technique, plans readability, etc.) to the overall objective.

17.2 <u>Authority</u>

The Florida Statutes (20.23(3)) specifically states that the Department shall ensure quality and monitor implementation of policies and procedures.

17.3 Areas of Responsibility

- A) <u>Central Office Role—Quality Assurance</u>. This is all planned and systematic actions necessary to provide adequate direction to the districts so that all resulting design products can meet predetermined requirements. This involves the establishment of design policies, procedures, standards, guidelines, training and the monitoring and review of district compliance with these items.
- B) <u>District Role--Quality Control</u>. Following established design policies, procedures, standards and guidelines in the preparation of all design products; the checking and review of individual designs for compliance and good engineering practice.
- C) <u>Area Design Engineer (ADE)</u>. Manage the Quality Assurance plan, document findings and maintain continuity with the department on design policy, procedures, standards and quidelines.

17.4 <u>Design Functions</u>

Currently, the Roadway Quality Assurance plan will monitor the following functions:

- 1. Geometric Design
- 2. Traffic Design
- 3. Pavement Design
- 4. Drainage
- 5. Preliminary Estimates and Specifications

The plan includes sub-functional areas to be reviewed under each function. The functions to be monitored can be revised to better suit the Department's need. Refer to the current plan for current functional areas to be monitored.

17.5 Documentation

- A. <u>Documentation</u> The Quality Assurance process will be documented in a quarterly report that will be distributed to the District Secretaries and other affected offices.
- B <u>Area Design Engineer (ADE).</u> An ADE is assigned to each district and will be responsible for conducting and/or coordinating all QA activities with that district.
- Review. A review will be conducted on each design function and its associated components a minimum of once annually. This review will be for assurance that the districts are complying with policy, procedures, standards and guidelines and the identification of any needs, excellent or noncompliance areas.

17.6 <u>Consultants' Role</u>

The consultants role is much the same as the districts' (Quality Control) since their primary responsibility is the preparation of contract plans as agents for the districts. Consultants must ensure quality and adherence to policies just as must any office with the Department. The Department no longer provides detailed reviews of consultants work and the consultant firms are held responsible for the quality, accuracy and completeness of the work supplied to the Department.

17.7 Plans Processing

The plans package, when complete in the District, is mailed to the State Roadway Design Engineer's Office in Tallahassee. This is the first step in the plans processing for letting.

The ADEs have no specific role in the plans processing. Necessary revisions are the responsibility of the District and no engineering revisions will be made by Central Office staff.

The Plans Processing Section is responsible for initiating the processing of each set of plans once received. The staff in this section conducts a short checklist to ensure completeness, as shown in Exhibit I-17-A. If a non-engineering revision or a District initiated revision is required, this section will coordinate the change or send plans back to the District and/or provide space in Tallahassee for the District to come and make the changes. The ADE is to be used as the District's contact on revisions only when special handling or time constraints dictate a higher level of urgency or complexity.

Once the plans are processed in the Roadway Design Office, they are sent to the Specifications Office; to the Reprographics Office for printing; copies are forwarded to Estimates for preparation of the official estimate; to Federal-Aid Office for Plans, Specifications and Estimates PS&E package preparation; and to the Contracts Office for preparation of the bidding documents and advertisements. The time frame, for processing the plans through the Central Office to receiving bids, is approximately three months (two months to process and one month for advertising).

Exhibit I-17-A

PLANS PROCESSING PROCEDURE

W.P.	I. No.:
Stat	e Project No.:
F.A.	Project No.:
Proc	essed by: Date:
 1.	Log plans in.
 2.	Check Plans Transmittal Memo for signature of District
	Director of Production or the District Deputy Assistant
	Secretary.
 3.	Obtain structure plans and include in package if
	requested by District.
 4.	Order metal plates of photo sheets.
 5.	Upon receipt of metal plates, return originals to
	plans.
 6.	Trainees included (F.A. Projects Only).
7.	Log plans out and deliver plans and file to
	Specifications Office.

EX-I-17-A

Chapter 18

PAY ITEMS

18.1 <u>General</u>

Project cost estimating and the establishment of contract time are important phases of the design project and occur after the roadway design is complete. This chapter briefly describes the various requirements for the completion of the Bid Package.

18.2 Pay Items

Pay Items are the various roadway construction elements for which payment is made to the contractor. A method of measurement and payment is established for each separate construction element of the roadway design. The <u>Basis of Estimate Manual</u> contains the description, item number and unit of measurement of each construction element. This manual also explains the establishment of pay item numbers for each construction element.

As previously mentioned in Volume II of this manual, pay items are identified at the 60% completion of the project and quantities added at the 90% completion. At the final or 100% completion of the project, all pay items are finalized and the final Summary of Pay Items is obtained.

18.3 CES

The purpose of the Contract Estimating System (CES) is to compile and produce an accurate contract cost estimate in the same manner as that of a contractor preparing his bid.

For detailed procedures on the use of the CES program, please refer to "Contract Estimating System, District Manual."

18.4 Computations Book

The computations book contains all computations required to calculate pay item quantities required for the project. All computations should be done on relevant computation forms as described in the <u>Basis of Estimates Manual</u> and completed as required in the <u>Computations Manual</u>. The computations book supports the pay item quantities and is required to be submitted with the 90% and the 100% phase review submittals.

18.5 Contract Time

After the 90% completion of the design project including the completion of the cost estimate, the plans package is submitted to either the district construction or design department of the DOT for establishing the contract duration. Contract duration - time required for the complete construction of the project - is calculated on a job to job basis. Computation Procedures and forms used to estimate contract time are available in the DOT <u>Guidelines For Establishing Contract Duration</u>. A copy of the contract time calculations is submitted to the Central Office in Tallahassee with the complete package. Certain large complex projects should have the desired contract duration established early in the design process. The project design and plans should be developed so that the construction can be accommodated within the allotted time.

18.6 Shop Drawings

For non-structural shop drawings, the Engineer of Record that will be responsible for the review must be listed on the key sheet of the specific plans, along with his/her mailing address.

Example:

Engineer of Record
Lighting Shop Drawings
John Doe, P.E.
111 S. Avery Street
Tallahassee, Florida 32301

Chapter 19

SIGNING AND SEALING DESIGN DRAWINGS

19.1 General

The Florida State Board of Professional Engineers has reviewed this Chapter and is in concurrence with its requirements. To assure continued concurrence, the Board will have the opportunity to review future changes.

Section 334.175 of the Florida Statutes, requires that all design drawings prepared by or for the Department be signed, sealed and certified by the professional engineer in responsible charge of the project work, in accordance with Chapter 471, Rules 21H-19, 21H-23 and 21H-26. Such professional engineer must be duly registered in this state. Responsible charge means direct control and personal supervision of engineering work done by oneself or by others over whom the engineer exercises supervisory authority. This chapter will outline the proper procedures of signing and sealing the Department's drawings.

The Department is currently transitioning from manually prepared drawings to fully automated (Computer Aided Design and Drafting - CADD) drawings. This chapter will set forth the appropriate quidelines for signing and sealing both types of drawings.

19.2 Signing and Sealing of Plans

19.2.1 <u>Signing and Sealing of Manually Drafted or Partially CADD</u> Produced Plans

Original Plan Sheets: - Each original plan sheet shall have the title block completely filled out. Initials may be used for each block except for the "Approved by", which shall be the signature of the responsible professional engineer. The "Approved by" will be either the responsible in-house professional engineer or the responsible consultant professional engineer, whichever applies. This may be printed, leroyed or by signature.

The key sheet for each component set (i.e., roadway, signing, etc.) shall have the responsible professional engineer's name printed, leroyed or by signature. The following format shall be used:

 lans Approved By:	Roadway
 Date:	
P.E. No:	

For the other components, "Roadway" should be substituted with the title of the component set, such as "Signing and Marking".

Official Record Set (Manual): - An official record set must be signed and sealed by the professional engineer in responsible charge. For in-house projects, one official record set is to be kept by the responsible professional engineer.

For Consultant projects, one record set is provided to the Department and one record set is to be kept by the Consultant.

The official record set shall have each sheet signed and sealed by the responsible professional engineer. The signature shall have the date immediately under the signature and the embossed seal placed over the signature and date. The professional engineer's registration number under the date is required only on the key sheet.

The official record set shall be a full size copy (bluelines, blacklines, etc.) of the originals. Seals are not to be placed on originals.

19.2.2 Signing and Sealing of CADD (Automated) Produced Plans

Original Plan Sheets - Each original plotted plan sheet shall have the title block completely filled out. Original plotted plan sheets are defined as the final sheets plotted by CADD that will be used as the original sheets and record copies made, signed and sealed. Initials may be used for each block, except for the "Approved by", which must be the full name (printed by CADD) of the responsible professional engineer. The "Approved by" will be either the responsible in-house professional engineer or the consultant responsible professional engineer, whichever applies.

The key sheet for each component set (i.e. roadway, signing, etc.) shall have the responsible professional engineers name (printed by CADD) in the following format:

 Plans Approved by:	Roadway
 Date:	
P.E. No:	

For the other components, "Roadway" should be substituted with the title of the component set, such as "Signing and Pavement Marking. Official Record Set (CADD) - The official record set shall be signed and sealed as required in Section 19.2.1 for manually prepared plans.

19.2.3 Signing and Sealing Revisions

Signing and Sealing Revisions by the original

Responsible Professional Engineer: - When revisions are required after the plans have been signed and sealed, and the original responsible professional engineer is to prepare the revisions, the revisions to the original sheets shall be prepared as outlined in Chapter 20 and the revision blocks filled out. This can be either manual or plotted as determined by the method used to prepare the plans.

Signing and Sealing Revisions by other than the

Original Responsible Professional Engineer: - When revisions are required, every effort should be made to have the original responsible professional engineer make, sign and seal the revision.

It is recognized that situations will arise where the original responsible engineer is not available to the Department. When this is the case, and another qualified professional engineer services is used to revise the plans, the procedure is to be the same as noted for the original professional engineer. The only addition to the normal distribution is to send one additional signed and sealed record set to the original responsible professional engineer, or to the consulting firm with whom he was employed at the time the service was provided, for inclusion in their record set.

19.3 Signing and Sealing Other Engineering Documents

Engineering documents, reports or recommendations that influence or limit the design engineer's decisions in the development of design plans, shall be signed and sealed by the responsible professional engineer. The signing and sealing professional engineer will note the number of pages in each document. Other engineering documents must be signed and sealed as required by Florida Statutes and/or the Board of Professional Engineers. It shall be the District's responsibility to ensure that plans and engineering documents are properly signed and sealed prior to transmittal to the Central Office. The following documents shall be signed and sealed by the responsible professional engineer:

Document	Placement of Seal
1. Specifications & Special Provisions	First sheet of official copy
2. Pavement Design Package	First sheet of official copy
3. Typical Section Package	First sheet of official copy
4. Drainage Computations	First sheet of official copy
5. Bridge Design Data Sheet	Official copy
6. Hydraulics Reports	First sheet of official copy
7. Traffic Engineering Reports and Recommendations	First sheet of official copy
8. Engineering Reports and Documents	First sheet of official copy
9. Environmental Engineering Documents providing engineer conclusions and/or recommenda	-
10. Soil Survey Reports	First sheet of official copy
11. Geotechnical Analysis	First sheet of official copy
12. Value Engineering Analysis	First sheet of official copy

13. Roadway and Traffic Design First sheet of official copy

Standards

19.4 Supplemental Specifications and Special Provisions

When more than one engineering discipline (Civil, Electrical, etc.), is involved, more than one signature may be required on the specifications. This can be accommodated on an index sheet indicating the pages of specifications which were the responsibility of each engineer in his discipline. This does not mean, for example, that a civil engineer who has expertise in traffic signals or highway lighting, cannot seal the signal plans or specifications, as it is entirely possible that a civil engineer could have that expertise. If, however, a professional electrical engineer was used, he must sign and seal the portions for which he was responsible.

19.5 <u>Information Requiring Certification</u>

Engineering decisions are often made on the basis of support data furnished by non-engineering staff or offices. These data are to be certified as being obtained in accordance with official Department procedures. The following data are to have the noted certification attached when submitted for use in engineering related work.

19.5.1 18 KIP Equivalent Single Axle Loads

"I have reviewed the Traffic Forecasting Procedure, adopted by the Florida Department of Transportation, and have arrived at the projected 18 Kip loading volumes. I have found these to be consistent with the historical traffic data and other available information."

Name
Signature
Title
Organization Unit
Date

19.5.2 Project Traffic (Traffic to be used for Design)

"I have reviewed the Traffic Forecasting Procedure, adopted by the Florida Department of Transportation, and arrived at the project traffic volumes. I have found these to be consistent with the historical traffic data and other available information."

Name			
Signature			
Title			
Organization Unit			
Date			

Chapter 20

PLAN REVISIONS

20.1 General

Plan revisons are occasionally necessary to change original plan sheets after they have been signed and sealed by the responsible professional engineer. Any such change after signing and sealing is a revision. There are three possible 'Revision Stages' for any project:

- 1. <u>Signed and Sealed (SS):</u> Record prints are signed and sealed; original plans are in the District or Consultant's office.
- 2. <u>Signed, Sealed and Shipped (SSS):</u> Record prints are signed and sealed. Originals have been shipped or electronically transferred to the Central Office in Tallahassee.
- 3. <u>Let to Contract (LTC):</u> Project let to contract. Project may be under construction.

20.2 Revision Process for Each Stage

20.2.1 Signed and Sealed (SS)

When plans are in this stage, and revisions are required, the following steps will be required:

- 1. If plans are federal-aid, FHWA approal must be obtained prior to making revisions.
- 2. The District Design Engineer or Project Manager/Coordinator must concur in revision.
- 3. The responsible professional engineer making the revisions shall complete the revision block on all revised sheets, sign and seal revision as noted in Chapter 19. Where no revision block exists (key sheets, pay items sheets ... etc.), the revision date, brief description of revision and the initials of reviser shall be noted on the sheet. (Lower right for key sheets and along the top of computer generated pay item sheets)
- 4. Prior to making any revisions, copies of all sheets to be revised shall be made and retained with the record set and inscribed with "Void, Preserve for Plans Record".
- 5. No revision letter is required for plans revised at this stage.

20.2.2 Signed, Sealed and Shipped (SSS)

When plans are in this stage and revisons are required, the following steps will be required:

- Plans must be requested to be returned to the District for both in-house and consultant designs. The District will be responsible for sending plans to Consultant for revisions.
- 2. If plans are federal-aid, FHWA approval must be obtained prior to making revisions.

- 3. The District Design Engineer or Project Manager/Coordinator must concur in revision.
- 4. Prior to making any revisions, copies of all sheets to be revised shall be made and retained with the record set and inscribed with "Void, Preserve for Plans Record."
- 5. The responsible professional engineer making the revision shall complete the revision block on all revised sheets, sign and seal revision as noted in Chapter 19. Where no revision block exists (key sheets, pay item sheets ... etc.), the revision date, brief description of revision and the initials of revisor shall be noted on the sheet. (Lower right for key sheet and along the top of computer generated pay item sheet.)
- 6. A revision letter is required (see Exhibit I-20-A).
- 7. If time remaining until letting date is 21 days or less by the time the revision will reach the Central Office, processing of the revision must be approved by the State Roadway Design Engineer.
- 8. If time remaining until letting date is fourteen days or less by the time the revision will reach the Central Office, processing of the revision must be approved by the State Roadway Design Engineer. Normally, only those revisions that can be processed and received by the contractors five working days before the letting, will be processed.
- 9. Only revisions that can be handled by a telegram will be processed within ten days of letting and must be approved by the State Roadway Design Engineer.
- 10. All quantities, pay items, computation books, etc., shall be updated as part of the revision.
- 11. The roadway key sheet shall have all the revisions noted in the lower left hand corner of the sheet. See Exhibit I-20-B.
- 12. When the project contains other components such as signing and pavement markings or signalization plans, the revisions

DATE:	1 of
TO: James W. Weeks, Eng	gineer of Specifications
FROM: , Dis	strict Design Engineer
COPIES TO: K.N. Morefield, J. W.C. Deloach, R.L. F	
SUBJECT: W.P.I. No.(s)	
State Project No.(s)	
F.A. Project No.(s)	
County	S.R. No
SECTION ROADWAY SIGN	ING SIGNALS LIGHTING
COST CENTER	
NO. SHEETS	
PREPARED BY:	REQUESTED BY:
PROCESSED BY:Central Office	DATE:
AUTHORIZED BY:FHWA	DATE:
APPROVAL IF WITHIN 21 DAYS	OF LETTING:DATE:
	lowing sheet(s) of the subject plans
has (have) been revised:	
These are the only sh	eets that need reprinting.
A complete reprinting of	the entire package will be necessary.

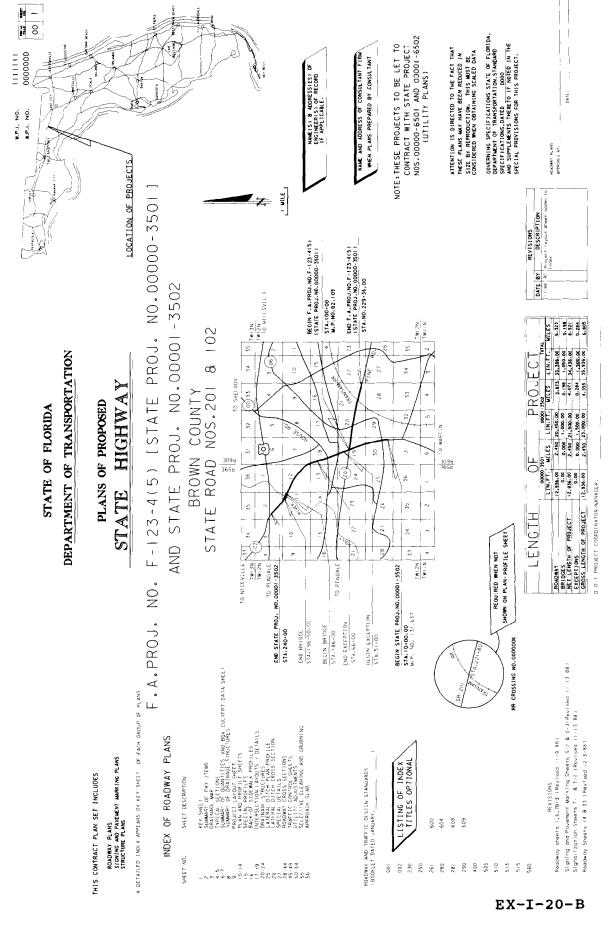
EX-I-20-A 1 of 2

DATE:	OF
PROJECT NO	
Sheets No.(s)	<u>Description of Revision</u>

EX. I-20-A

2 of 2

THIS EXHIBIT IS FOR EXAMPLE ONLY AND DOES NOT REFLECT THE DEPARTMENT'S DESIGN CRITERIA.



- shall be made on the plan sheets, and all the revisions, along with the other component revisions, noted on the key sheet only.
- 13. When preparing the plans package for resubmittal, all revised sheets shall be moved to the front of the plans package immediately behind the roadway key sheet, and a copy of the revision letter included with the plans. The "Void, Preserved for Record Copies" shall be placed in the proper order in the plans.

20.2.3 <u>Let to Contract (LITC)</u>

Once plans are let to contract, any changes (Supplemental Agreements or Change Orders) must be reflected on the plans.

The plans revision process for this stage is as follows:

- 1. If plans are federal-aid, FHWA approval must be obtained prior to making revisions.
- 2. The District Design Engineer or Project Manager/Coordinator should be contacted to concur in revision.
- 3. Original plans must be requested from the State Roadway Design Engineer's Office in Tallahassee.
- 4. Prior to making any revisions, reproducible copies of all sheets to be revised shall be made and inscribed with "Void, preserve for Plans Record" and returned with the original sheets to the Central Office.
- 5. The responsible professional engineer making the revision shall complete the revision block on all revised sheets and include in the description that the revision is for a Supplemental Agreement, Change Order, etc. The revised sheets shall be signed and sealed as noted in Chapter 19. Where no revision block exists (key sheets, pay item sheets ... etc.), the revision date, brief description of revision,

- and the initials of reviser shall be noted on the sheet. (Lower right for key sheet and along the top of computer generated pay item sheets.)
- 6. All pay item sheets, quantity sheets and tabulation of quantity sheets, <u>shall not</u> have the original quantity erased, but marked through the original quantity and the revised quantity written above, below or beside the old. The computation block must be updated.
- 7. The roadway key sheet shall have all the revisions noted in the lower left hand corner of the sheet, (See Exhibit I-20-B) and note the revision as being for Supplemental Agreement, Change Order, etc.
- 8. When the project contains other components such as signing and pavement markings or signalization plans, the revisions shall be made on the plan sheets and all the revisions, along with the other component revisions, noted on the roadway key sheet only.
- 9. When preparing the plans package for submittal to the Central Office, all revised sheets shall be moved to the front of the plans package immediately behind the roadway key sheet, a copy of the letter to the District Construction Engineer included, and the "Void, Preserve for Plans Record" copies placed in the correct order in the plans.
- 10. A letter addressed to the District Construction Engineer must accompany the revised sheets to the District Construction Office. The letter will address the reason for the revision, who requested the revision, who approved the revision, and if FHWA has concurred. All existing pay items that are affected must be listed with original and revised quantities. Deleted or added pay items must also be shown. Copies of the letter will go to the District Secretary, FHWA, State Construction Office and State Roadway Design Office (see Exhibit I-20-C for example letter). The District Construction Office shall return originals and "Yoid, Preserve for Record" sheets to the Central Office.

20.3 Automated Plans (CADD)

The revision process will be the same for manual or CADD produced plans, except that if the plans are automated, the revisions must also be automated to be compatible with the plans retention system.

DATE: July 22, 1988

TO: John Doe FROM: Jack Smith

COPIES TO: Jimmy Brown, Charlie Black, Frank White

SUBJECT: State Project 00000-0000

Meridian Road Revisions

Due to a commitment by the Department, the retention area #1 right of Station 75+87 has been revised to provide additional storage. These changes are indicated on sheets:

5 Summaries of Earthwork and Sodding revised

32 Excavation in SW corner indicated

83 Excavation indicated

The City's project on John Knox Road has not been let. Therefore, the following revisions are required, with changes indicated on sheets:

13 Existing revised, exception removed

43 Existing revised

S-5 Existing revised, exception removed

We have also corrected an error on Sheet 5 on the Summary of Underdrain. The plans have been changed to show 300 LF between Station 60+00 + 72+00; the total has been revised to 6039 LF.

EX-I-20-C 1 of 4

The above revisions have changed the Quantities for the following pay items:

ITEM NO.	DESC.	ORIGINAL QUANTITY	NEW <u>QUANTITY</u>	INCREASE	<u>DECREASE</u>
20-1	Reg. Exc.	1412 CY.	3,399	1,987	
300-1-2	Bit. Mat'l	6334 gals.	6,545	211	
	(Tack)				
300-1-6	Bit Mat'l	6268 gals.	6,522	254	
	(St)				
312-70	Cover Mat'l	20893 SY.	21,738	845	
440-1-10	Underdrain	8239 LF	6,039		2,200
	Type 1				
520-1-10	Curb &	14,324 LF	14,515	191	
	Gutter				
570-5	Fertilizer	1.2 Ton	1.4	0.2	
570 - 9	Water	13.535	15.885	2.350	

EX.-I-20-C 2 of 4 John Doe July 27, 1988 Page 2

ITEM NO.	DESC.	ORIGINAL QUANTITY	NEW QUANTITY	<u>INCREASE</u>	DECREASE
575-1	Sodding	13,103 SY	15,377	2,274	
5331-2	Type S	2,799.1	2,830.8	31.7	
5332-2-7	Type II	2256.7 TN	2,332.7	76.54	
5337-1-5	Friction	34,386.0 SF	35,231	845	
	Course				
706-1-12	Ref. Pav't	966 EA	997	31	
	Marker				
711-4	Directional	58 EA	59	1	
	Arrows				
711-6-181	Traffic	961 LF	1029	68	
	Stripe (18")				
711-6-241	Traffic	562 LF	639	77	
	Stripe (24")				
711-8-41	Traffic	5.713 NM	5.864	0.151	
	Stripe (4")				

In addition to these changes, the project number has been changed on sheet 25, 27, and S-2; on Sheet 28, signatures have been added.

EX.I-20-C 3 of 4 It has been requested by Property Analysis Corporation that the outfall right of Station 75+87 be shifted. Mr. Miller advised me the City is in agreement with this shift of right-of-way for the outfall. Since the Department has a joint project agreement with the City to construct the outfall of Station 75+87, Stuart Christmas advises me that the agreement should be modified. Mr. Christmas is reviewing the agreement to determine if the City can be held responsible for all additional cost caused by the change in the outfall location. These plan revisions will be transmitted at a later date.

By copy of letter, I am requesting that Frank White coordinate any revisions for permits.

If you have any questions, please call me at XXX-XXXX.

EX.-I-20-C 4 of 4

Chapter 21

CONSULITANT PROJECT MANAGEMENT

21.1 General

The Florida DOT may elect to use a consultant to provide all or a portion of the engineering services required for a transportation project. Guidelines for use in acquiring and managing such a consultant are contained in the Florida DOT Project Management Guidelines, presently under revision.

21.2 Project Definition

Once the project is included in the Florida DOT's Adopted Five Year Work Program and a decision has been made to acquire professional services, the work to be performed by the consultant must be carefully defined and stated in Project Concept Reports and Scopes of Work.

21.2.1 Project Concept Report

The Project Concept Report serves the function of identifying project goals and constraints. This document would precede and be supplemental to the normal scope of services and would identify and define existing conditions, issues, concepts and criteria. It then could provide the basis for the detailed scope of consultant services.

21.2.2 Scope of Services

The Scope of Services document is the cornerstone of the consultant procurement process. This document specifically outlines the features of the project in sufficient detail to form the basis of the technical and price proposals to be submitted by the consultants. It also forms the basis for the Florida DOT's evaluation of these proposals and for the preparation of the Florida DOT's man-hour estimates for each work component. Subsequently, it becomes the primary component of all contract agreements. This document should itemize all aspects for which the consultant is responsible and all aspects for which the department is responsible.

21.3 Consultant Acquisition

Consultant acquisition is accomplished according to established rules and procedures and includes; requests for proposals, proposal evaluation, negotiation and contract execution. Each of these are further explained in the Project Management Guidelines and in Contractual Services Office's procedures.

21.4 Technical Management

Technical Management consists of the application of proper planning, monitoring and control techniques applied throughout the life of the project. This process is used to guide the project through the many technical disciplines and their interfaces in order to ensure timely interaction and successful completion. Proper documentation of all deliverables as required by the scope of services as well as every decision reached as a result of inquiry or coordination meetings is required. For further explanation, consult the Project Management Guidelines.

21.5 Contract Administration

Contract administration is required to ensure that the terms of the consultant contract are met and to verify that any modifications to the contract, such as time extensions and supplemental agreements, are necessary and conform to the original agreement. Periodically, throughout the life of the contract, consultant progress will be measured and documented for the purpose of providing progress payments. Ultimately, all of the above is used as a basis to grade the consultant upon completion of the contract. Each of these areas is covered in the Project Management Guidelines and by existing Contractual Services Office's procedures.

21.6 Scheduling and Coordination

A major objective of project management is to determine the relationship of different tasks to each other, and to schedule and control these tasks so that the project is carried out logically and efficiently. In accomplishing this, the project manager should include status meetings as a routine part of the project schedule. The meetings should be held on a periodic basis, usually monthly. The purpose of these meetings will be to discuss the progress of the project and to make decisions for the remainder of the services. Functional area managers as well as other agencies should attend as required to share information and to develop an understanding of how their activities blend into the overall project objectives. Further discussion of these areas can be found in the Project Management Guidelines.

21.7 Consultant Use of Department Computer Programs

The Department has various computer programs which are of great value to the designer. Consultants designing projects for the Department may request the use of these programs through the DOT Project Manager/Coordinator. Consultant should contact the Department for available programs.

Various CADD files are also available for use by the consultant on projects for the Department. Information and authorization for use of these files may be obtained from the District CADD Coordinator.

Chapter 22

ARCHITECTURAL PLANS

22.1 General

Designs for building construction projects are normally based on square footage requirements as established by the Florida DOT's owner/user unit and the District Fixed Capital Outlay coordinator. Square footage requirements shall be in compliance with the Department of General Services space criteria (Form BPM 4056 R2-86) and based on anticipated staffing and use. The size of rest area buildings is determined by the number of toilet fixtures required. The number of toilet fixtures and parking places required at the rest area are computed based on the 20 year design traffic of the highway. These computations are done on a Facilities Computation from (Ex-I-22-G) and become a part of the project file.

The construction of all new Florida DOT buildings and renovations or additions to existing buildings require plans and specifications signed and sealed by a Licensed Florida Architect (F.S. 481).

Consultant contracts for architectural design of buildings in the Districts shall be administered and managed in the District office. A very detailed project definition and scope of services is essential to a successful architectural design contract.

Architectural plans often include other engineering plans. Typically, this requires the acquisition of engineering subconsultant(s) to perform those services for which the architect is not qualified under F.S. 481.

The architectural services required for the design of a building are generally done in four phases as listed below:

- 1. Schematic Design Phase
- 2. Design Development Phase
- 3. Contract Documents Phase
 - a. Construction Plans
 - b. Specifications & Special Provisions
 - c. Construction Cost Estimate
- 4. Construction Phase
 - a. Shop Drawing Review
 - b. Scheduled Periodic Construction Site Visits
 - c. Review of Contractor Pay Requests
 - d. Substantial and Final Inspections

Requirements and criteria for the four phases are addressed in this chapter.

22.2 Codes, Rules and Regulations

A. Codes, Rules and Regulations

 Certification: The Design Professional shall provide with each of his formal design review submissions a listing of all codes and regulations which are applicable to the project. He shall further certify over his signature, that his design documents fully comply with those codes and regulations.

B. Applicable Codes, Rules and Regulations

1. Building Codes: The design professional should use the following codes as applicable. Refer to Chapter 13D-17, Florida Administrative Code and Sections 553.43 and 553.73, Florida Statutes.

The Standard Building Code applies, current edition. Dade County does not use the Standard Building Code.

- 2. Fire and Life Safety: All requirements of the National Fire Protective Association, including the Life Safety Code (NFPA 101), current edition, shall apply.
- 3. Electrical and Plumbing Codes: Florida Electrical Code Part II, Chapter 533.19, Florida Statutes (National Electrical Code, current edition). NFPA 90A, 1978, air conditioning and ventilation.
- 4. Handicap Code: All requirements of the "Uniform Federal Accessibility Standards", and ANSI Al17.1, "Standards for Making Buildings and Facilities Accessible to, and Usable by, Physically Handicapped People", current editions, shall apply.

- 5. State Requirements: The following agencies have been charged by law with the responsibility of protecting the public health, safety and welfare in the following areas:
 - a. Fire Protection Rules and Regulations, State Fire Marshal's Office.
 - b. Rules of Florida Industrial Commission, Department of Commerce, Division of Labor, Bureau of Workman's Compensation enforce OSHA Building Standards CFR-291926.
 - c. Florida Hotel and Restaurant Division, Statutes and Rules, Department of Business Regulation.
 - d. Rules and Regulation of the Division of Health, Department of Health and Rehabilitative Services.
 - e. Rules for the Physically Disabled, Chapter 13D-1, Department of General Services.
 - f. Florida Electrical Code, Part II, Chapter 533.19, Florida Statutes, Department of General Services.
 - g. FLEET, Florida Lifecycle Energy Evaluation Technique, Chapter 255.251, Florida Statutes and Chapter 13D-10, Florida Administrative Code, Department of General Services.
 - h. Energy Efficiency Code for Building Construction, Chapter 553, Florida Statutes.

In every case where the regulations and rules of the above agencies apply, they shall be complied with in the development of the project.

C. Approval

It is the responsibility of the Design Professional to determine which other authorities have jurisdiction and assure that the development of the project is fully coordinated with these agencies and that their requirements are accommodated in the documents so that all approvals can be readily obtained. This coordination starts with the schematic design and must be completed prior to taking of bids, so that delays in start of construction do not occur.

1. Sewage

Connections or facilities for discharge of 2,000 gallons or less in 24 hours requires submission of DER-14-4 to the Florida Department of Environmental Regulation or other designated permitting agency. See DER 17-3, Water Quality Standards and DER 17-16, Waste and Domestic Wastewater Plants.

2. Zoning and Comprehensive Planning

All local governments, as required by Chapters 163.315(5), 163.3165(8), 163.3194(1) and 163.3211 Florida Statutes, have or will adopt comprehensive plans and zoning. The State, as is any Owner, is obligated to develop their property in accordance with the adopted plans or codes and obtain reviews and approvals including variances as required. The Architect should begin this coordination early in the design process and carry it through to the end to ensure that the project meets all local requirements and to resolve conflicts as they arise.

3. Use and Development Permits

Certain use and development permits are required by local, State and/or federal agencies prior to commencement of construction. These permits, covering such items as sewage, water, air quality, etc., must be obtained prior to release of documents for bidding. It is the responsibility of the Architect/Engineer to identify each such permit and to apply for these on behalf of the Owner.

4. Fire Department

The Design Professional is expected to work closely with the Fire Department serving the project area, and to accommodate their fire fighting methods, equipment, hose threads, etc.

5. Flood Prone Areas

Every site plan shall be evaluated for flood hazard and meet the minimum building requirements mandated by the National Flood Insurance program. If the local permitting office can not provide the necessary information, a flood hazard evaluation may be requested from: Mr. Howard Glassman, State Assistance Office, Federal Flood Insurance Program, 256 Howard Building, 2571 Executive Center Circle East, Tallahassee, Florida 32301.

6. FLEET

Every building over 5,000 square feet in area and leased space of over 20,000 square feet is subject to the provisions of Chapter 255.251, Florida Statutes and Chapter 13D-10 Florida Administrative Code. This program is administered by the Technical Evaluation Section, Bureau of Construction, Department of General Services.

22.3 Standard Document Requirements

A. Drawing Size

All drawings shall be 23" \times 34" (D-size). For border size and other sheet format information see paragraph 1.4 (II-1-3.0) of this manual.

B. Specification Size

All specifications and special provisions shall be printed in an 8 1/2" by 11" format, bound on the left hand side in a permanent type binder.

c. Lettering

All lettering shall be vertical uppercase and must be plain, free of adornment, and legible; minimum size shall be 1/8" when reduction is proposed. All symbols shall be clearly separated from one another and from adjacent lettering or lines.

D. Scales

Scales should be located directly under the title of each plan, elevation, section, detail, etc. The scale should be given as an example of the object drawings, i.e., Scale: 1/8" = 1'-0".

E. Title Blocks

The title block on each drawing shall appear in the lower right hand corner along the right margin. It shall contain information as delineated in Ex-I-22-A & I-22-B.

F. Title Sheet, Pages and Bindings

Title sheets, title pages, and binding of documents shall comply with the provisions of Ex-I-22-C (text) & I-22-D (layout of drawing, key sheet).

G. Signature and Seal

All final reproducible contract documents submitted by the Design Professional shall bear the appropriate professional signature. The Architect shall submit three originally signed and sealed sets of prints of the plans and specifications to serve as record sets of contract documents.

H. Drawing Arrangement

The drawing shall be arranged by subject category.

- 1. Key Sheet
- 2. Civil
- 3. Architectural (including site plan)
- 4. Structural
- 5. Mechanical
- 6. Plumbing
- 7. Electrical

I. Specification Organization

Specifications shall be organized in accordance with the format of the Uniform Construction Index as adopted and established by the Construction Specifications Institute (CSI) or The American Institute of Architects (AIA).

22.4 Schematic Design Phase Requirements

- A. A Schematic Design Phase shall be required as a part of all architectural design services.
- B. The Schematic Design Phase shall include, as appropriate, but not be limited to:
 - A narrative description of the design and construction concepts and how they are responsive to the project requirements.
 - 2. A plan showing how the project fits into the master plan for total facility development.
 - Site plans showing existing and proposed roads, circulation elements, utility systems, plantings and special site features.
 - 4. Studies and reports relative to the site and its topographical, ecological, botanical and other features contributing to the solution or requiring significant alteration of the existing site.
 - 5. A description of existing zoning and other site conditioning factors restricting development and their solutions or recommendations for resolving them.
 - 6. Floor plans, elevations, building sections, and sketches as necessary to adequately present the concept.

- 7. If the project is an addition, or is otherwise related to existing buildings on the site, the plans shall show such facilities and their general arrangements and relationships.
- 8. The general description analysis, and sketches, where applicable, of the design and construction concepts shall be provided for architectural, structural, plumbing, fire protection, mechanical (HVAC), communications, electronics and electrical systems.
- 9. Mechanical and electrical descriptions, including computerized building management systems, shall indicate proposed systems and equipment in suitable detail, and be accompanied by a complete schematic format to enable an energy design submission of FIEET in compliance with Florida Energy Conservation in Building Act. See Volume III of the Energy Conservation Guide.
- 10. Specific layouts of complex areas such as mechanical rooms, computer rooms, operation areas and conference rooms.
- 11. Description of the features and provisions provided in the facility for use by the handicapped.
- 12. Details as required.
- 13. A statement of the provisions that will be taken to satisfy acoustic requirements.

- 14. Outline specifications, using CSI/AIA format.
- 15. An identification and presentation of code requirements. Provide a review of codes having a bearing on the project and report specific problems encountered in conforming to these codes as well as solutions, waivers or variances, if any, being pursued or required.

22.5 <u>Design Development Phase</u>

22.5.1 General

Upon approval of the Schematic Design Phase and receipt of the Notice to Proceed into the Design Development Phase, the Design Professional should proceed with the amplification and refinement of his advanced schematic design study to comprehensive Design Development documents.

22.5.2 Development Procedures

The Architect should, with the concurrence of the Florida DOT, schedule necessary conferences and meetings with the Florida DOT and other affected parties, to ensure that the project is developed in a coordinated and optimum manner.

22.5.3 Reviews

A. General

The Design Professional shall submit the Design Development documents to the Florida DOT and other agencies (those agencies having jurisdiction or approval requirements) as designated by the Florida DOT's Project Manager and with his concurrence, schedule a Design Development review conference. At that conference, the Design Development shall be presented by the Architect/Engineer and be explained in depth and detail as appropriate. The Architect shall provide minutes of this review conference to all attending parties.

The Design Professional shall confer with the Florida DOT and other persons attending this conference, responding to all questions and comments. If the comments are critical and require response prior to proceeding with the next phase of the project, it shall be the responsibility of the Design Professional to submit such response as soon as possible.

B. Conceptual Approval

If the development is found to be acceptable by those present, the Florida DOT may issue conceptual approval of the Design Development Phase in order that the Architect may proceed with the development of the next phase.

22.5.4 Submission Requirements

A. General

The Architect shall present sufficient documentation in the form of drawings, specifications, design criteria, and cost This shall include working sketches and models as factors. necessary to fully explain and establish the scope of the project and all of its particulars so as to be readily understood by lay and technical reviewer. The design criteria shall supplement the Design Development drawings, specifications and cost estimates. It shall be a presentation of facts sufficiently complete to demonstrate that the concept of the project is fully understood and that the subsequent design details in their presentation in the final plans and specifications will be on a sound professional basis.

The criteria, the basis for design, may be in any form or any combination of forms. It should be for all intents and purposes a copy of the working data upon which the design is based. Sketches, calculations, notes, economic analysis and any other pertinent data shall be presented as the design criteria. The information to be presented consisting of drawings, specifications, cost estimates and design criteria, should be complementary and should include, but not be limited to, the categories described.

B. Site

The information pertaining to the site and its development should be presented as an entity passing across professional disciplines. It should include, but not be limited to:

1. Information Data

Location plots, property and topographical surveys, subsurface boring, lots and plans, ecological and botanical surveys, and other appropriate information.

2. Master Plan

A copy of the Master Plan indicating the location of the project and total scheme, if applicable.

3. Environmental Considerations

Necessary design data, specifications, and cost estimates for preservation, dust, erosion,

sedimentation and runoff control, where applicable, as an integral part of the design and construction project. Such controls will be limited to the areas involved in the construction operation and those required by applicable ordinances, rules, laws, etc. Environmental control will not be confused with landscaping. The information provided will include statements regarding the type of treatments selected, the affected areas, and the reasons for the selection of the type of controls chosen.

4. Grading and Site Development.

a. Site Construction

All permanent features to be constructed on the site.

b. Roads, Walks, Paths and Parking

The data provided, in addition to the proposed development, should include a statement of the general soil conditions with a brief outline of the soil exploration and testing performed as related to the development of roads, etc. The type and volume of traffic, controlling wheel loads, classes of surfacing under consideration with justification for same, and any deviation from criteria fitness for those classes should also be included.

5. Utility Services

All existing and proposed utility services including runs, locations capacities, sources, characteristics, materials and installation methods should be fully described.

a. Electrical

- (1) A statement relative to the adequacy of the primary supply at the point of takeoff. If the primary source is inadequate, state measures proposed to correct the deficiency.
- (2) The electrical characteristics of the power supply to the site or area, including circuit interrupting requirements, and voltage regulations.
- (3) An estimate of the total connected load and resulting kilowatt demand load by applying proper demand and diversity factors if a group of loads is involved.
- (4) The basis for selecting a primary and/or secondary distribution voltage.
- (5) The type, size and location of conductors.

- (6) Statements describing the pertinent standards of design, such as voltage drop, physical characteristics of overhead or underground circuit, types of lighting units and lighting intensities.
- (7) The type and adequacy of telephone, signal, fire alarm and other communication systems, including a statement as to the number of spare telephone conductors available and spare capacity on fire alarm circuits.
- (8) Emergency power systems.
- b. Fuel Distribution and Storage Information provided shall include for the following types:
 - (1) Gas
 - (a) Statement of type, location of take-off from supply, and available pressure.
 - (b) Statement of type and material for pipes and valves.
 - (2) Liquid Petroleum Products
 - (a) Statement of unloading facilities, such as dock, tank car, or truck.

- (b) Description of the type of system and proposed features.
- (c) Statement of the basis for storage capacity, rate of pumping and number of dispensing outlets.
- (d) Description of power supply and power requirements.
- (e) Selection of type of materials for pipes, tanks and valves.

c. Steam Distribution

Data provided should include points of connection, pressure, size, material and method of installation of proposed piping.

d. Central Chilled Water and Refrigeration

The information provided should include sizes, material and method of installation of proposed piping.

- e. Domestic Water and Fire Protection
 - (1) Source, minimum and maximum pressure at each building and in the system, and an explanation of the existing system covering particularly the type, capacity, condition, present water use, and unsatisfactory elements of the

component parts.

- (2) A statement of the type of construction proposed, materials for water mains, or wells, etc.
- (3) The distribution system, a statement of design, domestic and fire flow usage of well pressure, elevation differential, and the designer's basic estimate of tentative pipe sizes.
- (4) A statement of tentative sizes, elevations, capacities, etc. as can be readily determined without long computations or design considerations for reservoirs, treatment units, pumping plans, well pumps and such units.

f. Sewers and Sewage Disposal Systems

Information provided shall include:

- (1) An explanation of existing systems covering particularly the type, capacity, conditions, present flow and unsatisfactory elements or components.
- (2) The interpretation of the degree of treatment necessary by field requirement and units necessary for treatment.

- (3) A statement of the design factors with present design population per various units for the sewage treatment plant.
- (4) Statements of materials to be used for the sewage system, sewage collection system and the sewage treatment plants.
- (5) Means of effluent disposal.

g. Storm Drainage

- (1) An explanation of the existing system covering particularly the type, capacity, condition, and unsatisfactory elements or components.
- (2) A statement of the type of construction proposed, material, etc.
- (3) A statement of the design requirements and tentative pipe sizes.

h. Electronics and Instrumentation

As related to the site, the following information shall be provided:

- (1) System engineering concepts.
- (2) Site and location considerations.

- (3) Antenna requirements such as types, separation, height, aircraft clearance, and area requirements.
- (4) Site communications and control linkages.
- (5) Electronic security considerations.
- i. Cathodic Protection (in addition to the proposed development provided):
 - (1) Results of soil resistivity measurements.
 - (2) Variations in soil makeup.
 - (3) Soil moisture content and normal seasonal variations.
 - (4) Results of temporary cathodic protection tests, if any.
 - (5) Results of structure to soil potential measurements where protection is to be provided.
- j. Iawn Sprinkling Systems: Provided tentative layouts, material, sizes, etc.

6. Fencing

Type, height and justification for fencing.

7. Landscaping

Plant species, size and layout.

- 8. Lateral and transverse sections through the site shall indicate development of the site, when necessary, due to substantial elevation changes or circulation at more than one level.
- 9. Any special consideration pertinent to the site and its development.

C. Buildings

Each building should be fully described. The information should include, but not be limited to:

1. Architectural

- a. The plan of each floor showing furniture, equipment and any other use-determining factors.
- b. Area recap and square footage should be indicated for each floor area and for the projects in total in comparison to that required by the program.

- c. Exterior elevations, delineated and shadowed, giving floor elevations at each level and showing finish materials.
- d. Transverse and lateral sections through the building, indicating heights, vertical circulation, and relationship. The finished floor elevations of each level should be given.
- e. Wall sections and wall details necessary to indicate the methods of construction and to determine the overall values achieved as required by the Florida Energy Code.
- f. Schedules indicating finishes and equipment in specific areas.
- g. Provide color schemes and schedules for all areas interior and exterior. Material and sample pallets shall be provided.
- h. The class of construction, occupancy, openings, and fire ratings for hall doors and other such openings shall be provided or indicated.
- Provide large scale plans describing complex, intensely equipped or furnished areas, and areas needing clarification.

- j. Provide a list of all safety equipment, including costs that have been included in the project.
- k. Sketches as necessary to portray the design concept. The formal architectural renderings, if required by the Contract for Professional Services, should be submitted after approval of the Design Development documents.
- A description of the materials used for all major items of construction.
- m. Other information considered necessary for the development of the program or explanation of the design.

2. Structural

- a. A description of foundation conditions, types of foundations to be used, the method by which the allowable bearing value is to be determined, and the maximum allowable bearing capacity for the foundation.
- b. Statement as to the type of construction adopted and reasons therefore, with capacities, dimension, or other size criteria.

- c. Preliminary floor plans showing framing members and column sizes indicating special design features and noting floor elevations. The description of structural floor system proposed with length and spacing of principal members, dimensions etc.
- d. The description of structural roof system proposed with principal members, dimensions etc.
- e. Provide structural building sections, transverse and longitudinal, indicating vertical relationships and headroom.
- f. Note limited load carrying capacities and statements of live loading to be used including floor loads, winds, earthquake etc., with justifying data.
- g. Provide calculations and design criteria when requested.
- h. A statement of any special considerations that affect the design.
- 3. Heating, Ventilation, Air Conditioning (HVAC)
 - a. Provide floor plan showing equipment layouts and single line layouts of duct work.
 - Provide sections showing equipment and locations of duct work.

- c. Energy Conservation: Provide calculations and criteria in the form and detail required to fully comply with the requirements of the Energy Conservation Manual and the requirements as set forth by the Energy Conservation Project Director.
- d. Provide full description of automated management systems proposed for use, including current and future capabilities.

Heating System

- (1) Statement of indoor and outdoor design temperatures and "U" factors for walls, ceilings, floors, etc., to be used in design.
- (2) Heating medium, such as steam, hot water, gas or electric.
- (3) Type of heating system such as convector baseboard, forced warm air, unit heaters, etc.
- (4) Types of building temperature control; such as electric, electronic or pneumatic, and indicate outdoor anticipator or indoor thermostat.
- (5) Location and type of heating plant.

(6) Brief explanation of the basis for selection of type of fuel, including an economic comparison with other fuels.

f. Ventilation

- (1) Statement of whether gravity or mechanical system.
- (2) Number of air changes per hour in various areas.
- (3) Ventilation air quantity during heating season and how acquired.

g. Air Conditioning

- (1) Provide a brief description of the air conditioning system proposed, such as factory assembled or built-up system; number of zones, if applicable; or unit type, chilled water system, or direct expansion, type of refrigerant, etc.
- (2) A statement of areas to be air conditioned.

- (3) Statement of inside temperatures and relative humidities, if applicable, outside wet and dry bulb design temperatures, "U" factors for the type of construction proposed and a statement of the economics of applying insulation and/or sun shades.
- (4) Description of equipment to be used such as reciprocating or centrifugal compressor, condensers, air handling equipment, duct system, piping, etc.
- (5) Type of building temperature control system such as electric, electronic or pneumatic and sequence of operation.

h. Evaporative Cooling

- (1) Statement of areas that are to be evaporatively cooled.
- (2) Description of equipment to be used.
- (3) Provide any other data as deemed necessary.

4. Electrical

a. Indicate electrical service entrance characteristics, transformer requirements, etc.

- b. Indicate electrical characteristics such as phase, voltage, number of wires, etc., of each circuit. Provide a breakdown of the estimated connection load to show:
 - (1) Lighting and convenience outlet load.
 - (2) Power load for building equipment such as heating, air conditioning, etc.
 - (3) Loads for special operating equipment such as compressors, welders, pumps, motors, etc., and for power receptacles being provided to energize special equipment. Apply an appropriate demand factor to each to compute a total demand load.
- c. Indicate the location of the main switchboard or power panels, light panels and all equipment panels.
- d. Indicate type of wiring system, such as rigid conduit, electrical metallic tubing, non-metallic sheathed cable, and where proposed for use.
- e. The type of conductors such as rubber insulated, varnished cambric, lead covered, etc., and their proposed location.
- f. Show the location of all lights, power outlets, switches, etc.

- g. Describe the proposed pertinent standards of design such as voltage drop, lighting intensities, and types of lighting fixtures.
- h. Describe the short-circuit duty required for all protective devices and switch gear.
- i. Indicate the requirements for emergency electrical systems.
- j. Ensure that the electrical information for the facility is fully coordinated with the site electrical requirements and with the low voltage communications systems requirements.
- k. Provide any other information necessary.
- 5. Communications, Electronic and Instrumentation Provisions
 - a. Provide sufficient information including engineering concepts for review purposes of the systems proposed; i.e., intercom system, telephone system, public address system, radio and antenna systems, television antenna systems, protective alarm system, response tie-ins and any other data or systems deemed necessary.
 - b. Indicate equipment selection, including special equipment requiring development, research, or breadboard methods to meet the requirements.
 - c. Site or location consideration.

- d. Required radio paths and propagations.
- e. Antenna requirements such as types, separation, tower heights, aircraft clearance and area requirements.
- f. Antenna transmission lines, terminations and switching.
- g. Bonding and grounding requirements.
- h. Communication, control cables and radio links.
- i. Test equipment, repair shop and spare parts storage requirements.
- j. Equipment and instrumentation arrangement and space requirements indicating requirement for racks, consoles, and for individual mounting.
- k. Wiring and cable requirement plus terminations.
- 1. Power and lighting requirements, including emergency or standby requirements.
- m. Air conditioning requirements including humidity and dust control requirements.
- n. Interference and clearance requirements

6. Plumbing

- a. Provide preliminary layout of utility lines and building construction services lines and elevations and sizes fundamental to design.
- b. Preliminary floor plans showing major horizontal and vertical services, the location and size of fixtures, equipment and the number of persons served.
- c. Preliminary building sections showing riser and branch lines, fixtures and equipment.
- d. Provide the estimated number of fixtures units in order, demand and GPM for all plumbing fixtures.
- e. Provide the estimated minimum and maximum water pressure at each building.
- f. Indicate the type of heater and capacity for hot water supply.
- g. Additional details as necessary to describe or clarify any other conditions.

7. Fire Protection

- Indicate service hydrants, stand pipes and test values.
- b. Indicate risers and hose cabinets.

- c. For sprinkler systems, indicate the authority for the installation, the hazard rate of occupancy, the type of sprinkler system (wet or dry), and the water volume and pressure required. Delineate any special system such as carbon dioxide, foam, etc., that will be required.
- d. Provide insurance rating groups governing the design.
- e. Provide any other data deemed necessary.

8. Special Equipment

Indicate all special equipment such as:

- a. Kitchen equipment
- b. Window coverings
- c. Lawn sprinkler equipment systems
- d. Any other systems deemed necessary for inclusion

D. Area Analysis

The Design Professional should submit an area analysis of the project. Use the criteria shown in EX-I-22-E and the summary form shown in Ex-I-22-F.

E. Other Data

Provide such data as deemed necessary to explain the design concept and Design Development documents.

22.6 Contract Document Phase

22.6.1 General

Subsequent to the receipt of approval of the Design Development documents, and upon authorization by the Florida DOT, the Architect, shall, in accordance with his Contract prepare complete contract documents of adequate and sufficient scope for construction purposes.

22.6.2 <u>Development Procedures</u>

The Architect shall hold conferences and meetings with the Florida DOT, applicable permitting agencies, and others as required to fully amplify the project and prepare his contract documents.

22.6.3 Review and Approvals

- A. Initial Reviews and Conferences: The Architect shall review in detail with the Florida DOT all codes and permits required for the project prior to proceeding with the development of his construction documents.
- B. 50% Contract Document Review Submittal: In accordance with the Contract, the Architect shall submit "in progress" review documents to the Florida DOT at 50% completion of Contract Documents.

C. Final Reviews and Approvals

1. Upon completion of his contract documents, the Architect shall submit complete document review

sets to the Florida DOT in a quantity sufficient to cover simultaneous review. He shall also submit review sets to any other agency having jurisdiction over the project and from which approvals will be required.

- 2. The complete contract documents will be reviewed by and commented on by all agencies having jurisdiction. Their comments should be transmitted by the Florida DOT to the Architect for response prior to scheduling, if necessary, a review conference at which the Architect should respond to all comments.
- 3. The Architect shall respond in writing to the comments received and shall correct or modify his documents as required. If necessary, he will resubmit the Construction Documents for additional review and comment.
- 4. Upon satisfactory response to all comments, and modification and correction of the contract documents, the Florida DOT will formally review and comment on the acceptability of the contract documents.

22.6.4 <u>Documents Required</u>

A. General

The completed contract documents shall be in accordance with the requirements of the Contract.

B. Area Analysis

The Architect shall submit an area analysis of the project, shown as Exhibit I-22-E.

C. Color Schedules

The Architect shall include in his specifications and on his drawings, where applicable, the actual colors or color ranges chosen to be used on the project. The Design Professional shall also submit with his contract documents a complete listing of colors to be used, providing pallets and material samples as necessary.

D. Drawing Requirements

1. General

The Architect shall ensure that his drawings are final and complete with all elements thoroughly checked and coordinated to ensure that there are no conflicts between architectural, structural, mechanical, electrical and other portions of the work. Particular emphasis shall be placed on this coordination when certain elements of the design are performed under

subcontract to the Design Professional. The drawings should be prepared so that change orders to construction will not be necessary due to errors, omissions, inadequacies or conflict between various component parts or with the specifications. When applicable, the design data should be shown on the drawings, i.e.:

a. Loads

- (1) Roof and Floor Live Loads
- (2) Wind Loads
- (3) Total Loads

b. Basic Working Stresses For

- (1) Concrete
- (2) Structural Steel
- (3) Wood
- (4) Concrete Block, Masonry

c. Foundations

- (1) Allowable Soil Pressure for Spread Footings
- (2) Bearing Value for Piles

2. Material

Unless otherwise instructed, the working documents shall be prepared in ink on plastic film (mylar).

E. Specification and Special Provision Requirements

1. General

The specifications shall be complete and final with all elements thoroughly checked and coordinated. Particular emphasis should be placed on the coordination of various elements of the specifications or portions of the specifications prepared under subcontract to the Design Professional.

2. Standard Specifications

Maximum use shall be made of standard materials and methods of construction and standard specifications.

Specifications for classifications of work and material issued by an approved association, such as ASTM, ASA, ASME, etc., may be included. Each referenced specification must be examined before its use to ensure that it is suitable for its intended purpose and that proper choice is made of the options given in it.

When a small quantity of materials is needed and a standard commercial product would be suitable, reference to a standard specification should not be made. Federal and military specifications may be used with the expressed approval of the Florida DOT.

When specifying by product, model number, etc., three acceptable manufacturer's products or equal will be specified.

In referencing standard specifications the following rules should be followed:

- a. Avoid reference to specific paragraphs in the standard specifications, since it limits the requirements to the paragraph referenced.
- b. Avoid repeated references to a standard specification within the same section.
- c. Specify types, classes, weights and similar applicable characteristics required to ensure an accurate description.

The Architect shall submit to the Florida DOT with his construction documents submittal a copy of each referenced specification.

22.6.5 Release for Printing

Upon receipt of formal review and comment from the Florida DOT, and upon authorization, the Architect shall print and properly sigh and seal three sets of documents for record purposes only.

22.6.6 Pre-Bid Conference

The Architect will attend a pre-bid conference administered by the Department and he will be prepared to answer questions from prospective bidders who have reviewed the contract documents.

22.7 Construction Phase Requirements

22.7.1 Notice to Proceed

The Notice to Proceed, authorizing the Contractor to proceed with the construction phase work, will be issued by the Florida DOT.

22.7.2 Preconstruction Conference

The Florida DOT, prior to commencement of construction, will arrange and chair a meeting with the Contractor, Design Professional, major subcontractors, and other interested parties. The purpose of this conference shall be:

- A. To discuss requirements and responsibilities of the various parties involved to achieve expeditious handling of the construction contract.
- B. The instruction of all parties concerning required and standard procedures, required submissions and federal or other regulations that are applicable to the project.
- C. Resolution of all problems as to the scope of the project and the signing of plans and specifications by all parties to the contract.

27.7.3 Periodic Construction Conferences

The Florida DOT shall schedule construction conferences monthly and/or at a frequency at the discretion of the Florida DOT in order to resolve current problems or to ensure that the project

is progressing in a satisfactory manner. It shall be essential that the Contractor and the Architect attend.

These meetings should cover the project as a whole, and certain essential information shall be sought and reported in every conference. The items are:

- A. Progress, as related to the schedule, to be reviewed by the Department and the Contractor prior to the meeting.
- B. Payment Request
- C. Change Orders
- D. The Florida DOT and Architect evaluations of the progress of the work.
- E. Special problems and remedial actions, and results of previous remedial actions.

22.7.4 Shop Drawings, Material, and Equipment Submittals

A. General

It shall be the responsibility of the Architect to expeditiously review all equipment brochures, shop drawings, catalog data, finished hardware schedules, etc., which are required to be submitted by the construction documents. The Architect should review all equipment and material submittals in such a manner as to determine whether or not the materials submitted are being offered as a substitute to those specified, and that the requirements of the contract documents are fully met.

22.7.5 Clarification Drawings and Specifications

A. General

The Architect will be required to issue supplementary drawings and/or specifications clarifying his contract documents as required. Whenever such drawings and specifications are issued by the Architect, he should ensure that said drawings and specifications do not obligate the Florida DOT for a change in scope of the project, either positively or negatively. Such changes in scope must be handled by Supplemental Agreement.

B. Issuance and Transmittal

Whenever the Architect finds it necessary to issue to the Contractor drawings and specifications to modify or clarify the work, a copy shall be supplied to the Florida DOT.

C. Final Inspection

Upon completion of construction and completion of all punchlist items, the Contractor shall arrange and schedule, with the concurrence of the Department, a final inspection of the project. He should coordinate the schedule with representatives of the Department and any other interested party. The Architect shall administer the inspection.

Following the inspection, the Architect shall provide the Contractor with a list of any additional requirements for corrective action and make another inspection, if necessary, to satisfy himself that the work has been completed.

22.7.6 Substantial Completion

A. General

The Architect shall conduct a substantial completion inspection upon notification by the contractor that the project is ready for such. The Architect shall coordinate with all parties in determining the time of the substantial completion inspection (FDOT, A/E, Subconsultants, contractor, etc.).

B. Requirements

Upon completion of the substantial completion inspection, the Architect shall prepare the Certificate of Substantial Completion and punchlist.

Punchlist items shall include all items not satisfactory or not completed at the time of inspection.

C. Approvals and Declaration

Official occupancy of the project will not occur until substantial completion has been declared by the Architect and accepted by the Florida DOT.

22.7.7 Final Acceptance and Contract Close-Out

A. General

The Contractor shall notify the Florida DOT when the project has been fully completed and schedule the final inspection. The Architect shall provide the Contractor with a list of any items requiring correction and ascertain that the items have been corrected. The Architect shall then coordinate and conduct the final inspection and prepare the Certificate of Contract Completion and shall then distribute the certificates to the Contractor, who shall complete all copies and return them to the Florida DOT. The total contract amount must include all Supplemental Agreements.

Drawings and Title Block Requirements

Project Title
Project Location
State Project Number

Phase of Development (i.e., Conceptual Schematics, Advanced Schematics, Design Development, Contract Documents)

State of Florida Department of Transportation

Name of Architect
Address of Architect
Telephone Number of Architect

Sheet Number
Date of Preparation
Initials of Preparer
Signature of Reviewer
Date of Review

Date of Revisions
Initials of Revisers
Signature of Reviewer
Date of Review

On Each Sheet as Applicable Name of Consultant Address of Consultant

PORTATION	PROJECT NO.					N X A DO		
state of florida department of transportation 0FFICE OF SPECIAL FACILITIES			APPROVED BY			Drawing No.		
ORIDA DEPART	COUNTY		Dates A					
STATE OF FL			Names					
o,	ROAD NO.			Designed by	Checked by	Quantities by	Checked by	Supervised by
	REVISIONS	Descriptions						
		Dates						

Note:

The basic organization of the title block (Ex-I-22-B) shall be the same for each drawing, changing the information on each sheet as appropriate.

The sheet number shall always appear either on the lower right hand corner or along the right margin of the drawing.

Three prints of each drawing shall be sealed and signed by the responsible professional so as to produce three record sets of plans.

Title Sheets, Pages and Covers

Note:

The title sheet for drawings and the title page for specifications and/or special provisions should be made as simple as possible without undue embellishment and shall contain the following information:

Drawings, Specifications, Special Provisions

Project Title
Project Location
State Project Number
Work Program Item Number
For State of Florida
Department of Transportation

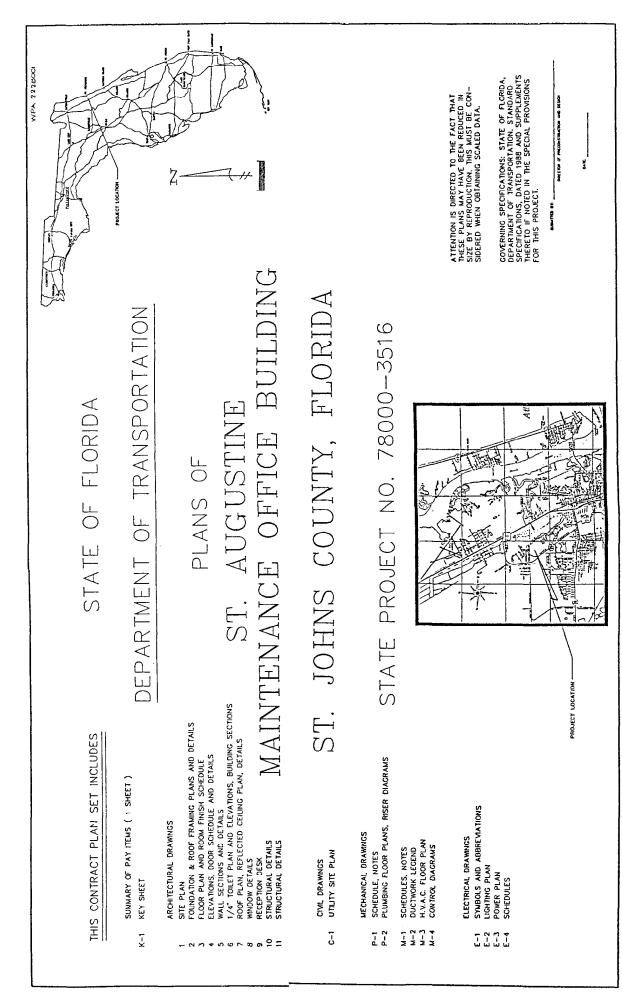
Name of Architect Address of Architect

Date of Publication

Drawings Only

Map of Florida showing project location Range Scale vicinity map showing location of project

EX I-22-C



The covers for the specifications should indicate only the project title, location, state project number and work program item number.

The title sheet for the drawings may serve as the drawings cover or a plain manila sheet may be used for protection.

Specifications (Special Provisions) and drawings shall be bound in such a manner as to discourage separation or removal of pages.

Standard Method of Area Calculation and Space Measurement

Purpose

The purpose of the standard is: (1) to permit communication and computation of building areas and space measurement on a standard clear and understandable basis. (2) to allow comparison of values on the basis of generally agreed upon units of measurement (re: net square footage and gross square footage).

Applicability

This standard shall be used in measuring space in old as well as new buildings, leased space as well as State-owned space. It is applicable to any architectural design or type of construction, being based on the premise that the net area being measured is that which the agency may occupy and use for its furnishings and its people.

Type and Measurement of Space

The following types of space shall be applicable for measurements:

1. Net Usable Area

Net usable area shall be the sum of all occupiable space in a building, that is the measurable area of a building actually having usable value. The area shall be measured from the inside finish of permanent outer building walls to the office side of permanent corridors and/or other permanent partitions, and to

EX-I-22-E

the center of partitions that separate the premises from adjoining usable areas. This usable area shall include; bathrooms, stairs, elevator shafts, flues, pipe shafts, vertical ducts, air-conditioning rooms, fan rooms, janitor closets, electrical closets, and such other rooms not actually available to the tenant for his use.

No deductions shall be made for columns and projections to the building. For glass walls measure from the inside glass face.

2. Public Convenience Area

The public convenience area shall be the sum of all areas devoted to the welfare of the general public such as toilets, vending machine areas, telephone booths, etc.

3. Mechanical Area

The mechanical area shall be the sum of all areas devoted to the mechanical operation of the facility. Spaces shall include mechanical rooms, electrical closets, telephone closets, machinery rooms, pipe and mechanical chases, etc.

EX-I-22-E

4. Building Service and Storage Area

The building service and storage area shall be the sum of all areas within a building devoted to building service, such as janitor's closets, general building storage, maintenance areas and room, receiving areas, loading docks, etc.

5. General Circulation

The general circulation area shall be the sum of all areas within the building devoted to horizontal and vertical circulation, which are for general use and not assignable as net usable area.

6. Exterior Sheltered Area

Exterior sheltered area shall be the sum of all covered walkways, open roofed-over areas that are paved, porches, and similar space. In calculating total footage this area shall be multiplied by a factor of 0.50.

7. Structural Area

The structural area shall be the sum of all areas structurally devoted to the building. Generally it shall be the gross building area less the space designated as areas above including the exterior sheltered area multiplied by a factor of 0.50.

EX-I-22-E

8. Gross Building Area (Architectural Area)

The gross building areas of a building shall be the sum of:

- a. The several floors of the building including basements, mezzanine, and intermediate floored tiers and penthouse of headroom height, measured from the exterior faces of exterior walls or from the centerline of walls separating buildings.
- b. Gross building srea shall not include such features as pipe trenches, exterior terraces or steps, chimneys, roof overhangs, etc.
- c. Void areas such as elevator shafts, mechanical shafts, etc., on each floor shall be included in the gross building area.

9. Exterior Developed Area

The exterior developed area shall be the sum of all hard surfaced specific purpose areas, such as 9decorative walls, plazas, fountains, etc.

10. Gross Project Area

Gross project area shall be the sum of the gross building area and the exterior developed area multiplied by a factor of 0.25.

EX-I-22-E

11. Net Maintainable Area

The net maintainable area shall be the gross project area less the structural area.

NOTE: Generally all area measurements should be taken at a plane 4'5" above finished floor and shall include all partitions in non-usable areas.

EX-I-22-E

AREA	A ANALYSIS - SUMMARY	
Proj	ect Title:	
Proj	ect Number:	
1.	Net Usable Area	+
2.	Public Convenience Area	+
3.	Mechanical Area	+
4.	Building Service & Storage Area	+
5.	General Circulation Area	+
6.	Exterior Sheltered (x 0.50)	+
7.	Structural Area	+
8.	Gross Building (AIA)	+
9.	Gross Project	+
10.	Gross Project	+
11.	Less Structural Area	
12.	Net Maintainable	
Effi	.cient = Net Area (1)	
	Gross Area (8)	

EX-I-22-F

FACILITIES COMPUTATION FORM

overall traffic represented = Percent of = 20 Year ADT (Allow for local commuter traffic.) ≡ ∀

by trucks.

= Ratio of Design Hourly Volume to ADT. (Generally 0.135)

Н × 11 Q

- Directional distribution of Design Hourly Volume

- DIIECLIONAL ALBERTANICIO OL DESTÂN NOMELLO $-$	OT COMMISSION TO	
(Generally 0.6+)		
	Insert Factor	Total
B = Peak hourly directional traffic:	AxKxD=	
C = Traffic composition:		
C1 - Cars (80% to 85%)	% x B =	
C2 - Cars with trailers (3% to 8%)	8 x B =	
Prucks (5% to 13%)	8 x B =	
ດ = B =	100% x B = 1	
D = Vehicles per hour stopping at Rest Area:	_	
D1 - Cars	8 x C1	
(a) Near commercial or recreational facilities (5%)	_	
(b) Typical rural route (9%)		
(c) Information and welcome centers (15%)		
D2 - Cars with trailers (10% to 15%)	8 x C2	
D3 - Trucks (10% to 15%)	% x C3	
D = Total of D1, D2 and D3		
E = Total parking spaces:	_	
E1 - Cars	x D1	
(a) Avg. stop - 15 to 20 min. (0.25 to 0.33 hr.)	_	
(b) At information and welcome centers - 20 to 30 min.		
(0.33 to 0.5 hr.)		
E2 - Cars with trailers - 30 min. avg. (0.5 hr.)	0.50 x D2	
1	0.50 x D3	
F = Persons per hour using comfort facilities:	$D \times 2.25 = 1$	
G = Toilet and urinal fixtures: 2.5 min. avg. use	$0.04 \times F = $	
	יייייייייייייייייייייייייייייייייייייי	

(p) Women (a) Men

 $0.50 \times G$ 0.50 x G

(If the resultant number is uneven, the extra fixture

shall be on the men's side.

Chapter 23

Design Exceptions

23.1 General

Occasionally, it becomes necessary to deviate from the standard criteria used in the design process. When this is the case, documentation and approval may be required. Two specific deviations may occur: (1) design exceptions (2) design concurrence.

It is the policy of the Department to utilize design criteria and standards that are within the ranges established by AASHTO, and the Florida Manual of Uniform Standards for the Design, Construction and Maintenance of Streets and Highways (Green Book).

23.2 Department Design Standards and Criteria

While the Department utilizes AASHTO and the Florida Green Book in establishing criteria, it must be noted that the standards and criteria adopted may exceed the minimum recommended values given in both. The Florida Green Book was the result of Florida Statutes requiring minimum standards for all streets and highways in Florida. While the Department oversees the development of these minimum standards, they do not necessarily represent the minimum standards or criteria adopted by the Department. Department criteria will be found in specific manuals such as this one.

23.3 <u>Design Exceptions</u>

Design Exceptions are required anytime design parameters are specified which fall below the minimums established by AASHTO. For federal aid projects, this requires a formal request for a design exception from the FHWA. This request must be submitted under the District Secretary's or Director's signature. Currently, only the Division Administrator for the FHWA can approve Design Exceptions.

For non federal aid projects, the project file must contain the justification for the exception as developed by the responsible professional engineer.

23.4 <u>Design Concurrence</u>

Design Concurrence is required anytime design parameters are specified which fall below Department established values, but not below AASHTO minimums.

For federal aid projects, this requires approval of the FHWA (usually the Area Engineer) during the plans development stage

This approval should be documented in the project file.

For non federal aid project, it it recommended that the responsible professional engineer document his/her file for future reference, should questions arise.

23.5 <u>Terminology</u>

It is very important that the correct term is used when variations occur. Design Exceptions require specific action and documentation, while Design Concurrence requires less formal action.

APPENDIX A

GLOSSARY OF TERMS

AADT Average Annual Daily Traffic.

ADE Area Design Engineer

ADT (two way) Average Daily Traffic.

Approach Slab A section of a roadway adjacent to,

and at the end of a bridge, requiring special design and

construction considerations.

Arterial A general term denoting a highway

primarily for through traffic,

usually on a continuous route.

A-2 or A-3 Material Materials consisting of sands

deficient in coarse materials and

soil binder.

A-8 Material A national classification of a type

of unsuitable material.

Base course The layer or layers of specified or

selected material of design thickness placed on a subbase or subgrade to support a structural

course.

Baseline An accurately measured line from

which the position of other points may be determined, or on which a

survey may be based.

Benchmark

A relatively permanent object, natural or artificial, bearing a marked point whose elevation above or below an adopted datum is known.

BHRS

Bridge Hydraulic Recommendation Sheet.

Bifurcated Section

A section of a divided roadway separated by a very wide area of natural ground.

Border Width

A term usually used in conjunction with urban roadway cross section denoting the width of cross section from the face of curb to the right-of-way.

Borrow Material

Material excavated from designated areas for use as 'fill'.

Borrow Pit

An excavation site outside the limits of a roadway for producing material necessary for roadway construction.

Bridge Culvert

Culverts whose dimensions exceed a 20' distance measured along project centerline between the inside faces of exterior walls..

Bulkage

Increase in soil volume due to manipulation.

CADD

Computer Aided Design and Drafting.

Catagorical Exclusion:

Projects that may be excluded from the Environmental Impact Process due to the type of work involved example resurfacing projects.

Centerline

The axis along the middle of a road or other facility from which features can be conveniently measured.

CES

Cost Estimating System - The Department's program for estimating construction costs for projects.

Channelization

Usage of traffic islands and other devices to direct traffic into definite paths.

Clear zone

A traversable and unobstructed roadside area available for errant vehicles to safely regain control.

Clearing and Grubbing

Process of clearing the roadway construction site of unwanted features.

Collector

A general term denoting a roadway that links neighborhoods or areas of homogeneous land use with arterial streets.

Compound Curve

A curve consisting of two or more arcs of different radii curving in the same direction and having a common point. Contract

A legal document stating the terms and conditions of an agreement between the Department and a private company to provide a service.

Contract Time

Number of calendar days allowed for completion of the contract, including authorized time extensions.

Contractor

An individual or company that undertakes to provide service specified in contract documents.

Control Radius

Radius by which a turning vehicle can maneuver with the least amount of difficulty.

Crest Vertical Curve

A convex parabolic curve providing a smooth transition between two grades.

Cross Slopes

Lateral slope given to the pavement to provide adequate drainage.

Cross Drain

A drainage structure utilized to convey water from one side of the roadway to the other, including median drains and culverts under intersecting streets.

Crown Line

The inside top of a culvert.

Culverts

A round or special shaped pipe or box used to convey water, especially under roadways or other facilities.

II-A-4

Curb Returns

The curved portion of the curb at which driveways and cross roads intersect with a roadway.

Cut

That portion of a road site where the formation has been excavated below ground level.

Datum

A known or measured point, line or plane to which others may be referred for vertical or horizontal control.

Delineator

Reflector units capable of clearly reflecting light under normal atmospheric conditions from a distance of 1000 feet when illuminated by the upper beam of standard automobile lights.

Design Speed

A speed determined for design and correlation of the physical features of a highway that influence vehicle operation.

Design Exception

Approved deviation from AASHTO or Department criteria.

Detention Area, Basins, and Pond

Drainage basins specially constructed and used to retard stormwater, discharging at a controlled rate for a specific period of time.

DHV

Design Hourly Volume - the traffic volume on which the functional design of a highway is based.

DPI

Ditch Point of Intersection of ditch grades.

Drainage Areas

The portion of the land surface which drains to a specific point, including paved areas, roofs and unpaved land.

Drainage Divides

The area of higher ground separating drainage areas or basins.

Driver Expectancy

A condition whereby drivers are conditioned, by encounters with repetitive features, to expect a certain driving environment. When that environment is provided, driver reaction is very predictable. When expectancy is violated, drivers may react slowly or improperly.

DHW

Design High Water elevation.

Earthwork

The excavation and filling required to construct embankment.

EIS

Environmental Impact Statement

Embankment

The constructed earth fill and excavation built to carry a road.

ESAL

Equivalent single axle load.

Esthetics

Visual impact of the roadway environment on drivers and other vehicle occupants.

Excavation

Removal of all materials of whatever nature to complete earthen cuts, ditching, sub-excavation and borrow pits.

Exceptions

Those portions of the roadway within the project limits that are excluded.

Fill

A portion of the proposed cross section which falls above the existing groundline and indicating volume of fill.

Flow Line

The inside low point or lowest line of water flow in an open gutter, swale, ditch or other drainage element.

Freeway

An expressway with fully controlled access - the highest type of arterial highway.

Friction Course

The top layer of an asphalt pavement to provide resistance to skidding, traffic abrasions and the disintegrating effects of climate.

Functional Classification

Classification of highways by design types based on the major geometric features.

F.A.

Federal Aid - used in conjunction with projects having Federal Aid funds.

Geometrics

Visible elements of a roadway, such as alignment, grades, sight distances, widths, slopes, etc.

II-A-7

Grade

A rate of rise or fall on any length, with respect to horizontal.

G.M.

Gross Mile.

High mast

Free standing poles or towers of height 80' or more utilized for highway lighting to provide uniform, and glare free, light distribution over large areas of highway.

30th Highest Hourly Volume

The hourly volume that is exceed by 29 hourly volumes during a designated year.

Imagery

Visible representation of characters, line drawings and symbols.

K, D and T Values

K: Ratio of DHV to ADT.

D: Directional distribution of DHV expressed as a percentage.

T: Percentage of trucks, inclusive of light delivery, expressed as percentage of DHV.

Lane Taper

Divergence of lane edge for the purpose of adding or dropping lanes.

Lane Transition

Lateral shift of a travel lane.

Iateral Ditch

A ditch which runs more or less perpendicular to the centerline of roadway. LBR

Limerock Bearing Ratio - specifies load bearing capacity of the material, as related to that of limerock.

Level of Service:

A qualitative rating of the effectiveness of a highway in serving traffic, measured in terms of operating conditions.

Leveling Course

One or more layers of asphalt mix used to restore a distorted existing pavement to a uniform cross section and an acceptable level of rideability.

Limited Access R/W

The Right-of-Way wherein the right of owners or occupants of abutting land, or other persons to access a highway facility is limited to designated points, such as interchanges.

May

Permissive condition.

MUTCD

Manual of Uniform Traffic Control

Devices.

MUTS

Manual on Uniform Traffic Studies.

Mylar

Polyester film used as reproducible

drafting media.

National Sign Code

Code numbers assigned to standard

road signs.

N.M.

Net mile.

II-A-9

Overbuild

Multiple layers of asphalt mix used to build up one side of an existing crown to provide a uniform cross-slope.

Overland Flow

Diffused surface flow of water.

Overlay

The construction of a structural course and, if necessary, leveling course and overbuild course, to increase the source life and improve the rideability of an existing pavement.

Overtopping Elevation

Elevation at or above which water will flow over a structure, the highway grade or a drainage divide.

Pavement Design

Description of the types and thicknesses of various layers constituting a pavement structure.

Pay Item Number

Number assigned by the Department to construction components for pay purposes.

PC Station

Point of Curvature Station - station at the beginning of a horizontal curve.

PD & E Study

Project Development and Environmental Study.

Photogrammetry

Photographic process of topographic mapping using stereographic plotters.

PID

Plans In District.

PI Station

Station of the Point of Intersection of two tangents.

Plans

The approved plans, including reproductions thereof, showing the location, character, dimensions and details of the work to be done.

Posted Speed

Regulatory speed limit established in accordance with department policy and posted on the roadway.

Profile Grade Line

A longitudinal line which controls the vertical geometry of the project, usually the inside edge of a divided highway or the centerline of an undivided highway.

Profile Grade Point

A specific point along the Profile Grade Line.

PS & E

Plans, Specifications and Estimate.

PT Station

Point of Tangent Station - station at the termination of a horizontal curve and at the beginning of the tangent. Quality Assurance

Is all planned and systematic actions necessary to provide adequate direction so that all resulting design products can meet predetermined requirements. This includes the establishment of design policies, procedures, standards, guidelines, training and monitoring for compliance.

Quality Control

Following established design policies, procedures, standards and guidelines in the preparation of all design products. This includes the checking and review of individual designs for compliance and good engineering practice.

Ramp

That portion of the traveled way connecting two roadways at a grade separated intersection.

Range

An area of 36 square miles enclosed between nationally established survey lines running north-south, six miles apart, and township lines.

Recovery Area

See "Clear Zone".

Reference Points

One of several fixed objects for which measurements are made to enable a point to be accurately located. Resurfacing A supplemental or replacement

surface placed on an existing pavement to improve its surface or

increase its strength.

Retention Area, Basin or Pond A drainage facility designed to

retain runoff without a direct

outlet discharge structure.

Returns That extension of the roadway which

allows entrance and exit to

sidestreets, parking lots, etc.

Reverse Curve A combination of two horizontal

curves in opposite directions with

a common tangent.

3R Resurfacing, Restoration,

Rehabilitation of a roadway.

R/W The areas, existing or acquired by

permanent easement, for highway

purposes.

Sag Vertical Curve A concave parabolic curve providing

a smooth transition between two

grades.

Section Lines Established survey grid lines

enclosing approximately a one mile

square area of land.

Shall Mandatory condition.

Shop Drawings Detailed drawings of elements

requiring special fabrication.

Should

Advisory condition.

Shoulder

The portion of the roadway contiguous with the traveled way and used for lateral support of base and surface courses, emergencies and safe recovery of errant vehicles.

Shrinkage

Reduction in volume of soil mass.

Sidedrain

A drainage structure placed more or less parallel to the centerline of a roadway for conveyance of water under driveways, and other such obstructions.

Soil Survey

The exploring and recording of soil types and conditions.

Special Ditch

Roadside ditch whose dimensions do not conform to those shown on the typical section.

Special Provisions

Special directions, provisions or requirements peculiar to the project under consideration and not otherwise thoroughly or satisfactorily detailed or set forth in the specifications.

Specifications

Document containing the directions, provisions, requirements and stipulations relating to the method and manner of performing the work.

Speed Change Lanes

Special lanes provided for the use of accelerating or decelerating vehicles.

Station Equations

Station along an alignment where the numerical continuity is broken.

Storm Sewer or Storm Drain

Pipe system or portion thereof used to collect or convey storm water runoff.

Stabilizing

Process by which the subbase is brought up to a bearing value sufficient to support the base.

Structural Course

One or more layers of asphalt mix placed to provide the major structural component of the pavement or to increase the service life of an existing pavement.

Subbase

The layer or layers of specified or selected material of designated thickness placed on a subgrade to support the basecourse.

Subgrade

The top surface of a roadbed upon which the pavement structure and shoulders are constructed.

Superelevation

A tilt given to a road at a horizontal curve to counteract the effect of centrifugal force.

Superelevation Transition

Transition of a cross section from normal cross slope to full superelevated cross slope, or vice versa.

Surface Course

One or more layers of a pavement structure designed to accommodate traffic load.

Survey Reference Points

Same as reference point.

Template

The sum of elements of widths, depths and cross slopes which define the roadway cross section.

Topography

Representation, on a plan, of the existing physical features in an area.

Township

An area of 36 square miles enclosed between nationally established survey lines running east-west, six miles apart, and range lines.

Travelway

The portion of the roadway for the movement of vehicles, exclusive of shoulders and auxiliary lanes.

Traversable

Crashworthy roadside conditions that would allow an errant vehicle to regain control without serious damage.

Turning Radius

Outside wheel path of a turning vehicle.

Typical Section

Shows the design elements for the cross section of a proposed roadway.

Underdrains

A subsurface drainage system.

Unsuitable Material

Types of dirt that are classified unsuitable for roadway construction.

Value Engineering

An analysis of materials, processes and products in which functions are related to costs and from which a selection may be made for the purpose of achieving the required function at the lowest overall cost consistent with the requirements for performance reliability and maintainability.

Vellum

Translucent paper used as reproducible drafting media.

Vertical Curve

A parabolic curve used to give smooth transition between tangent grade-change. Weaving Movement

The crossing of traffic streams moving in the same general direction, accomplished by merging and diverging.

W.P.I. Number

Work Program Item number (assigned by the Department).

INDEX

VOLUME I

PLANS PREPARATION AND ASSEMBLY

2D Dogiem	1_6 12_5
3R Design 3R Projects	1-5, 13-5 16-2
	14 - 6
Accident Data	
Advance Warning Arrow Panels	10-20
Advisory Speeds	10-55
Aesthetics	1-9
Architectural Plans	22-1
Area Design Engineer	17-2
Auxiliary Lanes	2-8
Bicycle Facilities	8-1, 8-5
Bicycle Lanes	2-8
Bicycles	8-1
Border Width	2-14
Borrow	3-1, 3-2
CADD	20-9
Canals	4-5
Capacity	1-3
CES	18-1
Channelizing Devices	10-25
Clear Zone	4-1
Clearing and Grubbing	3-6
Codes	22-3
Comprehensive Planning	22-5
Consultant Design	13-8
Consultants	17-3, 21-1
Contract Administration	21-4
Contract Documents	22-35
Contract Time	18-2
Controller Timing	7-16
Crash Cushions	4-16, 10-28
Cross Sections	2-23, 2-28, 2-35
Cross Slopes	2-14
Crossing Surfaces	6-3
Data Collection	14-1
Design Consistency	1-6
Design Plans	15-2, 16-1
	15-2, 10-1 1-1
Design Parameters	
Design Speed	2-2
Design Standard Ranges	2-2
Detours	10-47

Ditahan	0.34
Ditches	2-14
Driver Expectancy	1-6
Drop-offs	10-49
Earthwork	3-1
Embankment	3-1
End Treatments	4-9, 10-28
Environmental Documents	14-5
Final Design	13-3
Final Dressing	3–6
Five Year Work Program	13-1
Fixed Standards	2-1
Flaggers	10-31
Friction Course	2-12
Functional Classification	1-4
Geometrics	2-31
Handicap Access	8-4
Horizontal Alignment	2-17
Horizontal Curves	2 - 17
HVAC	22 - 26
In-house Design	13- 7
Interchanges	2-24
Intersections	2-24, 7-19
Landscaping	9-1
Iane Closure Analysis	10-35
Lane Widths	2-4, 10-35
Layouts	10-32
Left Turns	7-16
Level-of-Service	1-3
Lighting	7-6
Limited Access	2-25
Mailbox Supports	4-20
Marking	7-2
Median Barriers	4-15
Original Plans	14-6
Passing Sight Distance	2-3
Pavement Design	14-5, 15-13
Pavement Marking	10-26
Pay Items	11-1, 18-1
PD & E Studies	1-2, 10-3, 13-2
Pedestrians	8-1
Permits	15-13, 22-6
Plans Processing	17-4
Plans Review	16-1
Plan Revisions	20-1
Pole Design	7-8
Profiles	2-26, 2-32
Project Concept Report	21-2
Project Cost	14 - 2
Project Description	14-2
Project Description Project Development	13-1
Project Letting	13-3
Froject recting	12-2

- · · · · ·	
Project Number	14-2
Project Scheduling	13-9, 14-2, 21-4
Quality Assurance	17-1
Quantities	11-1
Railroad Crossings	6-1
Ramp Widths	2-8
Regulatory Speeds	10-51
Right-of-Way	12-1, 15-15
Roadside Barrier	4-8
Rural Projects	2-26
Scope of Services	21-2
Sewage	22-5
Shoulder Widths	2-9
Side Slopes	2-14
Sidewalks	8-2
Sight Distance	10-34
Sign Supports	4-19
Signal Preemption	7 - 19
Signing	7-19 7-2
Signing and Sealing	7-2 19 - 1
Signs	
	10-17
Specifications	11-1
Speed Zoneing	10-51
Stop Line	7-15
Stopping Site Distance	2-3
Structural Clearances	6–4
Structural Plans	15-5
Submittals	15-1
Subsoil Excavation	3–6
Superelevation	2-22, 10-34
Surveys	14-3, 15-8
Taper Lengths	10-32
Temporary Barrier Walls	10-27
Through Lanes	2-4
Traffic Barriers	10-27
Traffic Control Devices	7-1, 10-16
Traffic Control Plan	10-5, 10-8
Traffic Data	1-2, 14-5
Traffic Signals	10-24, 7-14
Training	10-14
Transitions	2-5, 2-20, 4-11
Truck-Mounted Attenuator	10-29
Typical Sections	15-8
Underdeck Lighting	7 - 9
Urban Projects	2-30
Utilities	
Utility Coordination	5-1
Utility Pologation	5 - 8
Utility Relocation	5 - 2
Value Engineering	15-17
Variable Message Signs	10-21

Verification of Utilities	5-4
Vertical Alignment	2-21
Warning Lights	10-19
Wide Curb Lanes	2-6
Work Zone Traffic Control	10-1
Zoning	22- 5