

Criteria for Selecting Numeric Hydraulic Modeling Software (NCHRP 24-24)

Mark Gosselin, P.E., Ph.D.

Shawn McLemore, P.E.

Philip Dompe, P.E.

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OEA, Inc.

Purpose

Develop a Decision Analysis Tool and
Guidelines for Selecting the Most
Appropriate Numerical Model for
Analyzing Bridge Openings

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Approach

- Phase I
 - Literature Review and Survey of the State of the Practice
 - Commonly Employed Software
 - Site Conditions and Design Requirements that Affect Model Selection
 - Locate Appropriate Data Sets for the Verification Stage of This Work
 - Synthesize Results
 - Sensitivity Testing
 - Applicability of Models for Various Site Conditions
 - Determined from Survey/Literature Review
 - Develop Preliminary Decision Tool
 - Interim Report
- Phase II
 - Additional Tool Development
 - Perform Example Application
 - Final Report

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Literature Review

- Purpose – Compare/Contrast Applications of One-dimensional and Two-dimensional Models at Bridge Crossings
- Very Little Literature Exists
- Literature Review Revised:
 - Commonly Applied One- and Two-dimensional Models
 - Case Studies
 - Comparison Articles

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Email Survey

- Short and Focused
- Identify
 - Models Employed
 - Common Problems
 - Data Sets
- Sent to State DOTs and FHWA Personnel (80 people)
- Received 47 Responses (42/50 DOTs)

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Survey Results – Models

Model	Riverine				Tidal			
	1 st	2 nd	3 rd	4 th +	1 st	2 nd	3 rd	4 th +
HEC-RAS	36	3			9			
WSPRO	3	13	8		1			
SWMM			1	1	1			
HEC-2		15	6			7		
UNET				1				
E-431				2				
Bri-Stars	1		1	1				
HEC-6			2					

Model	River				Tidal			
	1 st	2 nd	3 rd	4 th +	1 st	2 nd	3 rd	4 th +
FESWMS	18				6	2		
RMA2		4			2	2		

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Survey Results - Other

- 26% Have Model Selection Guidelines
- 47% of Agencies Prohibits or Discourages Use of Specific Programs
 - Only Accept Specific Models
 - HEC-RAS
 - HEC-2
 - FESWMS

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Synthesis of Review

- Several Common Themes
- Ability to Test Factors
- Combinations of More Quantifiable Factors
- Measure of Modeling Accuracy
- Factors:
 - Multiple Openings
 - Bridges Located on River Bends
 - Bridges near Confluences
 - Bridges with Significant Constrictions
 - Overtopping Flow
 - Embankment Skew
 - Bridges over Meandering Rivers
 - Bridges with Asymmetric Floodplains
 - Bridges with Large Piers/High Blockage
 - Tidal Hydraulics

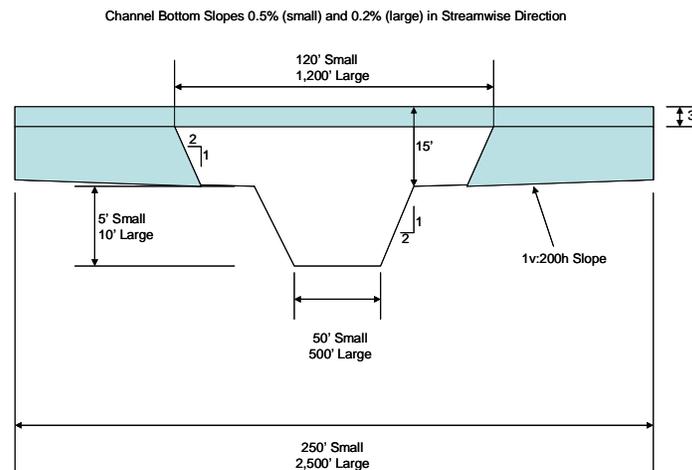
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Sensitivity Testing Plan

- One-dimensional Model: HEC-RAS
- Two-dimensional Model:
 - FESWMS – Steady State Flows
 - RMA2 – Time Dependent Flows
- Baseline Geometry
 - Small Channel
 - Large Channel

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Baseline Geometry



Sensitivity Testing Plan

- 10 Factors Tested
- Boundary Conditions:
 - Flow :
 - 95,000 cfs (Large)
 - 5,000 cfs (Small)
 - Elevation: Adjusted Such That
 - ~10 ft Normal Depth at Bridge (Small)
 - ~15 ft Normal Depth at Bridge (Large)
- 88 Individual Conditions, 176 Individual Simulations

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Modeling Approach

- Two dimensional models developed first
 - Average Water Density = 1.937 slugs/ft³
 - Unit Flow Convergence = 0.01 to 0.001
 - Unit Water Depth Convergence = 0.01 to 0.001
 - Depth Tolerance for Drying = 0.25 to 0.5 ft
 - Manning's n (Constant with Depth)
 - 0.025 for the Channel
 - 0.045 for the Roadway Embankments
 - 0.75 for the Overbank Areas
 - Constant Eddy Viscosities of 5 to 10 ft²/sec in the Channel and 10 to 50 ft²/sec on the Embankments.
 - Small Relaxation Factor and a High Number of Iterations to Ensure Convergence

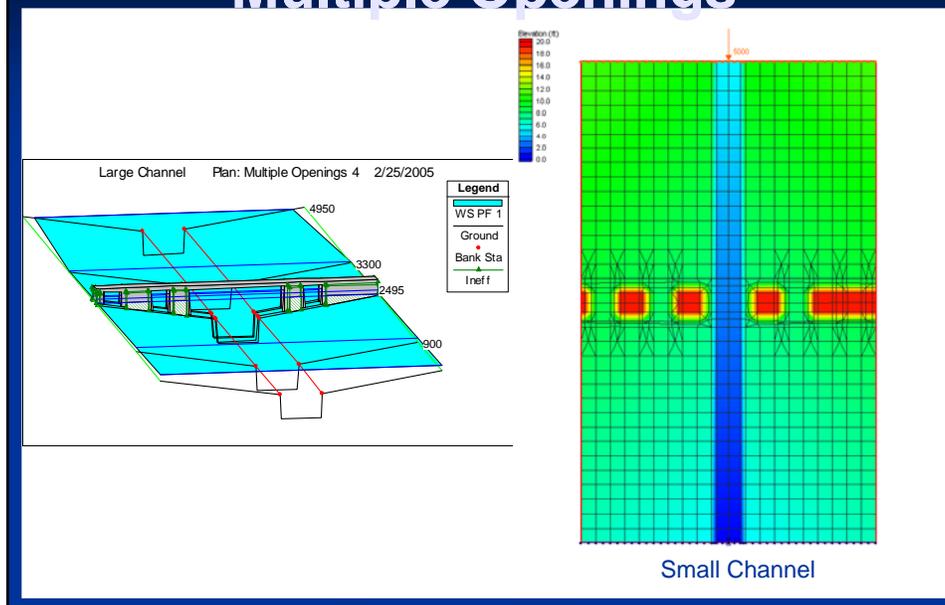
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Modeling Approach

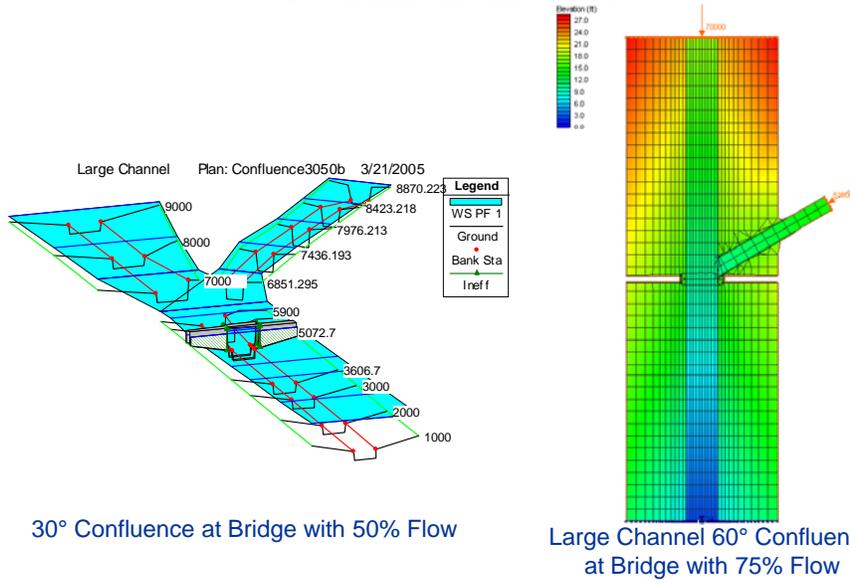
- Next, 2-D Mesh Provided to 1-D Modeler
 - Energy (Standard Step) Bridge Method
 - Cross Sections near Bridge Located According to the HEC-RAS Users Manual and the Applications Guide
 - Expansion and Contraction Coefficients = 0.1 and 0.3, Respectively
 - Roughness Values = 2-D model Values, Except Higher Roughness for Abutments
 - Boundary Conditions (Flow and Starting Water Surface Elevation) Matched 2-D Model

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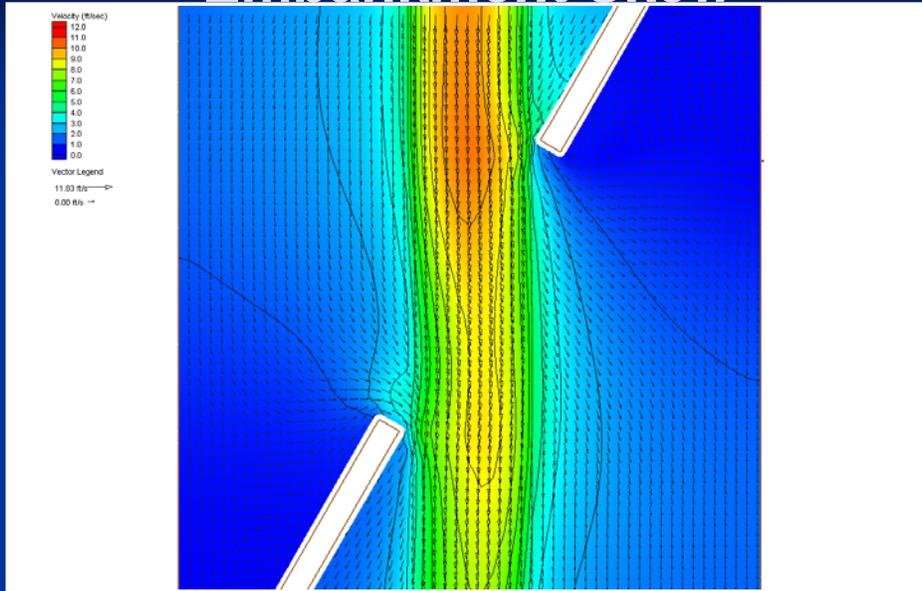
Multiple Openings



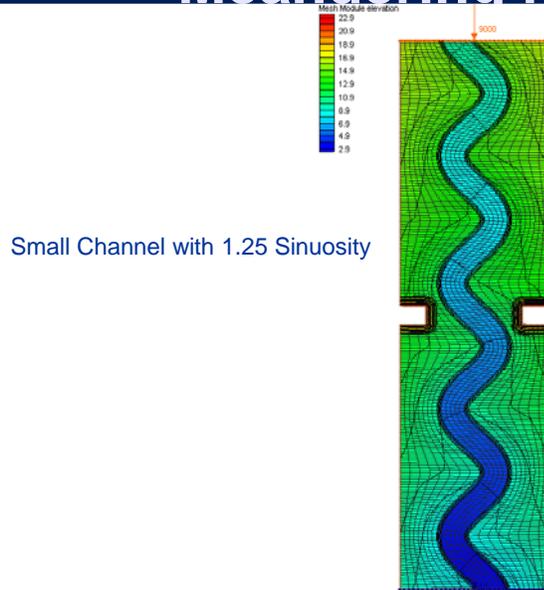
Confluences



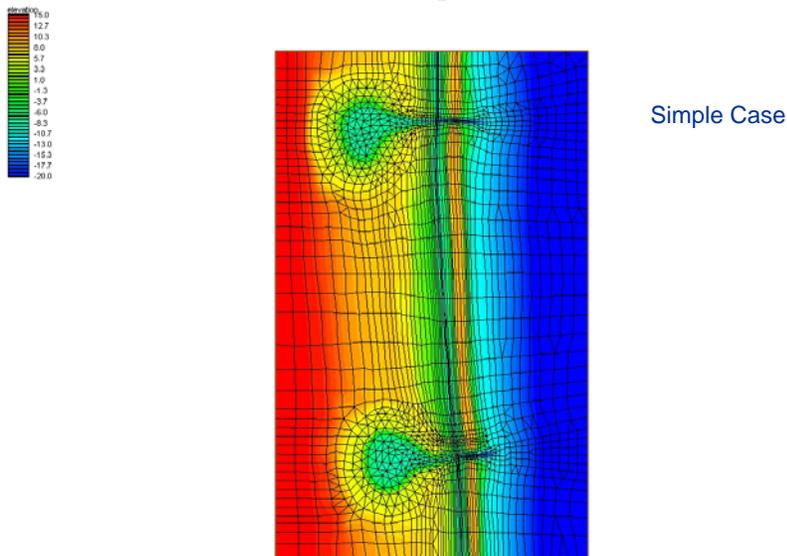
Embankment Skew



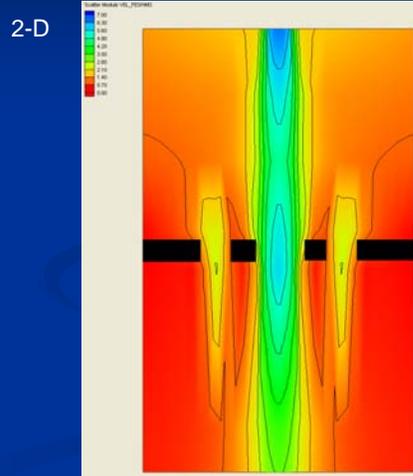
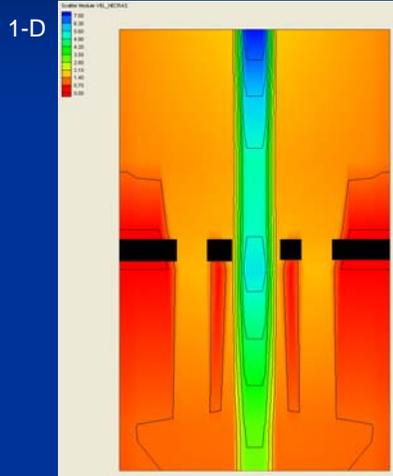
Meandering Rivers



Tidal Hydraulics

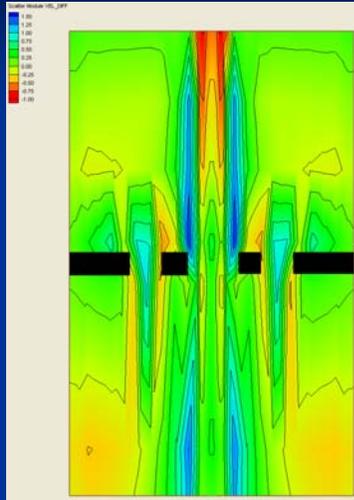


Velocity Magnitude Contours

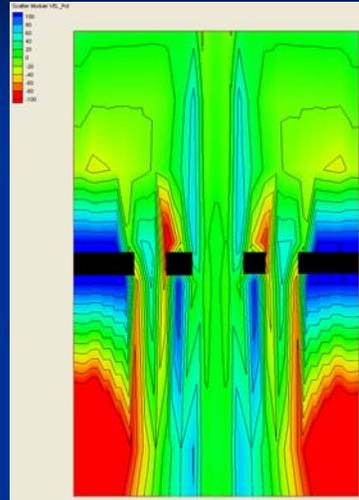


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Difference Contours



Absolute Difference

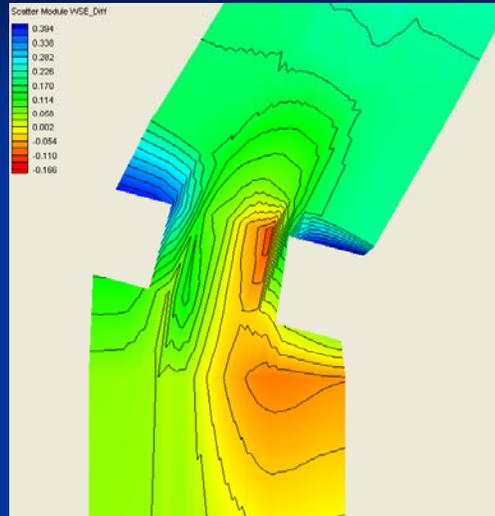


Percentage Difference

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Difference Contours

- Small Channel
- Half Radius
- 30° Bend
- WSE Absolute Difference Contours



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Results – General Comments

- Large Differences in Flow Direction Results
- Better Agreement the More “One-dimensional” the Flow
 - Multiple Openings
 - Overtopping
- Labor Averages
 - One-Dimensional Models — 3.1 hrs/model
 - Two-Dimensional Models — 2.4 hrs/model
 - Averages Skewed by Tidal Cases and Modeler Experience

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Decision Tool Development

- Model Selection Based on:
 - Specific Site Conditions
 - Design Applications
 - Data Availability
 - Modeler's Experience and Resources
- Complex Process
- Requires Multi-pronged Approach

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Approach

- Increase Engineer's Awareness with Report
 - Discussion of Model Theories
 - Comparison of Relative Performance
- Provide Design Examples/Sensitivity
 - Study Types (e.g. Flooding, Scour Eval.)
 - Structure Design (e.g. Slope Protection)
- Decision Tool & Guidelines

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Design Criteria Considerations

- Riprap Sizing for Scour, Abutment, or Slope Protection
- Armor Units for Scour, Abutment, or Slope Protection
- Concrete Block for Scour, Abutment, or Slope Protection
- Abutment Scour Calculation
- Pier Scour Calculation
- FEMA “No-Rise” Studies
- Bendway Weirs/Stone Spurs

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Other Project Considerations

- Available Resources
 - Available Software
 - Software Features
 - User Experience
- Outside Constraints
 - Schedule
 - Data Availability

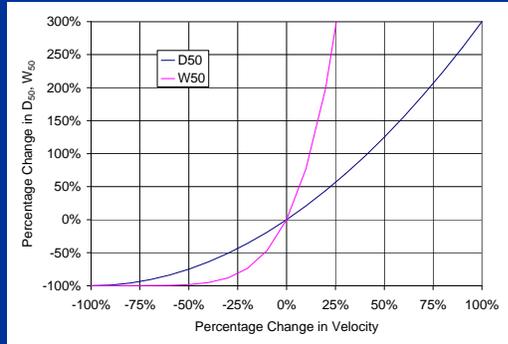
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$$\frac{D_{50}}{y} = \frac{K}{(S_s - 1)} \left[\frac{V^2}{gy} \right]$$

Design Issues Example

- Riprap Sizing for Bank Slope Protection
- HEC-23: Ishbash Equation

$$\frac{D_{50}}{y} = \frac{K}{(S_s - 1)} \left[\frac{V^2}{gy} \right]$$



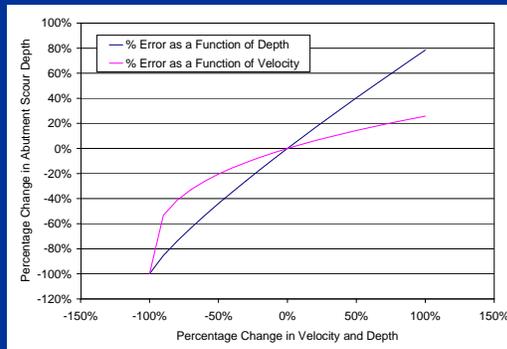
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$$\frac{y_s}{y_1} = 4Fr_1^{0.33} \frac{K_1}{0.55} K_2$$

Design Issues Example (continued)

- Abutment Scour Calculation
- HEC-18: HIRE Equation

$$\frac{y_s}{y_1} = 4Fr_1^{0.33} \frac{K_1}{0.55} K_2$$

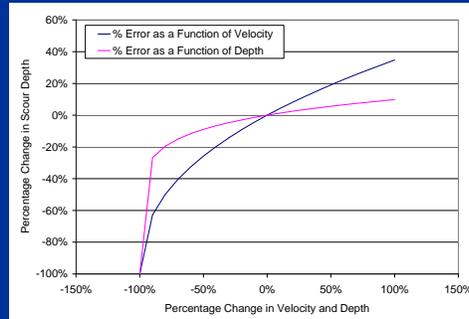


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Design Issues Example (continued)

- Local Scour Calculation
- HEC-18: CSU Equation

$$\frac{y_s}{y_1} = 2.0K_1K_2K_3K_4 \left(\frac{a}{y_1} \right)^{0.65} Fr_1^{0.43}$$



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Decision Tool

- Decision Matrix – 4 Step Development
 - 1) Identification of Alternatives
 - 1-D vs. 2-D
 - 2) Identification of Decision Criteria
 - Site Conditions
 - Design Considerations
 - Project Considerations

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Decision Tool

- 3) Weighting the Criteria
 - Critical Evaluation by Engineer
 - Relative Importance
 - Assignment of Weights
- 4) Scoring System/Tool Application
 - Model Performance
 - Design Sensitivity
 - Project Specifics/Engineer Qualifications

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Design Criteria	Weight	One Dimensional Model		Two Dimensional Model	
		Score 1=low 3=medium 5=high	Weight x Score	Score 1=low 3=medium 5=high	Weight x Score
Site Conditions	(1-10)				
Multiple Openings					
Bridges Located on River Bends					
Bridges near Confluences					
Bridges with Significant Constrictions					
Overtopping Flow					
Embankment Skew					
Bridges over Meandering Rivers					
Bridges with Asymmetric Floodplains					
Bridges with Large Piers/High Blockage					
Tidal Hydraulics					
Design Requirements	(1-10)				
Riprap					
Armor Units					
Concrete Block					
Abutment Scour Calculation					
Pier Scour Calculation					
FEMA "No-Rise"					
Bendway Weirs					
Other Considerations	(1-10)				
Modeler Experience					
Scheduling					
Data Availability					
Totals (Sum of Weight x Score)					

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Design Criteria	Weight	One Dimensional Model		Two Dimensional Model	
		Score	Weight	Score	Weight
		1=low 3=medium 5=high	x Score	1=low 3=medium 5=high	x Score
Site Conditions	(1-10)				
Multiple Openings	4	3	12	3	12
Bridges Located on River Ends	10	3	30	5	50
Bridges near Confluences					
Bridges with Significant Constrictions					
Overtopping Flow					
Embarkment Skew					
Bridges over Meandering Rivers					
Bridges with Asymmetric Floodplains	5	3	15	5	25
Bridges with Large Piers/High Blockage					
Tidal Hydraulics					
Design Requirements	(1-10)				
Equip	10	3	30	5	50
Armor Units					
Concrete Block					
Abutment Scour Calculation					
Pier Scour Calculation	10	3	30	5	50
FEMA "No-Rise"					
Bendway Weirs					
Other Considerations	(1-10)				
Modeler Experience	8	5	40	1	8
Scheduling	10	5	50	1	10
Data Availability	2	3	6	3	6
Totals (Sum of Weight x Score)			213		211

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Example

Highway 130 Bridge over Buckhorn Creek



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Example

- Step 1 – Identify Alternatives
 - Choosing between FESWMS and HEC-RAS
- Step 2 – Identify Decision Criteria
 - Asymmetric Floodplains
 - Bridges over Meandering Rivers
 - Riprap
 - Pier Scour Calculation
 - Modeler Experience
 - Scheduling
 - Data Availability

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Example

- Step 3 – Weight the Criteria

Design Criteria	Weight	One Dimensional Model		Two Dimensional Model	
		Score 1=low 3=medium 5=high	Weight x Score	Score 1=low 3=medium 5=high	Weight x Score
Site Conditions	(1-10)				
Bridges over Meandering Rivers	2				
Bridges with Asymmetric Floodplains	7				
Design Requirements	(1-10)				
Riprap	9				
Pier Scour Calculation	3				
Other Considerations	(1-10)				
Modeler Experience	3				
Scheduling	10				
Data Availability	3				
Totals (Sum of Weight x Score)					

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Example

■ Step 4 - Scoring

Design Criteria	Weight	One Dimensional Model		Two Dimensional Model	
		Score	Weight x Score	Score	Weight x Score
		1=low 3=medium 5=high		1=low 3=medium 5=high	
Site Conditions	(1-10)				
Bridges over Meandering Rivers	2	3	6	5	10
Bridges with Asymmetric Floodplains	7	3	21	5	35
Design Requirements	(1-10)				
Riprap	9	3	27	3	27
Pier Scour Calculation	3	3	9	3	9
Other Considerations	(1-10)				
Modeler Experience	3	5	15	5	15
Scheduling	10	5	50	3	30
Data Availability	3	5	15	5	15
Totals (Sum of Weight x Score)			143		141

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Summary

- Resources for Engineers
 - Comparison of Models
 - Examination of Theory
 - Sensitivity Test
 - Consequences of Selection
 - Study Type
 - Design Issues
 - Framework for Selection – Decision Tool

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Questions, Comments?

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