FLORIDA DEPARTMENT OF TRANSPORTATION

STRUCTURES DETAILING MANUAL

FDOT STRUCTURES MANUAL
VOLUME 2
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Structures Design
Florida's Transportation Engineers

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INTRODUCTION

I.1 GENERAL

A. The ***FDOT Structures Detailing Manual***, (***SDM***) is Volume 2 of the ***Structures Manual***. See the ***Structures Manual Introduction*** for additional information including authority, scope, distribution and process for modifications to the ***Structures Manual***.

B. This volume of the Structures Manual provides guidance for drafting and detailing criteria and methods for use in preparing Florida Department of Transportation (FDOT) contract plans for structural elements or systems. These elements or systems include bridges, overhead sign structures, earth retaining structures and miscellaneous highway structures. The SDM includes preferred details and examples of general component plan sheets. The information required to be shown on each type of sheet includes but is not limited to the items listed for the sheet in the SDM chapters.

C. Additional information on overhead sign structures, high-mast light poles, and miscellaneous roadway appurtenances as well as general administrative, geometric, shop drawing, and plans processing may be found in the ***Plans Preparation Manual***, ***(PPM)*** Topic Nos. 625-000-007 and 625-000-008.

D. As a supplement to the ***SDM***, a series of examples which represent some of the many situations a designer may encounter when designing a bridge has been compiled. These ***Structures Detailing Manual Examples*** are provided in a separate PDF document that is referenced in links throughout the ***SDM***. The information presented in the ***SDM*** figures and examples is meant to convey detailing and organizational requirements and not to present actual design examples.

I.2 FORMAT

A. The ***SDM*** is presented in both 8 1/2-inch x 11-inch and 11-inch x 17-inch formats consisting of text, figures, charts, graphs and example drawings. This Manual provides standard engineering criteria and guidelines to be used in the development of engineering drawings of structures for which the Structures Design Office (SDO) has responsibility.

B. The ***SDM*** is intended to be used in conjunction with the ***Structures Design Guidelines (SDG)***, ***Design Standards***, ***Plans Preparation Manual, (PPM)*** (Topic Nos. 625-000-007 and 625-000-008), and the ***CADD Production Criteria Handbook (CPCH)***.

C. The ***SDM*** is written primarily in the active voice to Structural Designers, Professional Engineers, Engineers of Record, Structural Engineers, and Geotechnical Engineers engaged in work for the Florida Department of Transportation.

D. Refer to the ***CPCH***, Chapter 18, for FDOT Drafting Standards related to file naming convention, level names, line weights, line styles and color, text fonts, size, weight and color and dimension styles for Structures QC electronic plan submittal requirements.

I.3 COORDINATION

A. Coordinate all plans production activities and requirements between the ***Structures Manual*** (all Volumes), ***PPM***, and ***AASHTO LRFD Bridge Design Specifications (LRFD)***. Each of these documents has criteria pertaining to bridge or structures design projects, and normally, all must be consulted to assure proper completion of a project for the Department.

B. Direct all questions concerning the applicability or requirements of any of these or other referenced documents to the appropriate FDOT Structures Design Engineer. For a list of Structures Contacts, see the contacts section of the SDO website.

I.4 QUANTITY CALCULATIONS

A. The ***Basis of Estimates Handbook*** describes the method of measurement, basis of payment and required rounding accuracy for frequently used items.

B. Show all quantities on the Computation Book Forms. Include pay item number, description, units and dimensions used if applicable.

C. Show quantity breakdown for individual components by construction phase, such as end bents, diaphragms, deck, traffic barriers, expansion joints, bearings, reinforcing steel, etc. in the estimated quantity blocks located on corresponding plan detail sheets.

D. Show quantities on plan detail sheets to one additional decimal place of accuracy. See Chapter 18 of the ***CPCH*** for directory location and file format for hand calculations and computer input and output requirements.

E. Complete each quantity block and deliver all calculations with the design notes.

I.5 SUMMARY OF BRIDGE PAY ITEMS

A. The estimated bridge quantities are given in the "Summary of Bridge Pay Items" sheet. When this sheet is used, include it immediately following the Bridge Key Sheet. The item numbers, descriptions, units, quantities, and totals will be verified before entry into the Department's Cost Estimating System (TRNS*PORT). Include a separate quantity column for each bridge in the project.

B. Submit quantity booklets and a summary of the estimated bridge quantities with the final submittal package. Refer to the Department's ***Basis of Estimates Handbook*** for guidance in preparing the quantity booklets and the summary of quantities.

C. Input estimated bridge quantities (for each bridge) into the Department's computerized Cost Estimating System (TRNS*PORT) and check the printout.

D. Place quantity calculations for retaining walls, critical temporary walls, and sound barriers in the Roadway Computation Book. Include pay items in the Roadway portion of TRNS*PORT.

E. Review the final Contract Documents to ensure that a clear method of payment is conveyed for all items of work on the project either by the appropriate pay item or in the pay item notes.

I.6 PHASE SUBMITTAL REQUIREMENTS

During the design process, submittals will be made at various stages of project development. The purpose of these phase submittals is to ensure the design meets the Department's intent for a given project and that the work done matches the scope agreed upon at the initiation of the contract. For a summary of phase submittals, see ***Plans Preparation Manual***, (***PPM***), Volume 1, Chapter 26.

1 DRAFTING AND PRINTING REQUIREMENTS

1.1 GENERAL

This chapter contains references to general detailing standards and requirements for various bridge components. The instructions are also applicable to most other highway related structures such as retaining walls, pile-supported roadways, etc. Refer to the ***CADD Production Criteria Handbook*** (***CPCH***) for specific instructions related to computer aided drafting.

1.2 DRAFTING REQUIREMENTS

A. FDOT requires the use of a CADD system for plans production.

B. Differentiate outline and dimension line weights.

C. The relative line weight and the chosen lettering should provide perfect legibility when reproduced by normal printing procedures.

D. See the ***Plans Preparation Manual*** (***PPM***) and the ***CPCH*** for additional drafting requirements.

1.3 QUALITY ASSURANCE

Chapter 18 of the ***PPM***, Volume 1, explains the processes for Quality Assurance and Quality Control. Chapter 20, ***PPM*** Volume 1, discusses acceptable printing methods, paper size, and quality of the media and print. From time to time, the Department may conduct an audit of a firm's Quality Assurance/Quality Control (QA/QC) process to ensure the QA/QC plan outlined at the beginning of the project is being followed. All QA/QC documentation, including check prints, design calculations, quantity computations, etc., should be kept on file until construction of the project is complete at a minimum.

1.4 DRAWING REVISIONS

A. When changes are required prior to contract award, follow the procedures outlined in Chapter 20 of the ***PPM***, Volume 1.

B. When changes are required after the contract award, follow the procedures outlined in Chapter 4 of the ***Preparation and Documentation Manual***.

2 DETAILING INSTRUCTIONS

2.1 DETAILING AIDS

The following detailing aids can assist the detailer in efficiently creating a set of plans.

A. When using FDOT programs (Reinforcing Bar Lists, etc.), use the forms and user's manuals provided by the Structures Design Office and/or the Engineering CADD Systems Office as listed on the following web sites: http://www.dot.state.fl.us/structures; http://www.dot.state.fl.us/ecso/

B. Plan the drawings by determining which details and information need to be placed on each sheet, the scale to be used, the number of sheets required and the sequence of the sheets.

C. The Structures Cell Library contains many commonly used cells in the development of structures plans and is located in the Structures Bar Menu. The Structures Bar Menu is available through the Engineering CADD Systems Office (ECSO). Maintenance releases for the Bar Menu are available on the ECSO website. Structures cells are posted on the FDOT Structures website. To receive notifications of changes to the Structures Cell Library and other important updates, sign up for the FDOT Contact Mailer list.

2.2 STRUCTURES IDENTIFICATION NUMBERS

A. FDOT assigns identification numbers to bridges, overhead signs, high-mast light poles and traffic signal mast arms. Structures supporting Intelligent Transportation System (ITS) equipment may be similar to overhead signs or high-mast light poles, these structures are also assigned identification numbers.

B. Contact the District Structures Maintenance Engineer early in the design process to obtain structure identification numbers.

C. New numbers will be assigned to all new and replacement bridges. Widened bridges generally retain their existing numbers. If the widening joins existing structures, the District Structures Maintenance Engineer will decide which bridge number to retain.

D. Show the bridge number on the lower right side above the Title Block of all sheets. For ancillary structures (overhead signs, high mast light poles and traffic signal mast arms) the structure identification number should be shown on the appropriate plan sheet, including data table sheets, above the title block.

E. Place the bridge number of any bridge that shows up in the plan view, including existing bridges to be removed, on the actual bridge. Only the subject bridge number should appear in the lower right corner of the sheet.

2.3 BRIDGE LENGTHS AND HORIZONTAL CONTROL

A. A bridge's length is the distance measured along the Station Line between begin and end of bridge (front faces of end bent backwall or end of adjoining approach slabs for end bents with no back wall.)

B. Horizontal Control Lines

   1. Alignment Line: Show the alignment control line that applies within the limits of the bridge.
   2. Station Line: This is the horizontal control line from which basic distances, lines and angles are referenced for locating bridge components in the field. This line is usually the same line as the Alignment Line. Use the centerline of construction (℄), Base Line Survey, Profile Grade Line or Baseline (℄) to show the stations along the project. Refer to this as the "Station Line."

2.4 FINANCIAL PROJECT NUMBER AND FEDERAL-AID PROJECT NUMBER

A. Show the Financial Project Identification Numbers (FPID) in the Title Block on all bridge plans. Place the FPID Number on the existing bridge plans if included in the submittal.

B. Do not show Federal-Aid Project Numbers (F.A.P. No.) on the bridge plans. Federal-Aid Project Numbers should be shown on the Key Sheet only.

2.5 INITIAL BLOCK

Include the initials or name of the person performing each function and the date completed for each sheet. If a function is not applicable, place a dash through the name and date block.

2.6 TITLE BLOCK

In upper case letters, include the following information in the title block, of each plan sheet:

A. Sheet title

B. Project Name (a project description and bridge location)

C. Sheet Number

D. Initials (Detailers', Designers' and Checker’s)

E. EOR information (Consultant or FDOT logo with Engineer of Record's name, address, PE License number and consultant Certificate of Authorization number)

F. Financial Project ID Number

G. County (or counties, if project covers more than one county)

H. Road number

2.7 ORTHOGRAPHIC PROJECTION

A. Use orthographic projection (a multi view system using as many dimensioned views as necessary) to show an object's features. In general, detail objects using more than one view.

B. Use perspective and isometric views to clarify complicated details.

2.8 VIEWS

A. Before starting a drawing, study the bridge or component and determine the views and sections required to describe it fully and to the best advantage. Plan the layout and detail accordingly, allowing sufficient space for dimensions and notes.

B. Generally, all details throughout the bridge plans shall be oriented consistently. Show layouts with stationing increasing from left to right. Detail End Bent 1 looking back station; detail all other substructure elements looking ahead station. Detail superstructure sections looking ahead station.

C. Cross-reference all sections or notes on a drawing.

D. Use a planned system to arrange details on a sheet. Do not randomly place views and sections on the drawing. Avoid crowding details on a sheet. In general, it is preferred to layout a sheet with the plan view in the upper left, elevation aligned with plan view in lower left, side/section views on upper right and quantity block and notes on lower right.

2.9 SCALES

Select a scale large enough to clearly show required details when printed to 11-inch x 17-inch size with a minimum of 5/8-inch left and right margins. Do not indicate scale on the drawings. Set the annotation scale and active scale through the Bar Menu and save settings to ensure consistency. The following scales are recommended:

A. Plan and Elevation: Depending on the size of the bridge and/or how congested the sheet will be, use 1"= 10' through 1"= 50'.

B. Foundation Layout: 1"= 10' or to fit the sheet (longitudinal and lateral scales may be different and piling may be exaggerated in size for clarity).

C. Substructures

1. Plan and Elevation views 3/8"= 1'-0"
2. Sections and Details 3/4"= 1'-0" or larger

D. Superstructure

1. Plan View 1/4"= 1'-0"
2. Cross Sections 3/8"= 1'-0"
3. Details 3/4"= 1'-0" or larger.

2.10 STRENGTH AND CONTRAST OF LINES

A. Contrast between various line weights should be in the width of the line and not in the intensity. The exception is topo file and other reference files which should be screened back to reduce intensity. Verify that all lines are legible on prints.

B. Vary the line weight to accentuate important features. Use consistent line weights for similar purposes. (See the ***CPCH***.)

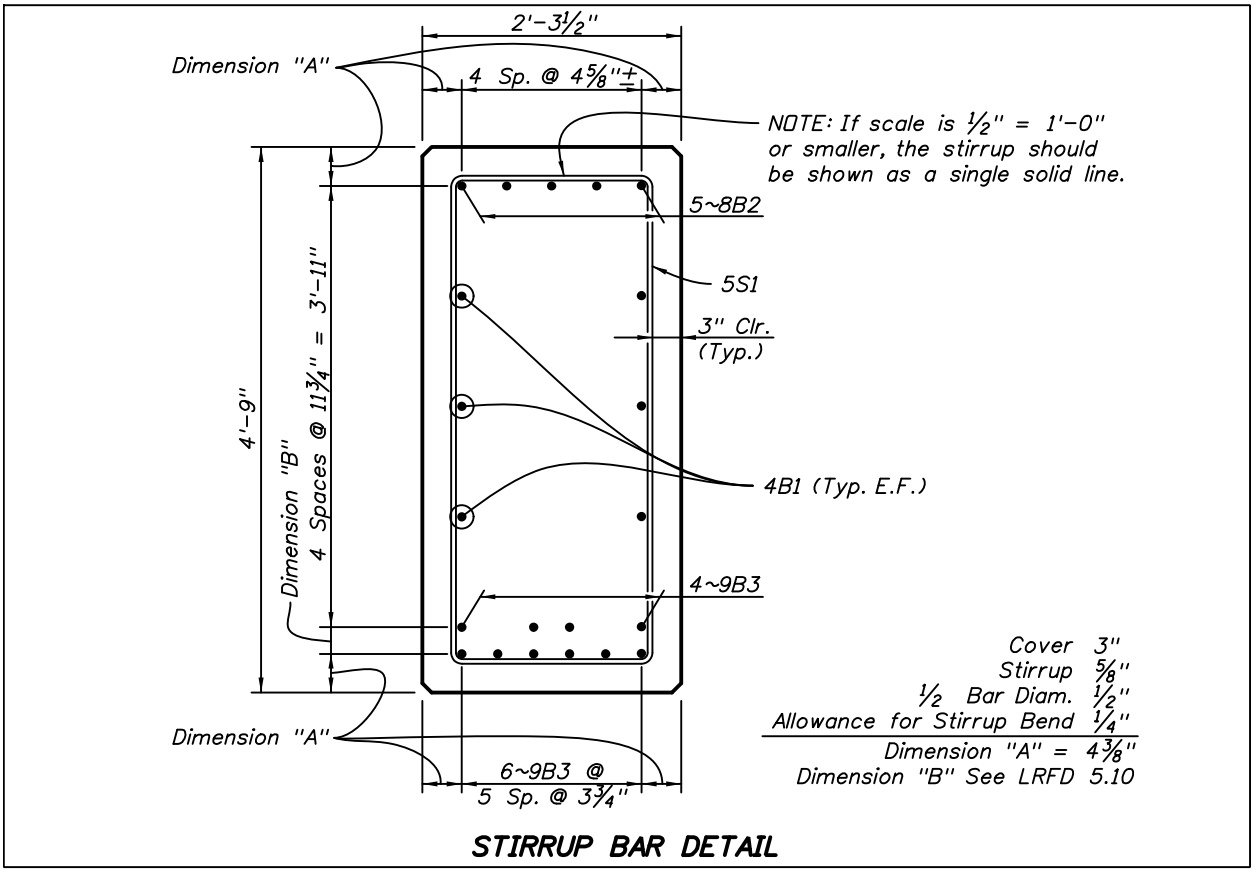
2.11 DIMENSIONING

2.11.1 General

A. A dimension is a linear measurement used to describe an object's size.

B. A value is a quantity used to express a magnitude. An integer used to quantify a number of items such as bars, spacing, bolts, holes, etc. (e.g., 10 spaces @ 4", 10 ~ 4A1). See Figure 2.11.1-1.

Figure 2.11.1-1 Stirrup Bar Detail



C. A unit is a precise quantity in terms of a reference for measurement.

D. Linear dimension: Use a value in conjunction with a unit of measurement (e.g., 5'-6¼").

E. Elevation: The unit for elevations is feet (ft). The unit is understood and should not be shown. Show most elevations to three decimal places (e.g., 25.384). See section 2.11.3 for details.

F. Angle: Show angles to needed accuracy up to the nearest second.

G. Sizes of Structural Steel and Aluminum members: When showing size, show all applicable units. Use industry standards such as ASTM or show the manufacturer's size when applicable. Examples of how to designate structural steel and aluminum members are as follows:

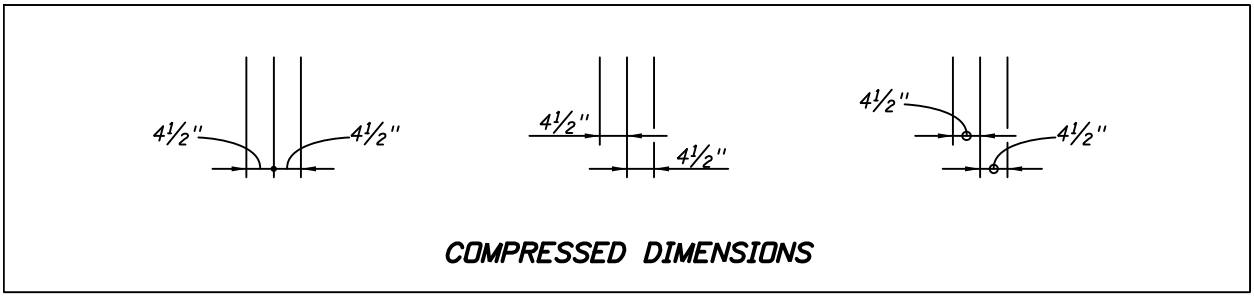
1. For solid shapes such as plates and bars - Thickness" x Width" x Length"
2. For W-Shapes - W 30 x 90
3. For Channels - C 10 x 15.3
4. For Angles - L 4 x 4 x ¼
5. For Structural Tees - WT 16.5 x 59

2.11.2 Dimensions and Text

A. Dimensions are displayed by associating Values and Units. Show dimensions clearly, accurately and tied to a control line. Not all dimensions shown on a drawing are for construction purposes; many are engineering dimensions given for convenient reference and checking.

B. Compressed dimensions, due to limited space, may be shown without sacrificing legibility. See Figure 2.11.2-1 for examples.

Figure 2.11.2-1 Compressed Dimensions



C. Dimension lines should be spaced about 3/4-inch from the object when plotted.

D. Parallel dimension lines should be spaced 3/8-inch minimum when plotted.

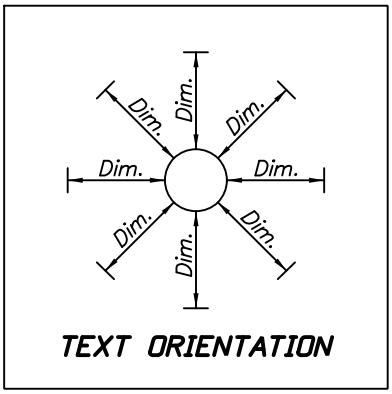
E. Dimensions should be kept outside the views (between Extension Lines), but occasionally may be placed inside views or at the end of a leader line.

F. Show dimensions in units of feet and inches. Show dimensions of 12-inches or more in feet, inches and fractions of an inch. Show dimension greater than 1-inch but less than 12-inches in inches and fractions of an inch. Show dimensions less than one inch in fractions of an inch. Some exceptions to this rule are component or member designations (i.e., 24" Square Piling, Existing 36" Steel Beam, etc.) and elevations.

G. Dimensions are to be read from the following directions (see Figure 2.11.2-2):

1. Place numerals on horizontal dimension lines so that they can be read from the bottom of the drawing.
2. Place numerals on vertical dimension lines so that they can be read from the right side of the drawing.
3. Place numerals on inclined dimension lines so that they can be read horizontally by rotating the sheet through the smallest possible angle.

Figure 2.11.2-2
Text Orientation



H. Show all dimension numerals parallel to the dimension line.

I. When dimension numerals occupy more space than provided by the dimension line, show on extension lines or by leader lines to the dimension line.

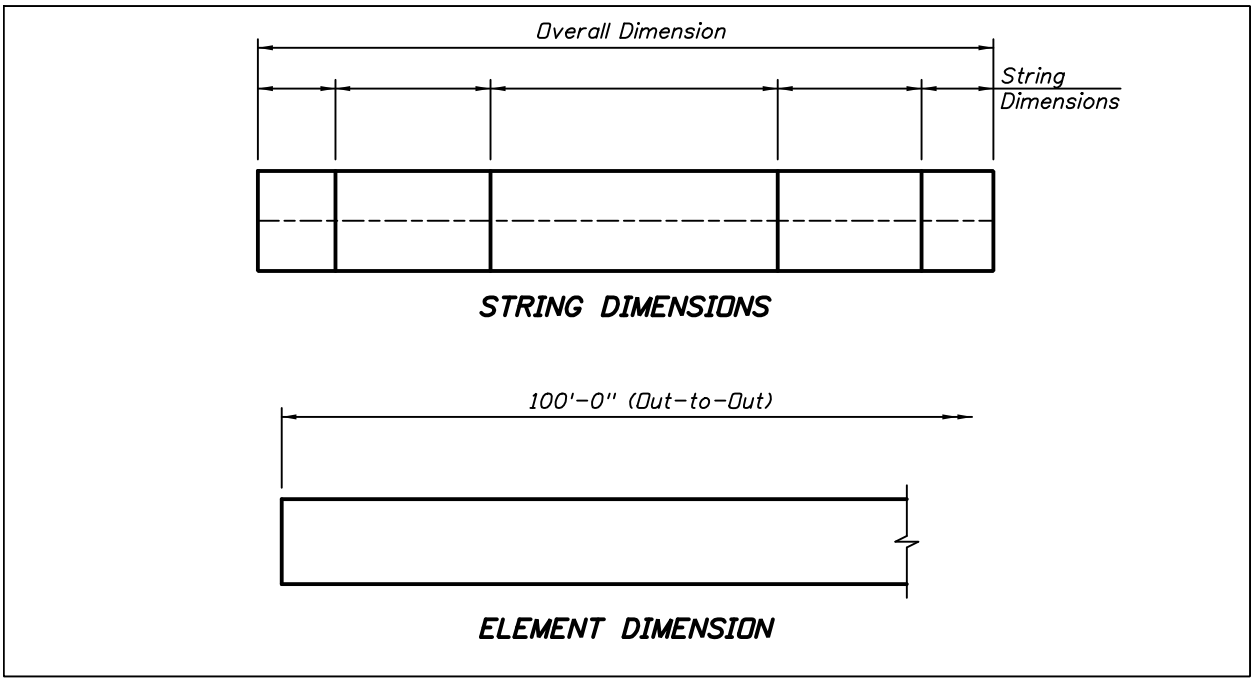
J. Ensure the sum of string dimensions equals the total overall dimension. If this presents a mathematical impossibility without violating the accuracy standards in Section 2.11.3 of this Chapter, use the (+) or (-) signs to indicate heavy or light dimensions. When dimensioning a series of spaces, show incremental dimensions. Avoid the use of "Equal Spaces".

K. When it is necessary to include a dimension between certain points in a detail, small filled circles may be used to emphasize the extremities of the line being measured.

L. Terminate dimension lines with arrowheads. Arrowheads should be a uniform size.

M. Double arrowheads on a dimension line are used on partial views, in congested areas, or when it is not necessary to show the dimension line to its termination. Note dimension numerals on the line along with a description of the magnitude or boundaries in parenthesis. Double arrowheads should not be terminated at extension lines. See Figure 2.11.2-3.

Figure 2.11.2-3 Dimensions



N. Mark centerlines with the centerline symbol. Do not use a centerline as a dimension line, though it may serve as an extension line.

O. Show leader lines with straight lines or continuous curves. Leader lines may cross extension and object lines but may not cross dimension lines.

P. Extend extension lines beyond the point of the arrowhead and leave a gap from the object.

Q. Label radii, surface finishes, and angles as required. Show angles and bearings without hyphens. See Figure 2.11.2-4 and Figure 2.11.2-5.

Figure 2.11.2-4 Angles

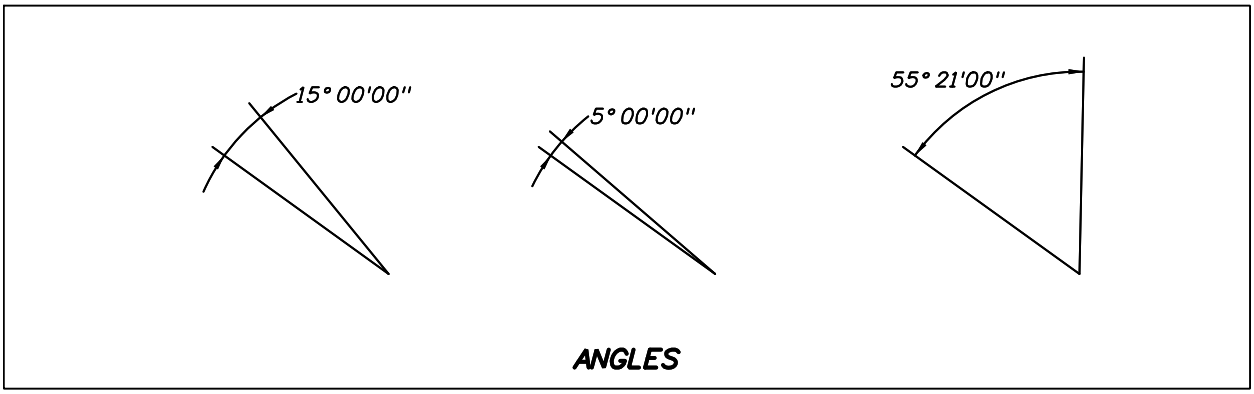
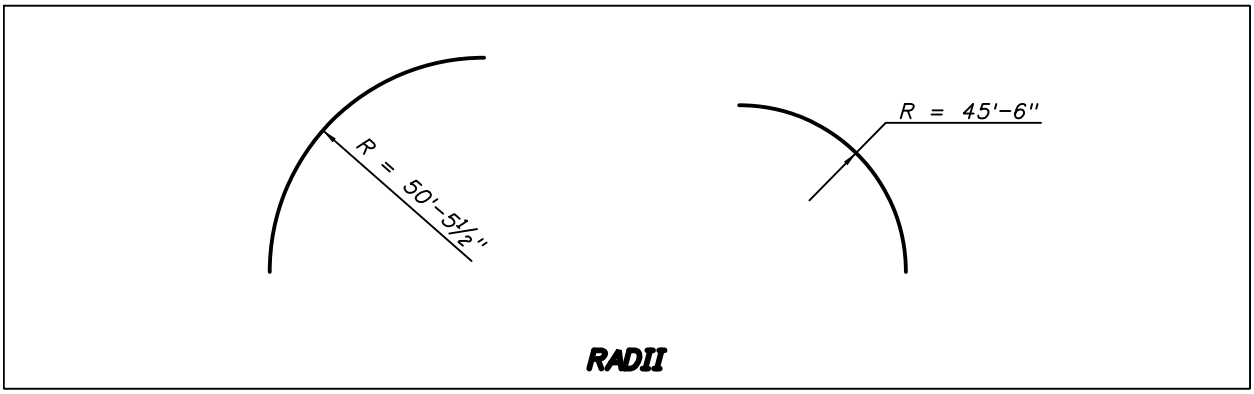


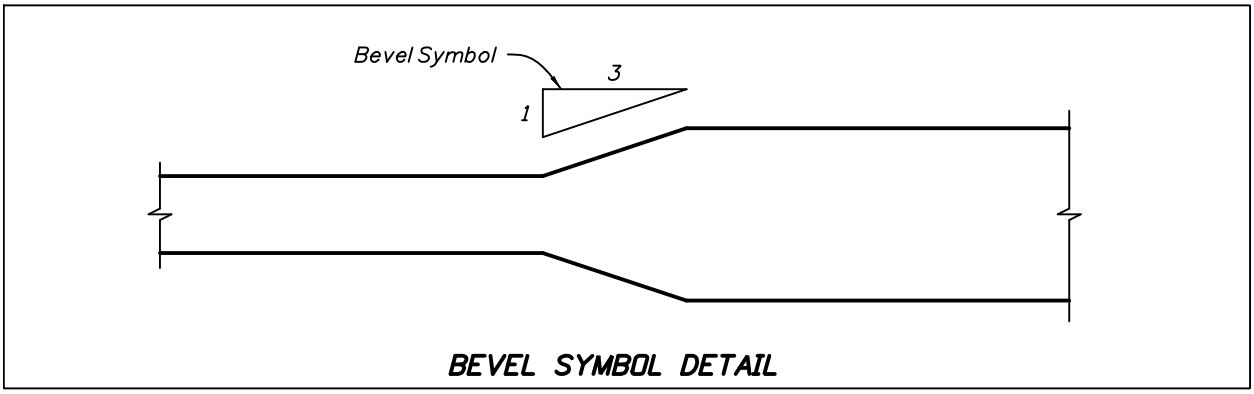
Figure 2.11.2-5 Radii



R. For non-level surfaces with extremities not specifically defined by vertical dimensions, note to slope "down" a specific vertical dimension over a defined horizontal length or at a uniform rate.

S. For non-plumb surfaces with extremities not specifically defined by horizontal and vertical dimensions, note to bevel at a uniform rate with the bevel symbol. See Figure 2.11.2-6. For the batter of non-plumb piling, note to batter with the bevel symbol or the amount of batter noted and connected to piling by leader lines with the direction of pile batter clearly shown on the drawings.

Figure 2.11.2-6 Bevel Symbol Detail



T. When dimensions are shown by methods other than described above, the unit should be provided. In this event, dimensions are defined as text (i.e., titles, sub-titles, headings, labels, notes, and free standing texts). For free standing texts, the unit may be spelled out to add clarity. If the dimensional text used to describe the size of an object is placed at the end of a leader line pointing directly to the object, show the units.

2.11.3 Dimensioning Precision

A. Show dimensions in feet, inches and fraction of an inch and elevations in decimal of a foot.

B. Dimension concrete to the nearest 1/8-inch.

C. Dimension structural steel to the nearest 1/16-inch.

D. Dimension partial lengths of reinforcing steel to nearest 1/4-inch; dimension overall lengths to the nearest inch.

E. Show stations and offsets to the nearest 0.01 foot.

F. Show layout dimensions (dimensions along tangents, etc.) to the nearest 1/16-inch.

G. Show foundation layout dimensions to the nearest 1/8-inch or 0.01 foot stationing.

H. Show dead load and live load deflections to the nearest 1/16-inch.

I. Show elevations to the nearest 0.001 foot, except pile cut off elevations to the nearest 0.1-foot and water elevations and groundline elevations to nearest 0.01 foot.

J. Show skew angles and bearings to the nearest second.
Example: 69° 38' 32", N 69° 38' 32" E

K. Show other angles given such that dependent dimensions meet the above criteria to the nearest second.

L. Show spacing of reinforcing steel to the nearest 1/4-inch.

M. Show manufactured items to industry standards.

2.12 SYMBOLS AND PATTERNS

A. To simplify the construction and clarity of details, patterns may be used to represent certain materials.

B. Use only enough material indication to clarify details.

C. Verify legibility when the drawings are reproduced to 11-inch x 17-inch print size.

D. Common symbols and patterns are included in the FDOT Structures Cell Library.

E. Use the symbologies (layers, linestyles, and line weights) appropriate to each element of the drawing, based on the levels provided in the Structures CADD seed file provided by the ECSO with the FDOT CADD software, and the requirements of CPCH Chapter 18.

2.13 ARCHITECTURAL TREATMENT

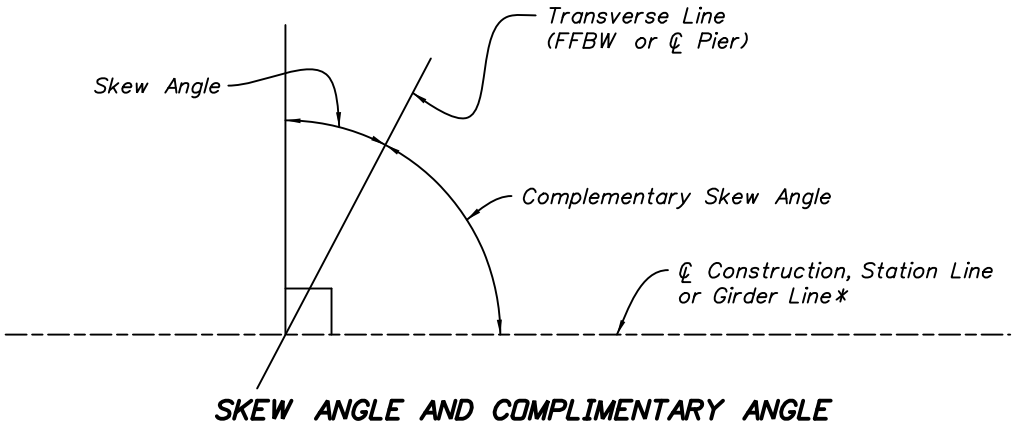
A. Do not use architectural treatments such as shades and shadows on bridge drawings.

B. Keep required pictorial views with shades and shadows separate from the bridge details.

2.14 SKEW ANGLE AND COMPLEMENTARY SKEW ANGLE

A. A skew angle is the acute angle measured between a line perpendicular to the longitudinal line and the skew line itself. See Figure 2.14-1.

Figure 2.14-1 Skew Angle and Complimentary Angle



SKEW ANGLE AND COMPLIMENTARY ANGLE

* For bridges with horizontally curved alignments, measure the skew from a line tangent to the curve

B. The skew angle referred to by computer geometry programs may actually be complementary to the plan skew angle. The sum of the skew angle and the complementary skew angle is 90 degrees.

2.15 SECTION CUT LINE AND IDENTIFICATION

A. A section cut line is an imaginary line extending between right angles at the location of the section. Use section arrows to indicate the direction of the section view.

B. Place the identification letters of the section on the interior side of the cut line. For sections located on another sheet, provide cross-reference notes on both sheets.

2.16 ENLARGED DETAILS

When an enlarged detail of a certain area in a view is required, place a circle or an ellipse large enough to encompass the area that is to be shown in the enlarged detail. Annotate the circle/ellipse with a leader line and a label such as: See Detail "A", Sheet _. Entitle the enlarged detail: DETAIL "A". If the Detail is located on another sheet, provide cross-reference notes on both sheets.

2.17 USING STANDARD ABBREVIATIONS

A. Do not use abbreviations when the meaning may be in doubt.

B. Avoid abbreviations in titles, subtitles, and notes.

C. For Standard Abbreviations, see ***Design Standard*** 001.

D. Use periods after all abbreviations other than those on ***Design Standard*** 001 noted above.

E. Define abbreviations in the General Notes or by placing a legend on the sheet containing the abbreviations.

2.18 ARROWS

A. North Arrow - Place North Arrows on drawings to aid in orienting the drawings to the actual site and bridge (or structure) location and orientation.

B. Direction of Stationing Arrow - Use an arrow to indicate the direction of stationing on plan views, superstructures, substructures etc., as well as orientation references of details and sections. Refer to ***SDM*** 2.8 regarding plan orientation.

C. Directional Arrow for Water Flow - Use an arrow to indicate direction of stream and/or tidal flow of water. The tidal flow arrow cell is located in the Structures Bar Menu.

D. Use the North Arrow or Direction of Stationing Arrow on all sheets requiring directional orientation. See instructions for individual sheets for more information.

3 COMPOSITION OF PLAN SET

3.1 SHEET NUMBERS FOR BRIDGES AND WALLS

A. Bridge plans are usually a component set of plans and includes walls. Projects with minor bridge work may include these features on sheets in the roadway plan set or detailed on roadway sheets. When prepared as a component set of plans, assemble the drawings as a separate plan set complete with a key sheet and all bridge and wall sheets, including the existing bridge plans. Number the sheets consecutively with the sheet numbers prefixed by the letter and letter/number combinations "B" for common sheets, "B1" for the first bridge, "B2" for the second bridge, "B3" for the third bridge, etc., "BW" sheets, if necessary, for all walls, and ending with the existing bridge plans, "BX1", "BX2", "BX3", etc., if necessary.

B. Start the sheet numbering with the Key Sheet numbered "B-1". Continue to use the "B" prefix for all sheets with common details to all the bridges. Begin the sheet numbering for the first sheet of the first bridge with "B1-1". Continue to use the "B1" prefix for all sheets with details pertaining to the first bridge ("B1-2", "B1-3", etc.). Number the second series of sheets for the next bridge, if included, "B2-1", "B2-2", "B2-3", etc., continuing to use the "B2" prefix for all sheets of the second bridge. Continue incrementing sheet prefix numbers, "B3-1", "B4-1", etc., for each additional bridge included in the plans. To further divide bridge sheets on complex bridge projects, use a Reference Drawing Number box in the lower right hand corner of the sheet (see ***SDM*** 3.5).

C. After all the bridge plans, place all the wall drawings (including cast-in-place retaining walls, proprietary wall control plans, temporary walls, and sound barriers) using a "BW" sheet prefix. To further divide wall sheets by wall number or type, use the Reference Drawing Number box in the lower right hand corner of the sheet (see ***SDM*** 3.5). At the end of the plan set, place all existing bridge sheets for each bridge in one PDF file named "B1ExistingPlans.pdf" for the first bridge (number sheets sequentially "BX1-1", "BX1-2", etc.) and "B2ExistingPlans.pdf" for the second bridge, etc.

D. Number other miscellaneous structures (signs, signals, lighting, etc) for the appropriate component set (see ***PPM*** Volume 2, Chapter 2) but place the drawing files in the Structures folder (see ***CADD Production Criteria Handbook***, ***CPCH***, Chapter 18). Use the Roadway border with initials found in the Structures Bar Menu for miscellaneous structures.

E. The preferred sheet order along with the file naming conventions and other CADD requirements are shown in the ***CPCH***, Chapter 18. All the sheets given in the Table may not be required in a given set of plans, while in others, additional sheets may be necessary. Any sheet name not listed in the table will flagged and not checked for compliance by the Quality Control checker (QC checker). The sheet order should correspond to the work sequence.

3.2 SHEET TITLES

If more than one sheet is required for a particular sheet type, add sheet numbers in the Sheet Title Block; e.g.: "General Notes (1 of 2)", "General Notes (2 of 2)".

3.3 SHEET REFERENCES FOR MULTIPLE BRIDGES AND/OR STRUCTURES

The drawings for a specific bridge may refer to other drawings with sheet numbers beginning with the same prefix letter and number or with the letter "B". For example, sheets in the B1-XX series may refer to any other sheet within the B1-XX series or the B-XX series but may not refer to sheets in the B2-XX series or B3-XX series, etc.

3.4 SHEET NUMBERS ON PROJECTS WITH ALTERNATE STRUCTURE TYPES

On projects with alternate structure types, designate each alternate for each bridge with a unique number following the "B" prefix (e.g. "B1" and "B2" for bridge 1 alternates; "B3" and "B4" for bridge 2 alternates, etc.). Since the sheet number will no longer correspond to the bridge number, cross reference each alternate with the bridge number in the List of Drawings.

3.5 REFERENCE DRAWING NUMBER BOX

When developing complex bridges or multiple wall systems, use the Reference Drawing Number box in the lower right hand corner on the plans to separate components and help in the development of cross references. Drawing letter/number combinations are assigned at the discretion of the designer and should include prefix combinations that correspond to the details on the drawing. The information in the Reference Drawing Number box is only used for cross referencing and plan preparation; no data from the box will be used in the Electronic Delivery process. For drawing letter/number combination suggestions, see the ***CPCH***, Chapter 18. Optional Reference Drawing Number box is required on all bascule bridges or bridges with multiple wall types. For projects with alternate designs, follow the Reference Drawing Number with the alternate designation (e.g. Drawing No. 45, Alt. B).

3.6 PHASE SUBMITTAL REQUIREMENTS

For a summary of phase submittals, see ***PPM***, Volume 1, Chapter 26.

3.7 USE OF FDOT DESIGN STANDARDS

A. The current FDOT Design Standards comprise the best practices of the FDOT in design code compliance, pay item consistency, and Specification coordination. See ***Structures Manual***, Volume 3, ***Instructions for Structures Related Design Standards*** for additional information. Do not include a list of applicable standards in the Index of Sheets.

B. In structures and wall plans, reference the applicable FDOT Design Standards by general description and index number. Place the reference on the primary drawings depicting the component. In many instances, several plan references are appropriate (e.g. beam index number references on framing plan and cross section sheets.) Provide at least one index number for each Design Standard used. Note the governing Design Standards and Design Standards Modifications on the lead project Key Sheet (see ***PPM***, Volume 2, Section 3.8). Do not include a list of Design Standards on the Index of Sheets.

C. Some Design Standards for structural components, e.g. prestressed beams, approach slabs, bearing pads, etc., require supplemental tables, notes and or graphics to be completed and included in the plans by the designer. Select the appropriate tables, notes and or graphics using the FDOT CADD software. For the latest version of the FDOT CADD software, go to: http://www.dot.state.fl.us/ecso/downloads/software/. For the latest version of the Structures Design Standards Data Table cell library (V8semi-standards.cel), go to: http://www.dot.state.fl.us/structures/CADD/standards/CurrentStandards/MicrostationDrawings.shtm

D. For the Design Standards, see the Roadway Design Office's web site at: http://www.dot.state.fl.us/rddesign/DesignStandards/Standards.shtm.

Commentary: Since all former Structures Standard Drawings are now contained in the Design Standards, specific references are necessary to clarify the Designer's intent to the Contractor.

3.8 EXISTING PLANS

Existing structures plans may be incorporated into new Contract Plans. When available, existing plans may be obtained from the District. Plans must be legible and reproducible. See ***CPCH*** Chapter 18 for Existing Plans file naming and formats required for Electronic Delivery. Add plan note indicating that existing plans are not available when applicable.

4 CONCRETE COMPONENTS

4.1 GENERAL

A. Concrete components for bridges are custom constructed either in place at the bridge site or at a precast facility and require clear, complete and fully detailed plans.

B. The concrete outlines, reinforcing steel, prestressing strands and/or post-tensioning tendons must be easily distinguishable. This can be accomplished by using the appropriate preset levels in the MicroStation Structures seed file provided by the ECSO with the FDOT software package and/or following the symbology guidelines provided in Chapter 18 of the ***CPCH***.

C. When detailing concrete components, show plan and elevation views along with sections and any details necessary for construction.

4.2 ITEMS EMBEDDED IN CONCRETE COMPONENTS

Show the vertical and horizontal locations of reinforcing steel, prestressing strands and/or post-tensioning tendons. Normally, the spacing, location and limits of reinforcing steel can be adequately shown with a few representative bars which are clearly labeled and/or dimensioned. It is important to ensure that all reinforcing steel is clearly identified without over complicating the drawing.

4.3 REINFORCING STEEL

A. Detail reinforcing bars in plan, elevation and sections to clearly indicate the size, location and spacing of individual bars. Show the number of reinforcing bars in plan or elevation views.

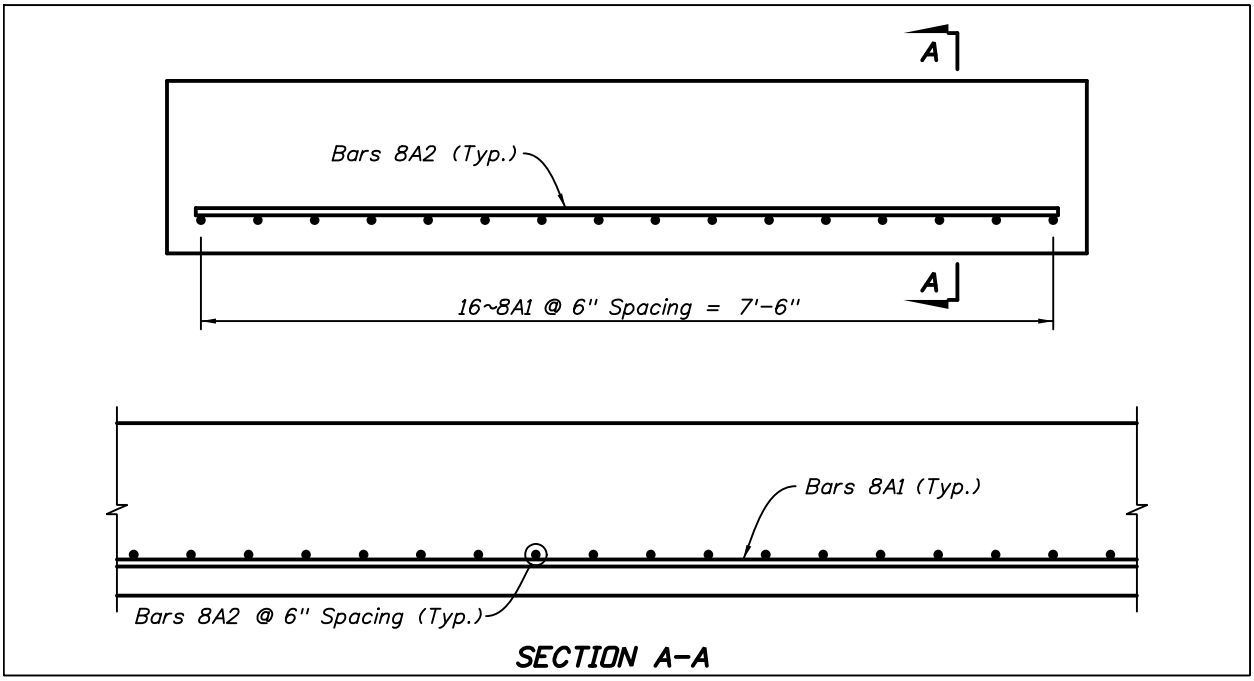
B. Usually, in plan or elevation views, only the first bar and the last bar of a series of bars need be drawn, and the number and spacing indicated between. Show all bars in section views.

C. Show the number of bars, followed by a tilde, the bar size, the bar mark and the spacing. For example, 12 ~ 8 A1 @ 6" means 12 bars, Size #8, Mark A1 at 6 inch spacing. The symbol "@" is optional for the word "at".

D. #4 bars are the smallest reinforcing steel size used in cast-in-place components for bridges.

E. Each bar mark (e.g. "F1") is to be unique to a single component. Where bars are shown in multiple views, bill bars in the view in which they are first encountered and only reference for clarity in other views. See Figure 4.3-1.

Figure 4.3-1 Reinforcing Bar Callout Detail



4.3.1 Maximum Bar Spacing

A. For maximum bar spacing for shrinkage and temperature, see ***LRFD***, Section 5.10.8.

B. For horizontal reinforcing steel in walls, the distance from the top of footing to the first bar in the stem is a maximum of one half the spacing of the bars immediately above it.

C. Bar spaces, plus cover to centerline of bars must equal the concrete dimension of the member. Use the following procedure to detail multiple bars equally spaced where the number of spaces times the nominal spacing does not exactly equal the overall concrete.

14 Bars @ 5½"± = 6'-0" or 13 sp @ 5½"± = 6'-0"

This means 13 equal spaces. The symbol “@” means “at”, and the symbol “±” means “approximately.”

4.3.2 Minimum Bar Spacing

A. For minimum bar spacing, see ***LRFD***, Section 5.

B. When multiple bars are lapped at the same location, the required minimum spacing measured between laps must be equal that for parallel bars.

C. Avoid using bundled bars. If bundled bars are required, they must meet ***LRFD*** requirements.

4.3.3 Minimum Concrete Cover

See ***SDG*** for minimum concrete cover requirements.

4.3.4 Fit and Clearance

A. Check reinforcing fit and clearance by calculations and with large scale drawings. Skews tend to aggravate problems of reinforcing fit. Consider tolerances normally allowed for cutting, bending and locating reinforcing. Refer to ***CRSI Manual of Standard Practice*** for industry fabrication tolerances.

B. Some common areas of interference are:

1. Between slab reinforcing and supporting element reinforcing, such as girder stirrups and monolithic end bent or intermediate bent.
2. Vertical column bars projecting through pier cap reinforcing.
3. Areas near expansion devices.
4. Anchor bolt blockouts for girders.
5. At anchorages for post-tensioning systems.
6. Between prestressing (pretensioned or post-tensioned) steel and reinforcing steel stirrups, ties, etc.
7. Between column bars to be lapped with footing dowels.
8. Drilled shaft steel projecting through footing steel.
9. Bars with large radii spaced close together or where fabrication tolerances exceed placement tolerances.
10. Bars greater than size #11 where fabrication tolerances are increased.

4.3.5 Bar Splicing

A. Detail splices for main reinforcement bars of different sizes. Other bars may be shown as "continuous" without showing splice locations because splices are detailed on the Reinforcing Bar List. Indicate splice locations as required (i.e., phase construction, construction joints, etc.). Detail locations and splice lengths for main reinforcing. Use mechanical splices or other positive connections for bars larger than #11.

B. For tension splices, the smaller bar governs the length of a lap splice between bars of different sizes.

C. For compression splices, the larger of the splice length of the smaller bar or the development length of the larger bar, governs.

D. Wherever practical, stagger main reinforcing bars so that only one-third are spliced at the same location. Exceptions include:

1. Phased construction.
2. Flat slab construction.
3. Compression zones.
4. Bases of stems of cantilevered retaining walls.

4.3.6 Dowels

Show minimum embedment length on the plans. Use standard hook bends when bent bars are used and depth of embedment permits. Show bent bars used for footing dowels resting on the bottom reinforcing steel mat in the footing. Verify that the minimum embedded length does not violate minimum cover requirements.

4.3.7 Bars in Section

A. Draw sections at a scale adequate to clearly show reinforcing details.

B. For stirrups and other bars not shown end-on, represent bars with single, unbroken lines at scales less than 1/2"=1'-0" and double, unbroken lines at 1/2" scale or larger.

C. Draw tie and stirrup hooks to scale. Dimensions are not necessary, unless it is a non-standard bar bend.

D. Use small circles to represent bars shown end-on. Circles may be left open or shown solid (filled). Use the chosen symbol consistently throughout the drawings. Show bars as filled circles when holes are also shown.

E. Identify bars shown end-on by leaders with circles or arrowheads pointing to the bar.

F. For complex reinforcing patterns, cut sections at specific locations along a member rather than showing a typical section.

G. Show corner bars enclosed by stirrups or ties at the corner of the bend.

4.3.8 Hook Bars

When the required concrete cover cannot be maintained with normal orientation of the hook, add the following note to the plans: "Rotate bar as necessary to maintain required cover."

4.3.9 Maximum Reinforcing Bar Lengths

#4 Bars and larger: 60 feet.

4.3.10 Reinforcing Bar Lists

A. Refer to ***Design Standard*** Index 21300, Standard Bar Bending Details and ***Structures Manual***, Volume 3, ***Instructions for Structures Related Design Standards***.

B. Using FDOT "Rebar Program", provided with the FDOT Structures Bar Menu, generate a reinforcing bar list for each structure. Provide a labeled tabulation for every reinforced component, (i.e., bents, piers, deck, approach slabs, etc.) Each bar designation must be unique for a given component but may be repeated for separate components. Designate bars "A1", "A2", "B1" etc. Show a separate section in the reinforcing bar list for each component and construction phase on a project.

Prestressed beams, piles, concrete sheet piles and traffic railings that are shown in the Design Standards do not need to be included on the reinforcing bar list.

C. Dimension all bars "out-to-out". Round the overall length of each individual bar to the next inch. Show all straight bar dimensions rounded to the nearest inch.

D. Separate reinforcing for sub-components into a logical sequence similar to the order in which they will be constructed. Identical components should be grouped together. The following list should be used as a guide of the minimum breakdown of sub-components.

Substructure

End Bent
Footing
Column/Pier
Bent Cap

Superstructure

End Diaphragms
Intermediate Diaphragms
Deck

Approach Slabs

Walls

Footing
Wall/Cap
Deadman Anchor

4.3.11 Preferred Maximum Reinforcing Steel Bar Sizes

A. Use the following maximum reinforcing steel sizes for design and detailing.

B. Use the smallest practical bar size in order to minimize stress concentrations, increase bonding strength, decrease corrosion potential, and comply with ***LRFD*** crack control criteria.

Table 4.3.11-1 Preferred Maximum Reinforcing Steel Bar Sizes

Bridge Component	Maximum Bar Size	Comments
Drilled Shafts	#18	For #14 and #18 bars, no bundling or bar bends
Footings	#11	
Pier Columns	#11 Main reinforcing #6 Ties	
Pier/Bent Caps	#11 Main reinforcing #6 Stirrups	
Webs of Segmental Boxes	#7	

4.4 CONCRETE SURFACE FINISHES

A. See Figure 4.4-1 and Figure 4.4-2 for depiction of areas that are to receive a "Class 5 Applied Finish Coating". This is a concrete texture coating utilized as a final concrete finish. Show appropriate notes in the General Notes and the corresponding sketches on the General Notes drawing. Conventional cast-in-place retaining walls will generally require a "Class 5 Applied Finish Coating". Unless project specific aesthetic goals require its use, in most cases precast portions of proprietary retaining walls do not require a Class 5 Applied Finish Coating.

B. When existing bridges are widened, specify a "Class 5 Applied Finish Coating" on the existing substructures along with the widened portions of the structure where applicable.

Figure 4.4-1 Class 5 Finish at Grade Separation Structures

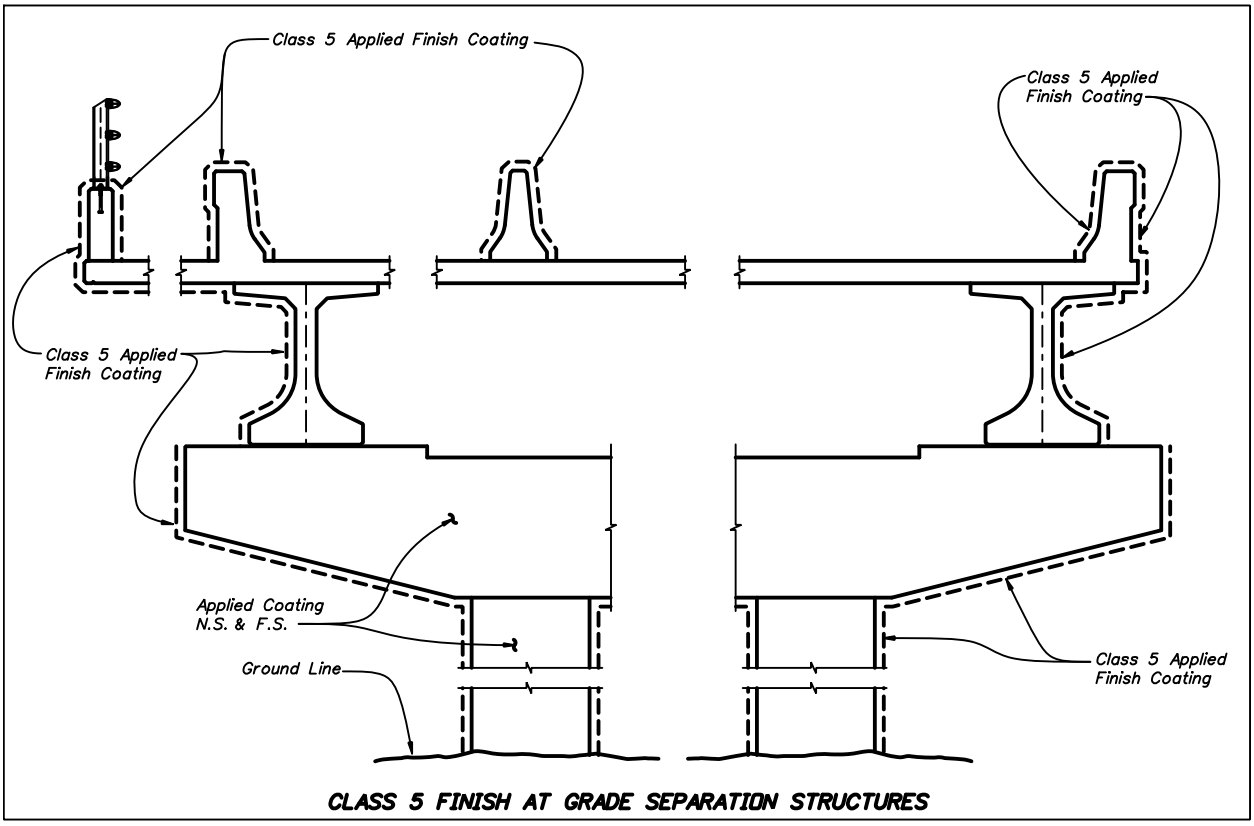
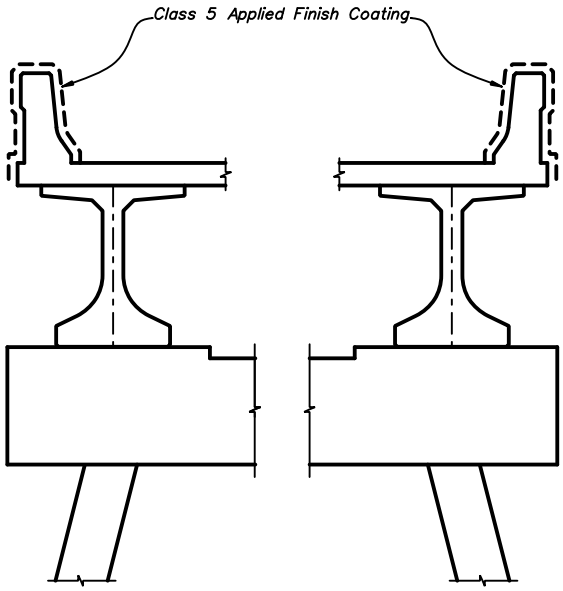


Figure 4.4-2 Class 5 Finish at Waterway Crossings and R.R. Separation Structures



CLASS 5 FINISH AT WATERWAY CROSSINGS AND R.R. SEPARATION STRUCTURES

5 GENERAL NOTES AND BID ITEM NOTES

5.1 GENERAL NOTES

A. Prepare a complete set of General Notes for each project.

B. As the first item under General Notes, list the version of the Structures Manual and any subsequent Structures Design Bulletins used as the basis for the design of the plans.

C. On projects that require two different construction methods (e.g., prestressed concrete beams and steel girders), show separate General Notes for each method of construction and identify the method to which they apply.

D. Organize notes under headings for Concrete Notes, Steel Notes, etc.

E. Include all General Notes and Pay Item Notes on the General Notes sheet. Notes for a specific element may be shown on the first sheet showing that element. The Index of Sheets is generally a separate sheet but may be included on the General Notes sheet if space permits. Do not include a list of Design Standards in the Index of Sheets.

F. Do not use General Notes or any other plan notes to repeat or modify requirements stated in the Specifications. If project specific modifications to the Specifications are required, prepare either a Modified Special Provision or a Technical Special Provision. Contact the District Specifications Office for guidance.

G. Never include proprietary information in a General Note or any other plan notes unless system compatibility is an issue and a signed Public Interest Finding (per FHWA requirements) is on file in the District Specifications Office. Do not specify a product or manufacturer name as this may give preferential treatment to the listed items. Do not use the term "or equal". Use performance criteria. Contact the District Specifications Office for guidance.

5.2 TYPICAL GENERAL NOTES

The following is a sample of typical notes to be included on the General Notes sheet. Place these notes on the General Notes sheet and modify for project-specific requirements:

A. Design Specifications

1. FDOT Structures Manual dated [January or July] 20XX and subsequent Structures Design Bulletins C[XX-XX], C[XX-XX] and C[XX-XX].
2. American Association of State Highway and Transportation Officials (AASHTO) Load and Resistance Factor (LRFD) Bridge Design Specifications, [4th] Edition and all subsequent interims.
3. FDOT Plans Preparation Manual dated January, 20XX.

B. Construction Specification

FDOT Standard Specifications for Road and Bridge Construction, 20XX Edition and supplements thereto.

C. Vertical Datum

[Indicate which vertical datum is used on the project - NAVD 88 or NGVD 29].

D. Environment

Bridge Number	Superstructure	Substructure	
		Concrete	Steel
700XXX	Slightly	[Slightly, Mod., Extrem.]	[Slightly, Mod., Extrem.]
407XXX	Moderately	[Slightly, Mod., Extrem.]	[Slightly, Mod., Extrem.]
121XXX	Extremely	[Slightly, Mod., Extrem.]	[Slightly, Mod., Extrem.]

E. Design Methodology

Load Resistance and Factor Design (LRFD) method using strength, service [extreme event] and fatigue limit states.

F. Design Loadings

1. Live Loads: HL-93 with Impact
2. Dead Loads: Traffic Railing 420 plf
   Median Traffic Railing 486 plf
   Stay-In-Place Forms 20 psf
   Reinforced Concrete 150 pcf
   Future Wearing Surface 15 psf
   [OR Design does not include an allowance of 15 psf for Future Wearing Surface.]
3. Vehicle Collision Load: New Pier Columns have been designed to withstand the 400 kip impact load.
4. Utilities: No allowance for utility loads has been included in the design. [If allowance for utility loads has been included, indicate the magnitude and location of the loads used in the design.]

G. Materials

1. Concrete:

Concrete Class	Min. 28-day Compressive Strength (psi)	Location of Concrete in Structure
II	3400	Traffic Railing
II (Bridge Deck)	4500	Bridge Deck
IV	5500	C.I.P. Substructure
V (Special)	6000	Prestressed Concrete Piles
VI	8500	Prestressed Concrete Beams

2. Concrete Cover: [Depends on Environmental Classification]

Cast-In-Place Superstructure (Top of Deck)	2½"
Cast-In-Place Superstructure (Except Top of Deck)	2"
Precast Prestressed Beams (Except Top Surface)	2"
Top Surface of Beam Top Flange	1"
Cast-In-Place Substructure (Cast Against Earth)	4"
Cast-In-Place Substructure (Formed Surfaces)	3"
Cast-In-Place Substructure (Top of Beam Pedestals)	2"

Concrete cover dimensions shown in the plans do not include placement and fabrication tolerances unless shown as "minimum cover". See Specification 415 for allowable tolerances. All dimensions pertaining to the location of reinforcing steel are to centerline of bar except where clear dimension is noted to face of concrete.

3. Reinforcing Steel: Provide ASTM A615 Grade 60 conventional reinforcing steel in accordance with Specification 415.

H. Applied Finish Coating

A Class 5 Finish Coating shall be applied to the portions of the structures shown on the Surface Finish Detail sheet(s).

I. Plan Dimensions

All dimensions in these plans are measured in feet either horizontally or vertically unless otherwise noted.

J. Utilities

For plan locations of existing utilities, see Plan and Elevation sheet(s). Locations of utilities, including under deck lighting, shown in the plans are approximate. For disposition of utilities, see the Utility Adjustment sheet(s) in the Roadway plans.

K. Bridge Name

Place the following bridge name on the traffic railing in accordance with the Traffic Railing Design Standards:

[Use the name of the bridge or non-roadway facility being crossed or include the name of both facilities for roadway crossings. e.g.:

THOMASVILLE ROAD FLYOVER
TOMOKA RIVER
CSX RAILROAD
US 19 OVER EAST BAY DR

For multiple bridges, identify the associated bridge number, e.g.

Bridge No.	Name
600103	CHOCTAWHATCHEE BAY
600104	CHOCTAWHATCHEE BAY RELIEF]

L. Screeding Deck Slabs

Screed the riding surface of the Bridge Deck and Approach slabs to achieve the Finish Grade Elevations shown in the plans. Account for theoretical deflections due to self weight, deck casting sequence, deck forming systems, construction loads, overlays and temporary shoring, etc. as required.

M. Stay-In-Place Deck Forms

Design includes allowance for 20psf over the projected plan area of the metal forms for the unit weight of the metal forms and the concrete required to fill the form flutes. Stay-in-place forms are not allowed at deck cantilevers. Detail stay-in-place forms to clear top lateral bracing of box girders.

OR: Stay in place deck forms will not be permitted on this project.

N. Joints In Concrete

Construction joints will be permitted only at the locations indicated in the plans. Additional construction joints or alterations to those shown shall require approval of the Engineer.

O. Existing Bridge Construction Considerations

1. Dimension Verification: Unless otherwise noted, the dimensions, elevations and intersecting angles shown are based on the information as detailed in the Original Construction Plans of the existing bridges and may not represent as-built conditions. It is the Contractor's responsibility to verify this data before beginning construction and notify the Engineer of any discrepancies.
2. Existing Reinforcing Steel [Widenings]: All superstructure deck transverse reinforcing steel, both top and bottom layers, and end bent reinforcing steel, shall be protected, salvaged and utilized in the new structure. Cutting of this reinforcing steel and substitution of epoxy bonded dowels is not permitted as a construction option.

P. Traffic Control Plans

[Insert traffic control notes for the project.]

Q. Phasing Of Work

Work phasing and progression of the work shall conform to the Traffic Control Plans located in the Roadway Plans and the notes on the Construction Sequence Drawings.

5.3 TYPICAL STEEL GENERAL NOTES

Include the following additional general notes when structural steel is to be used in the project. Place these notes on or after the General Notes sheet and modify for project-specific requirements:

A. Structural Steel

All structural steel shall be in accordance with ASTM A709, Grade 50 or HPS 70W unless otherwise noted. Stiffeners, internal and external crossframes, lateral bracing and other ancillary items may be Grade 36 unless otherwise noted.

B. Steel Fabrication

1. All shop splices in flanges or web plates shall be made prior to welding flange plates. All splices shall be indicated on the drawings.
2. Shop assemblies are required in accordance with Specification 460. [Specify shop assembly type based on Specification 460.]
3. All ends of girders, end diaphragms and pier diaphragms shall be vertical after dead load is applied. All intermediate stiffeners, intermediate crossframes and field splices shall be normal to grade.

C. Welding

1. Welds requiring non-destructive testing shall be radiographically inspected, except where the geometry of the region of the weld will not permit satisfactory information to be secured for verification of the weld quality. When such geometrical conditions exist, other inspection procedures or combinations of procedures such as Ultrasonic Inspection, Dye Penetrant Inspection and/or Magnetic Particle Inspection, shall be used. Non-destructive testing shall be performed as required by the current edition of the AASHTO/AWS D1.5 Bridge Welding Code.
2. Field welding to any structural steel for the purpose of attaching erection hardware or for anchoring conduits/boxes for box lighting shall be formally submitted to the Engineer for approval. Shear connector installation is governed by OSHA Steel Erection Rule.
3. The following members are classified as ancillary members in accordance with the current edition of the AASHTO/AWS D1.5 Bridge Welding Code:
    a. Expansion Dams
    b. Drainage Components
    c. Sheet Piling
    d. Bearings

D. Field Connections

All field connections shall be made with [X]" diameter high strength bolts in accordance with ASTM A325 unless otherwise shown. Threads shall be excluded from the shear plane for plate thickness of ¾" or greater adjacent to the nut. Bolt heads shall be on the exterior/exposed face of the girders.

E. Painting

All structural steel shall be painted in accordance with Sections 560 and 975 of the Specifications. Paint all structural steel with a high performance top coat system. The color of the finish coat shall conform to Federal Standard No. 595B, Color No. XXXXX.

For Steel Box Girders:

Paint the inside surface of steel box girders with one shop applied coat of approved primer and a compatible finish coat pigmented to Federal Standard 595B Color No. 37925 (white). Any other color used for the interior finish coat must be pre-approved by the District Structures Maintenance Engineer. The finish coat of the interior surface is not required to be UV resistant and does not have to be the same product as the finish coat of the exterior surface of the box girder.

F. Ladders and Platforms

Structural steel ladders and platforms shall conform to ASTM A36 and shall be hot-dipped galvanized in accordance with Specification 962. Welding shall conform to AWS D1.1.

5.4 TYPICAL PAY ITEM NOTES

Include in the Pay Item Notes information required to define, show limits of quantities or otherwise offer explanation to the list of Bridge Pay Items. All pay item notes should be shown in one location; not scattered throughout the plans. Examples are:

A. For Traffic Control Pay Item Notes, see Roadway Plans.

B. Bid on only one of the alternates designed.

AA - Florida-I Beam Superstructure.

AB - Steel Girder Superstructure.

C. Pay Item No. 400-7 includes __ sq. yards of Approach Slab Grooving.

D. For limits of removal of existing structures (Item Number 110-3), see Sheet No. XX-XX. (List sheets as needed)

Do not include estimated structural steel quantities in the plans.

6 SLOPE PROTECTION

6.1 GENERAL

A. This chapter provides the bridge designer with the necessary information to develop plan details for appropriate slope protection. In most cases the standard details depicted for slope protection, with minor modifications will be suitable. In some cases, typically in tidal areas or when severe scour conditions exist, special designs and details may be required.

B. Design aids are available here for slope protection details. It is the designer's responsibility to make the appropriate project-specific modifications to these design aides before incorporating them into the plan set. Once the appropriate modifications have been made, the designer assumes the responsibility of Engineer of Record.

6.2 GRADE SEPARATIONS

A. For grade separation bridges, design slope pavement on 1:2 front slopes with provisions to extend erosion protection to a minimum of four feet outside the superstructure coping.

B. To protect railroad track embankments, use sand-cement riprap instead of slope pavement.

6.3 WATER CROSSINGS

A. The Drainage (Hydraulic) Engineer will determine the design and extent of the slope protection in accordance with the FDOT ***Drainage Manual*** and other applicable guidelines such as ***HEC-18***. The slope protection for spill-through abutments (End Bents) adjacent to water will usually be rubble riprap. Sand-cement riprap is usually limited to bridges over streams or canals with extremely low, non-erodible flow velocities under all flood conditions. A 1:2 slope is the steepest desirable slope rate.

B. Bulkhead abutments can be protected by sheet piling or precast panels with toe protection provided by rubble riprap. Rubble riprap might also be recommended above the bulkhead or at its ends. Design the protection to extend at least four feet outside the superstructure coping.

6.4 DUAL BRIDGES

Extend the slope protection in the median between dual bridges to include:

A. The entire median width for rural area bridges with a separated median width of forty feet or less.

B. The entire median width for urban area bridges with a separated median width of fifty feet or less.

C. The entire width for urban area bridges inaccessible due to physical barriers or when access is severely limited due to design features or vehicular movement that will impede the ability to maintain the facility.

7 PLAN AND ELEVATION

7.1 GENERAL

A. This drawing is the general layout of the bridge in plan and elevation views.

B. Draw Plan and Elevation views to the same vertical and horizontal scale if possible. In some cases a vertical scale larger than horizontal is required. More than one sheet may be required. If multiple sheets are used, show overlap between sheets for clarity.

C. It is preferred to have the plan and elevation shown on the same sheet.

D. For projects with multiple bridges, include a key map on the plan and elevation sheet showing the location of the subject bridge within the project. If multiple plan and elevation sheets are needed for a single bridge, the key map needs only to be on the first sheet.

E. For examples illustrating the content and format of completed Plan and Elevation sheets, see the ***Structures Detailing Manual Examples***.

7.2 PLAN AND ELEVATION DRAWING - GENERAL

In general, include the following information on the Plan and Elevation sheet(s):

A. The bridge in both plan and elevation views, using an appropriate scale. Label views "PLAN" and "ELEVATION". This is mandatory for Bridge Development Reports and subsequent submittals.

B. All vertical and horizontal geometry including:

1. Horizontal alignment. (Horizontal curve data or Bearing of tangents.)
    a. Show PC and PT stations on plan view.
    b. Use horizontal curve data table cell from the Structures Cell Library for horizontal curve information.
    c. Include superelevation transition diagram where appropriate.
    d. Show bearings of tangents on Station Line.
    e. Vertical and horizontal geometries for intersecting facilities.
2. Vertical alignment (along PGL).
    a. When the bridge is on a tangent grade, show the grade and station and elevation of the nearest points of tangency.
    b. When the bridge is on a vertical curve, use the vertical curve data cell from the Structures Cell Library. Include a reference to the horizontal control line to which it applies. See ***SDM*** Chapter 2.

    When there is not enough space on the Plan and Elevation sheet(s) to show this information, include supplemental Profile Grade and Superelevation Transition sheet(s) as required.

C. North Arrow. Place in upper right corner of sheet when possible.

D. Traffic data (for each facility, if grade separation). Include as a minimum:
   - design speed
   - present and design year (+20) AADT
   - percentage of trucks

E. Adjacent roadway Guardrail, Concrete Barrier Wall and Pier Protection Barriers in Plan and Elevation views. Reference the appropriate ***Design Standard*** Index number.

F. Bridge-mounted lighting, signs and signals and related station/offset information.

G. Distance to nearest milepost from intersection of railroads.

H. All walls (temporary critical, permanent, MSE, etc.). Show graphic depiction and indicate wall type, number and designation.

I. Show shoreline in plan and elevation views using zero contour line or at location of MHW/NHW.

J. All applicable bridge numbers. For dual bridges, show bridge number inside the bridge in the plan view of the corresponding bridge.

K. Roadway lighting, if scale permits.

7.3 PLAN AND ELEVATION DRAWING - PLAN VIEW

At a minimum, include the following in the Plan View of the Plan and Elevation sheets:

A. Baseline, Centerline of Construction and Profile Grade Line(s) (PGL). Label whole stations and include 20ft tick marks. Indicate direction of stationing and station equations as required. Include centerline of lower roadways, canals, railroads, etc.

B. Stations at the following locations:

   1. Begin and end of bridge and approach slabs.
   2. Centerlines of bents or piers.
   3. Intersections of centerlines of lower roadways.
   4. Lower roadway, stream, railroad milepost or other physical feature at the location on the structure plan along the Station Line for the structure. Indicate bearing of tangents if applicable.

C. Skew angle at bents and intersecting base lines. See ***SDM*** 2.14 for details.

D. Direction of traffic. Show one arrow per lane.

E. Roadway width, traffic and pedestrian railing widths, inside and outside shoulder widths, lane widths, median width, sidewalk width, out-to-out width, gore area dimensions, width of widening and width of removal (including removal of slope protection).

F. Critical locations and dimensions of horizontal and vertical clearances. Identify location of low member.

G. Expansion joints. Use a solid line to indicate expansion joints in plan view.

H. Boring locations and labels.

I. All utilities, existing and proposed, buried and overhead. Also indicate status of utilities, e.g. abandoned, to be relocated, etc.

J. Right-of-way lines (roadway, railroad, etc.)

K. Limits of slope pavements, sand cement riprap or rubble riprap. Indicate slope in the following format: V:H. Also indicate type of slope protection and toe of slope location.

L. Edge of shoulder.

M. Berm width.

N. Locations of deck drains that are large enough to be legible. Do not show small bridge scuppers.

O. Fender systems and navigation lights for bridges over navigable waterways. Indicate Heavy, Medium or Light Duty and applicable index number. Indicate clear channel width, centerline of channel and bearing of channel in plan view.

P. Direction of flow of waterway. Indicate if waterway is tidal.

Q. Waterline at MHW/NHW elevation.

R. Limits of environmentally sensitive areas such as wetland lines, seagrass delineations, etc. Ensure that lines shown in the plan view coincide with lines shown in permit applications.

S. Limits of existing bridge. Indicate existing bridge number. Hatch area to be removed, if any.

T. Locations of permitted work bridges or platforms. Ensure that the locations of work platforms are consistent with locations shown in any required permits. Alternatively, this information may be shown on Construction Access drawings.

U. Consider showing ship impact zones in plan view.

7.4 PLAN AND ELEVATION DRAWING - ELEVATION VIEW

Show the elevation view of the bridge as viewed along the right coping. Curved bridges may be better represented by a line cut along the centerline of roadway. At a minimum, include the following in the Elevation View on the Plan and Elevation sheets:

A. The elevation (vertical) scale on both sides of the elevation view.

B. Span lengths and overall length of bridge. Label and dimension continuous units/decks.

C. Location of expansion and fixed bearings and integral piers. Label as E, F and I, respectively. Also indicate expansion joints as EJ in elevation view.

D. Existing ground and finished ground profiles, including sections of any intersecting road, railroad, waterway or other physical feature such as buildings or drainage structures, existing or proposed.

E. Low, mean and high water elevations as appropriate.

F. Embankment and canal slopes. Indicate the slope in the following format: V:H.

G. Location and value of minimum vertical clearance.

H. Location of where ground line is taken, i.e. ground line at centerline of construction or groundline at right edge of coping.

I. Roadway widths and clear distances to piers, space permitting.

J. Fenders. Also show clear channel width.

K. Foundation types. Indicate pile or shaft size as applicable.

8 BRIDGE HYDRAULIC RECOMMENDATION SHEET

8.1 PURPOSE

A. This drawing shows all pertinent hydraulic information necessary for the layout of a bridge at the location of a given water crossing.

B. This drawing, prepared by the District Drainage Engineer or a Consultant Drainage Engineer, should be included in the PD&E documents and/or must be in the 30% Plans submittal. This drawing must be included in the final bridge plans.

C. For a typical drawing, see ***PPM*** ***Exhibit BHD-1***.

8.2 GENERAL REQUIREMENTS AND DESIGN PROCEDURES

For General Requirements and Design Procedures involving the Bridge Hydraulic Recommendation Sheet, permits and other hydraulic considerations and requirements, see Chapter 27 of the ***PPM*** and the FDOT ***Drainage Manual***.

9 CONSTRUCTION SEQUENCE FOR BRIDGE WIDENING AND PHASED CONSTRUCTION

9.1 GENERAL

A. The purpose of the Construction Sequence sheets is to show the proposed sequence of bridge construction as well as the maintenance of traffic during bridge widening and/or phased bridge construction.

B. Construction sequence sheets are a supplement to the Traffic Control Plan (TCP). Ensure that dimensions and phasing, including phase naming conventions, are consistent between the two sets.

C. Show all phases of construction beginning with the existing condition and finishing with the completed proposed section. Show all cross sections looking up station.

D. Check for dimensional consistency between the Construction Sequence sheets and all related construction phasing shown elsewhere in the structures plans.

E. For examples illustrating the content and format of completed Construction Sequence sheets, see the ***Structures Detailing Manual Examples***.

9.2 CONSTRUCTION SEQUENCE DRAWINGS

At a minimum, include the following in the Construction Sequence sheets:

A. Existing bridge deck cross section(s). The first phase in the sequence is the existing bridge deck cross section. Show all existing elements as dashed.

B. Completed proposed bridge typical section. The final phase in the sequence is the completed proposed bridge typical section. Show all proposed elements as solid.

C. Direction of traffic for all lanes. Indicate traffic traveling up station with an upward pointing arrow and down station with a downward pointing arrow.

D. Dimensions for permanent traffic and pedestrian railings, temporary barriers including minimum clear distances to above ground hazards or drop-offs, shoulders, traffic lanes, bike lanes, median width, sidewalks, and construction and demolition limits at each phase. Reference dimensions to Station Line. Use the same Station Line as used in the Plan and Elevation sheet unless other considerations make this impractical.

E. Indicate required Type K Temporary Concrete Barrier connection to deck, e.g., freestanding or bolted down. See ***Design Standards*** Index 414 for more information and anchoring requirements.

F. Location and disposition of all bridge-mounted utilities during construction and at the final condition. Indicate the type of utility, e.g., FOC, SS, OE, etc. For a complete list of common abbreviations, see FDOT ***Design Standards*** Index 001. Wherever possible, indicate nominal dimensions of conduit or pipe.

G. Location of the deck cut line. Include location in dimension scheme such that the location is tied to the Station Line. Use hatching or shading to indicate portion of structure to be demolished in each phase.

H. Phase construction labels. Label each phase of construction as "Phase I Demolition", "Phase II Construction", etc. with the relevant dimension for the limit of construction and/or demolition. Coordinate with and use same naming conventions as the TCP.

I. Cross slope of bridge deck. Indicate whether or not the proposed cross slope matches the existing cross slope. If not, indicate both existing and proposed cross slopes.

J. Right-of-way (ROW) lines. Include Temporary Construction Easements (TCE). Show dimensions from these limits to the limits of construction and to temporary work bridges or platforms.

K. Temporary and permanent walls required for phased construction.

9.3 CONSTRUCTION SEQUENCING CONSIDERATIONS

Coordinate and balance constructability, safety, economy and traffic control when developing the construction sequence for a phase-constructed bridge. Observe the following guidelines when developing the construction sequence:

A. Be familiar with the space required for each construction activity and ensure that there is enough room between the limits of construction and adjacent ROW lines, TCE lines and live traffic.

B. Be aware of the traffic control devices that may be required in the TCP. Account for widths of temporary barriers, traffic drums, barricades, etc. where their dimensions could affect the limits of construction.

C. Maximize the work area where possible and place as much distance as practicable between construction personnel and traffic. Where construction activities will be directly behind a temporary barrier, traffic drum or barricade, try to limit the duration of the phase through practical methods applied in the plans.

D. Consider bolting or pinning-down Type K Temporary Concrete Barriers to minimize the required clear distance behind the barrier and maximize lane and shoulder width. See ***Design Standards*** Index 414 for more information and anchoring requirements.

E. Coordinate pile driving operations and other foundation activities such that driving leads do not interfere with traffic on the bridge. Be aware of undesirable effects associated with pile driving such as diesel overspray from the hammer or debris from the pile cushion finding its way into live traffic.

F. Consider hammer location when battered piles are required. Avoid phasing where battered piles will be driven from above live traffic or near occupied structures or overhead utilities.

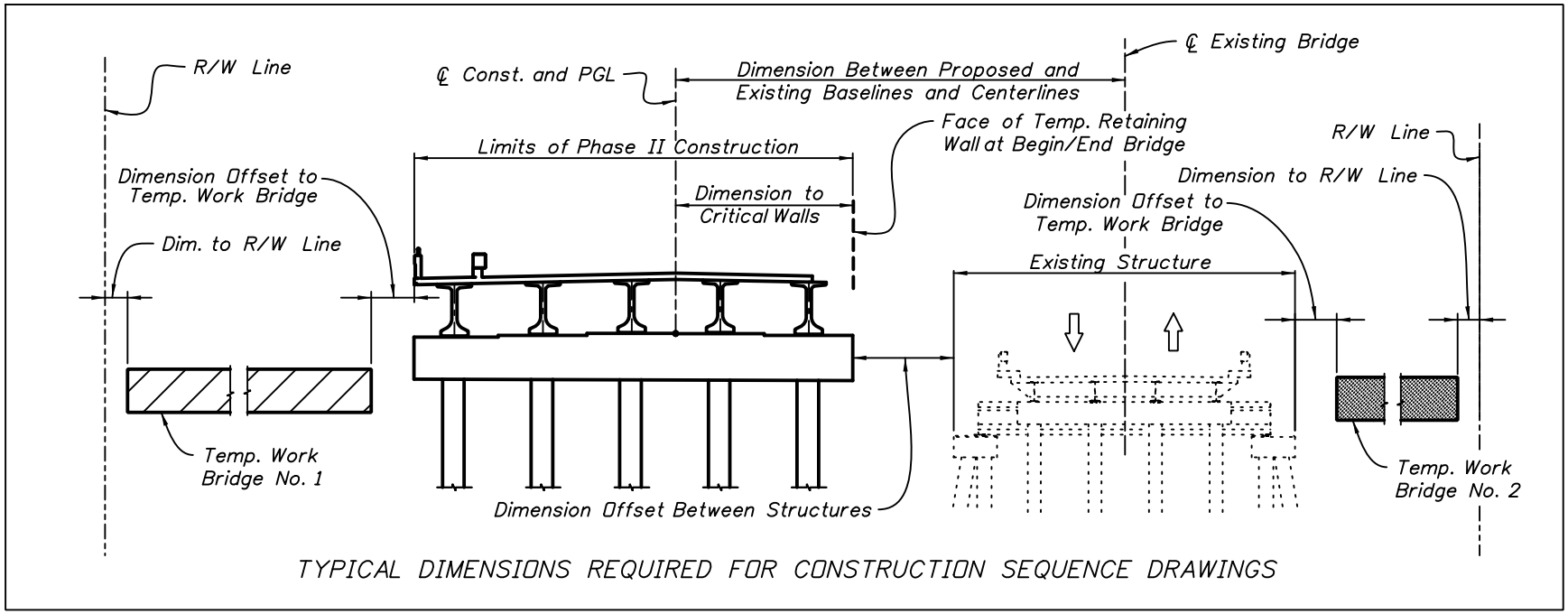
G. Properly address drainage and runoff conditions during construction. Coordinate with roadway and drainage engineers at the earliest stages of the design phase to properly incorporate safe drainage design into the construction sequence.

H. When phasing requires the use of temporary walls, in general use temporary MSE walls for fill conditions and sheet pile walls for cut conditions. If the plans call for a temporary critical sheet pile wall, include a table in the plans showing all design parameters.

I. See also other ***SDM*** chapters for specific phase construction requirements and considerations associated with pile driving, steel erection and deck casting.

J. The Construction Sequence sheets should provide sufficient information to confirm that the proposed bridge is constructible as designed. Include dimensions to R/W lines, TCE lines, existing structures, proposed structures and any other object that will aid the contractor in constructing the project. Show all potential conflicts graphically if applicable. See Figure 9.3-1 for typical dimensions.

Figure 9.3-1 Typical Dimensions Required for Construction Sequence Drawings



10 REPORT OF CORE BORINGS

10.1 GENERAL

A. This drawing, prepared by the District Geotechnical Engineer or a Consultant Geotechnical Firm, is a graphic portrayal of the subsurface conditions at the project site.

B. The information presented on this drawing and in the Geotechnical Report is used to arrive at a proper foundation design.

C. These drawings should be placed on the correct border found in the Structures Bar Menu.

10.2 SCALES

A. Draw the boring layout plan and boring logs in elevation to a scale large enough to legibly show the data and permit reasonable determination and interpretation of soil strata variations.

B. The vertical scale of the boring logs must be large enough to permit inclusion of all relevant boring data and need not be the same scale as the boring layout.

C. Selected scales must provide 100% legibility when reproduced to 11-inch x 17-inch prints.

10.3 DRAWING CONTENT

The following data should appear on the Report of Core Boring sheet(s):

A. Plan View (Boring Layout):

1. Station Line (show station values at 100-foot increments)
2. Station Line label (Base Line Survey, Center Line Construction, etc.)
3. North Arrow
4. Begin and end bridge stations and labels
5. Boring locations referenced to station line by station and offset
6. Boring labels

B. Elevation View (Boring Logs):

1. Elevation reference (vertical scale) on both left and right side of sheet (borings must be plotted in reference to elevation, not depth below ground surface)
2. Boring plots, labels, stations, offsets. (Use the soil-type symbols specified in the current ***Soils and Foundations Handbook***)
3. Ground surface elevation

4. Ground/surface water level and date recorded (note elevation of artesian head if encountered)
5. Strata description including Unified Classification Symbols
6. Standard Penetration Test (SPT) N-values
7. Rock Core Locations,% recoveries, RQD
8. Undisturbed soil sampling locations
9. Lab test results
10. In situ test locations (vane shear test, dilatometer test, pressure meter test, etc.) and corresponding test results
11. Note unusual circumstances such as: sudden drop of split spoon, loss of circulation, etc.

C. Other:

1. Soil Legend
2. Rig Type
3. SPT Hammer Type (Safety Hammer or Automatic Hammer)
4. Environmental Classification (superstructure, substructure)
5. Financial Project ID
6. Completed Title Block

10.4 TITLE BLOCK

A. The title of this drawing is "REPORT OF CORE BORINGS".

B. Show the names of the drillers who performed the borings, and the responsible Geotechnical Engineer.

11 FOUNDATION LAYOUT

11.1 GENERAL

A. This drawing shows a plan view of all spread footings, piling or drilled shafts and provides all information necessary for locating their positions in the field.

B. Use the same orientation as is shown on the Plan and Elevation sheets.

C. For examples illustrating the content and format of completed Foundation Layout sheets, see the ***Structures Detailing Manual Examples***.

11.2 FOUNDATION LAYOUT DRAWING

At a minimum, include the following on the Foundation Layout sheets:

A. Station Line at the scale required for clarity. Use the same Station Line referenced in the Plan and Elevation sheet.

B. Direction of stationing adjacent to the Station Line preferably at the extreme ahead or back station.

C. A plan of the substructure foundations such as a dashed outline of footings, bent caps, etc. Show substructure outline when considered critical for construction.

D. All horizontal curve data (or reference if shown elsewhere) including bearings of tangents.

E. Substructure stations on the Station Line. The substructure station is the intersection of the Station Line and the intermediate substructure centerline or begin/end of bridge at the front face of backwall (FFBW). For end bents, dimension the distance between FFBW and the centerline of piles.

F. On the Station Line, show the angle between intersecting reference lines.

   1. For bridges with straight (tangent) alignment, show the skew angle.
   2. For bridges with a single horizontal curve, show the angle between the substructure centerline or FFBW and the tangent to the station line at the intersection point.
   3. For bridges with multiple horizontal curves, combination of horizontal curve(s) and tangent(s), spiral curve(s), multiple consecutive tangents without horizontal curves or other complex alignments, show the angle between the substructure centerline or FFBW and the tangent to the station line at the intersection point and use coordinates to locate working (control) points for the substructure along the centerline of intermediate supports (piers or bents) or FFBW. Tie coordinates to the Florida State Plane Coordinate System.

G. The distance between the working (control) point or intersection point and adjacent pile or drilled shaft clusters, the center of footings, drilled shafts, or individual piles. In addition:

   1. Other foundation units may be dimensioned from adjacent foundations.

   2. Dimension pile or drilled shaft spacing within a cluster.

   3. Show proposed footing outlines as dashed where required for clarity.

H. All overhead and buried utilities and existing foundations in the vicinity and offset dimensions if applicable. Also indicate status of utilities, e.g. proposed, abandoned, to be relocated, etc.

I. Boring locations and labels.

J. Show test, production, existing and tension piles or drilled shafts using a unique symbol for each. Use a legend to define these symbols. Exaggerate pile or drilled shaft sizes when necessary for clarity.

K. Pile or drilled shaft sizes.

L. Indicate the amount and direction of battered piling. Indicate the control point for battered piles as the intersection of the center of pile and the bottom of footer or bottom of bent cap. All related dimensions should reference this point.

M. Pile or shaft numbers. Number piles or shafts in each substructure unit sequentially, beginning with "1" from left to right when facing the Direction of Stationing, then from extreme back station to extreme ahead station. Restart numbering at each substructure unit. This numbering scheme is not to be confused with the pile driving or shaft construction sequence. See ***Specifications*** Section 455 for more information.

N. Location of temporary critical walls in the immediate vicinity of foundation construction and/or phased construction. Indicate wall type. Reference the appropriate sheets for wall control drawings and details.

O. Sequence of construction limits. Indicate which portions of the foundation are to be constructed in each Phase of the construction sequence, if applicable.

P. Location and detail of existing and proposed foundations in the vicinity. This includes sign structure supports, retaining walls, tie backs or any other feature that may pose a potential conflict. To the extent possible, dimension existing foundations (including relic foundations of previously removed bridges) and indicate their distance from known reference points such as proposed foundations, station lines, etc. Clearly indicate existing piles that may conflict with proposed foundation elements (accounting for placement and batter tolerances) that need to be removed. Be aware of mud seal slabs which are typically larger than footing dimensions. Coordinate with the appropriate permitting agencies for direction on the removal of foundation elements, disposal of spoil, etc. In general, it is preferable to be conservative when estimating potential conflicts.

Q. North arrow, in upper right corner of sheet.

R. Right-of-way lines (roadway, railroad, etc.) and temporary and permanent easements.

S. Fender system piles.

T. Shoreline at MHW elevation.

U. Cofferdam locations. Show sheeting required to build cofferdams.

11.3 FOUNDATION LAYOUT DESIGN CONSIDERATIONS

To ensure a constructible, economical foundation design, incorporate the following guidelines when developing the foundation layout sheets:

A. Be aware of potential conflicts and/or vibration impacts on existing buildings due to foundation installation operations. For example, ophthalmologists (eye doctors), hospitals, schools, research facilities may all be negatively affected by even slight vibrations caused by foundation installation operations.

B. Test piles should be driven in Phase I for phased-constructed bridges where possible.

C. Ensure that there is sufficient overhead clearance to drive piles. Possible conflicts include overhead utilities, existing bridges and flight glide paths for bridges near airports.

D. It is strongly advised that all critical utilities be identified using Vvh (verified vertical elevation and horizontal location) methods during the design phase. Coordinate with the District Utility Engineer for determining which utilities are considered critical.

E. Additional requirements may be placed on the removal of existing foundations, excavation, transportation and disposal of contaminated materials by permitting agencies. Foundation layout plans, including removal of existing structures, disposal of spoil, disturbing environmentally sensitive areas, etc., must adhere to all permit requirements.

F. Investigate adjacent structures, including buildings, for possible deep foundation conflicts. Consider the possibility of existing battered piles obstructing the proposed bridge foundation. Retaining walls, tie backs and dead man anchors pose potential conflicts and should be considered during the design phase.

G. Also see the ***Soils and Foundations Handbook*** for additional information.

11.4 PILE DATA TABLE

The pile data table is located in the Structures Bar Menu in CADD. Do not add or delete columns within in the Installation Criteria or Design Criteria sections of the table; if information in the column is not pertinent to the project, populate the data cell with "N/A". What follows is a column-by-column description of the information to be used when filling

out the data table. See ***SDG*** 3.5 and Volume 3 ***Instructions for Structures Related Design Standards*** Index 20600 instructions for additional information.

A. Pier or Bent Number: Indicate in which pier or bent the piles are located. For pile groups within a single substructure unit that have different criteria, designate different pile groups in this column.

B. Pile Size: Indicate the standard pile size being used. For round piles, indicate diameter. Show all dimensions in inches. See ***Design Standards***.

C. Nominal Bearing Resistance: See Eq. 3.1 in ***SDG*** 3.5.

D. Tension Resistance: Indicate the required capacity of the pile to resist uplift in tons.

E. Minimum Tip Elevation: Indicate the minimum tip elevation required for lateral stability or to resist uplift. Round elevations down to the nearest foot. Do not use tenths.

F. Test Pile Length/Pile Order Length: Include the test pile length as provided by the geotechnical engineer on projects with test piles. Change “Test Pile Length (ft.)” to “Pile Order Length (ft.)” on the table heading for projects without test piles and include the pile order length as provided by the geotechnical engineer.

G. Required Jet Elevation: Indicate the required elevation that the piles should be jetted to as provided by the geotechnical engineer.

H. Required Preform Elevation: Indicate the elevation to which holes will be preformed to as provided by the geotechnical engineer.

I. Factored Design Load: Indicate the factored loads calculated during design.

J. Downdrag: Indicate the anticipated downdrag load as provided by the geotechnical engineer.

K. Total Scour Resistance: An estimate of the ultimate static side friction resistance provided by the scourable soil, as provided by the geotechnical engineer.

L. Net Scour Resistance: An estimate of the ultimate static side friction resistance provided by the soil from required preformed or jetting elevation to the scour elevation, as provided by the geotechnical engineer.

M. 100-Year Scour Elevation: Estimated scour elevation due to the 100 year storm event, found on the Bridge Hydraulic Recommendations sheet. Round elevation down to the nearest foot.

N. Long-Term Scour Elevation: Estimated scour elevation used in design for extreme event loading, found on the Bridge Hydraulic Recommendations sheet. Round elevation down to the nearest foot.

O. Resistance Factor: As provided by the geotechnical engineer.

P. Pile Cut-Off Elevations: Indicate the pile cut-off elevation to the nearest tenth of a foot. Edit Pile Cut-Off Elevation table columns or make a separate table for pile cut-off elevations if necessary.

11.5 DRILLED SHAFT DATA TABLE

Modify the pile data table for use on projects with drilled shafts as outlined in this section. The drilled shaft data table should be used on all projects with drilled shafts. Do not add or delete columns; if information in the column is not pertinent to the project, populate data table with "N/A". What follows is a column by column description of the information to be used when filling out the data table. See ***SDG*** 3.6 for additional information.

A. Pier or Bent Number: Indicate in which pier or bent the shafts are located. For shaft groups within a single substructure unit that have different criteria, designate different shaft groups in this column.

B. Shaft Size: Indicate the shaft diameter in inches. See ***Basis of Estimates*** for commonly available auger sizes.

C. Tip Elevation: The highest elevation the tip of the shaft can be constructed without authorization from the Engineer. As provided by the geotechnical engineer, round elevations down to the nearest foot.

D. Minimum Tip Elevation: The highest elevation allowed due to lateral stability and/or uplift requirements. As provided by the geotechnical engineer, round elevations down to the nearest foot.

E. Minimum Rock Socket Length: Indicate the minimum rock socket length as recommended by the geotechnical engineer.

F. Minimum Top of Rock Socket Elevation: As provided by the geotechnical engineer, indicate the anticipated elevation of layer in which top of the shaft socket will begin.

G. Factored Design Load: Indicate the factored loads calculated during design. Round loads up to the nearest ton.

H. Downdrag: Indicate the anticipated downdrag load. As provided by the geotechnical engineer, round up to the nearest ton.

I. Long Term Scour Elevation: Estimated scour elevation used in design for extreme event loading, found on the Bridge Hydraulic Recommendations sheet. Round elevation down to the nearest foot.

J. 100-year Scour Elevation: Estimated scour elevation due to the 100 year storm event, found on the Bridge Hydraulic Recommendations sheet. Round elevation down to the nearest foot.

K. Resistance Factor (phi): As provided by the geotechnical engineer.

L. Consider Nonredundant: See ***SDG*** 3.6.

12 SUBSTRUCTURE - BENTS

12.1 GENERAL

A. This Chapter covers end bents as well as intermediate bents.

B. Bent sheets will include all details necessary for the layout of the bent, bar placement and quantity estimation.

C. Show all views of the bent and sections required to construct the bent. For some structures, this may require more details than others.

D. See ***SDM*** Chapter 4 for details related to Concrete Components.

E. For examples illustrating the content and format of completed End Bent sheets, see the ***Structures Detailing Manual Examples***.

12.2 DRAWINGS AND DETAILS - GENERAL

The following details should be included in the End Bent or Intermediate Bent sheets, as applicable:

A. Plan and elevation views.

B. With the exception of End Bent 1, elevation views are typically shown looking ahead station. If it is necessary to show the view looking back station, then the view must be labeled accordingly.

C. Dimensions along front face of backwall (FFBW) or centerline cap to Station Line. Dimensions should be comprehensive to allow for complete layout of the bent. Some duplicate dimensions should be included to assist the contractor in verifying field measurements. Tie all dimensions to the horizontal control line.

D. Direction of Stationing in Plan View.

E. Angle between Station Line and FFBW or centerline cap.

F. Phase construction limits. Indicate the length along the FFBW or centerline cap to be constructed in each phase. Use separate details and sketches where necessary.

G. Existing structure removal limits. Indicate the length along the FFBW or centerline cap to be removed. Hatch existing sections showing removal limits. Use separate details and sketches where necessary or if existing plans are not available. Location of piling is critical in partial removals; therefore it is essential to locate during the design process.

H. Elevations at all locations critical to layout including:

   1. Pedestal/beam seat elevations.

   2. Top of backwall at ends, slope break point and phased construction limits.

3. Top and bottom of bent cap at ends, slope break point and phased construction limits.
4. Top of cheekwall.
5. Coping elevation at FFBW.
6. Wing wall elevations at similar locations.

Elevations may be tabulated if necessary for clarity.

I. Beam/girder centerlines. Indicate beam/girder number on each centerline. Indicate the acute angle between the beam centerline and the centerline of bearing for each beam or show as "Typical". Dimension beam spacing with respect to the bent cap and label as "Beam Spacing".

J. Pile or drilled shaft locations. Dimension pile or drilled shaft spacing with respect to the bent cap and label as "Pile/Drilled Shaft Spacing". Indicate pile batter. For battered piles, indicate where the cutoff elevation is given to, e.g., centerline of pile.

K. Pedestal / stepped cap spacing and dimensions. Dimension pedestal / stepped cap spacing with respect to the bent cap.

L. Centerline(s) of bearing and centerline of piles / drilled shafts. Dimension offset from centerline cap or FFBW and the centerlines if not coincident.

M. Pedestal details including reinforcement, concrete cover, preformed anchor bolt blockout locations & reinforcement embedment. Ensure that pedestal steel and preformed anchor bolt blockouts do not conflict with top reinforcement in bent cap.

N. Estimated quantities, including phasing and component breakdown.

O. Plan view of drilled shaft reinforcement. Detail how drilled shaft reinforcement will tie in to bent cap reinforcement.

P. Utility and drainage details. If a utility or drain pipe passes through end bent backwall, show reinforcement scheme and include bars in reinforcing schedule.

Q. Connection details for widenings. Indicate whether existing steel is to be lapped with new steel or if threaded couplers or drilling and doweling are required. Include detail for drill and dowel to replace damaged existing rebar that is to be incorporated into the completed bridge.

R. Keyways and construction joints. Show keyways and construction joints between backwall and bent cap, at ends of cap for phased construction and anywhere else required.

S. Preformed anchor bolt blockout locations. Preformed anchor bolt blockouts should be 4-inch diameter minimum. Completely detail blockouts and verify there are no conflicts with reinforcement.

12.3 DRAWINGS AND DETAILS - END BENT

In addition to the applicable detailing recommendations above, show the following when detailing End Bent sheets:

A. End bent plan and elevation views. It is generally preferred to show plan and elevation views for each end bent on a separate sheet, even if there is a great deal of similarity.

B. Begin and End bridge stations at intersection of FFBW and Station Line. Tie all dimensions back to the Station Line or the PGL whichever, is within the limits of the bent. Show dimension to the Station Line when not within the limits of the bent.

C. Outline of approach slab in elevation view.

D. Section of end bent. Section should include the backwall and include enough information for layout and bar placement. More than one section may be needed. Indicate location where all sections are taken on the end bent plan and elevation sheets.

E. Cheekwall detail showing reinforcing details. Up to four views may be required for skewed bridges and bridges with other conditions due to varying dimensions.

F. Location of soil reinforcing strap(s) at backwall when MSE walls are present. Also indicate the required load for the soil strap if it is to be designed by a separate engineer.

G. MSE walls should be shown in the plan and end bent section views when necessary. Ensure there are no conflicts between battered piles and MSE wall straps or MSE wall panels.

H. Wing wall details. Wing wall plan, side view and section should be shown on a separate sheet. Detail wing wall cap level.

I. Corner chamfers greater than ¾". On bents with skews greater than 30 degrees, chamfer acute corners a minimum of 4".

12.4 DRAWINGS AND DETAILS - INTERMEDIATE BENT

In addition to the applicable detailing recommendations in Section 12.2 above, the following issues should be addressed when detailing Intermediate Bent sheets:

A. Plan and elevation views. It is generally allowed to show a single intermediate bent sheet provided there is adequate similarity from bent to bent and enough room on the sheet for legibility.

B. Pedestal details. Stagger pedestals to accommodate skewed beams. For staggered pedestals, center the pedestal about the intersection of the centerline of beam and centerline bearing.

12.5 DESIGN CONSIDERATIONS - GENERAL

Constructability, site concerns, economy and durability must be taken into account when developing bent plans and details. The following considerations provide guidelines for the designer and detailer:

A. Make sure the dimensions of the bent are compatible with the bearing pad selection and skew. Place bearing pads orthogonal to the centerline of the beam wherever possible except where specifically shown to be skewed on the ***Design Standards***.

B. If loads and soil conditions permit, place beams directly over piles. Minimize the offset distance between the centerline of piles and centerline(s) of bearing.

C. Avoid double stirrups whenever possible by using a larger stirrup size and/or tighter stirrup spacing. Due to restrictions on bending during fabrication, #6 bars are generally the largest practical stirrup size. Keep stirrup spacing constant between piles. If triple stirrups are required, increase bent cap size.

D. A minimum height of 4" is required for all pedestals not poured monolithically. For aesthetic purposes, pedestals generally should be no more than 12" tall for bents supporting similarly-sized beams. If the pedestal exceeds 15" maximum height, step or slope cap to reduce pedestal height. Bents with beams of different heights are exempt from the 15" maximum height.

E. Size bent caps appropriately to accommodate pile driving tolerances outlined in ***Specifications*** Section 455 as well as minimum clearance requirements in the ***SDG***. Be aware of separate tolerances for battered piles.

12.6 DESIGN CONSIDERATIONS - END BENT

In addition to the applicable design considerations above, the following issues should be addressed when designing end bents:

A. When calculating elevations, allow for a minimum of 2" of clearance between the top of cheekwall and bottom of deck slab.

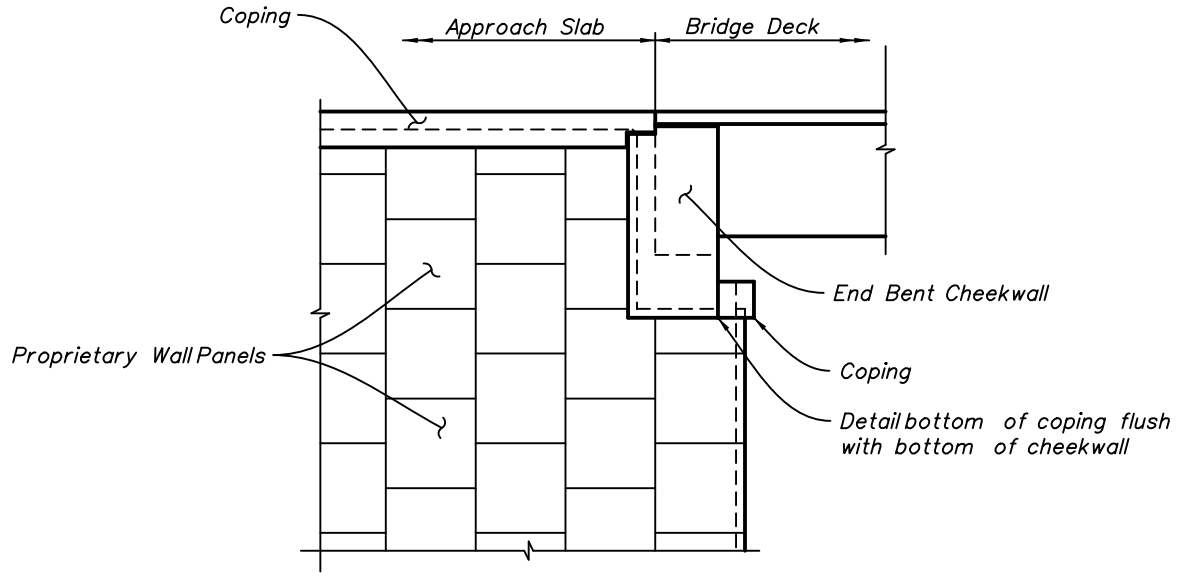
B. Minimum thickness for cheekwalls is generally 8". Backwalls and wingwalls are generally no less than 12" thick and may be thicker as required by design. When wingwalls are present on an end bent, make cheekwalls flush with wingwalls so that the outside surface is one continuous plane.

C. For end bents with backwalls greater than 50 feet in length, require vertical construction joints in backwall at a maximum of every 25 feet. Include construction joint detailed to ensure shear transfer. Place filter fabric behind backwall at all joint locations.

D. Consideration should be given to the possible need for future utilities to pass through end bent backwalls. In some instances, it may be beneficial to construct the backwall with openings intended for future use. In these cases, casting threaded inserts in the underside of the deck for future utility hangers would also be required. Design loads should include these future utilities.

E. For end bents built in conjunction with MSE walls, detail the bottom of the MSE wall coping to be flush with the bottom of the end bent cheekwall. See Figure 12.6-1.

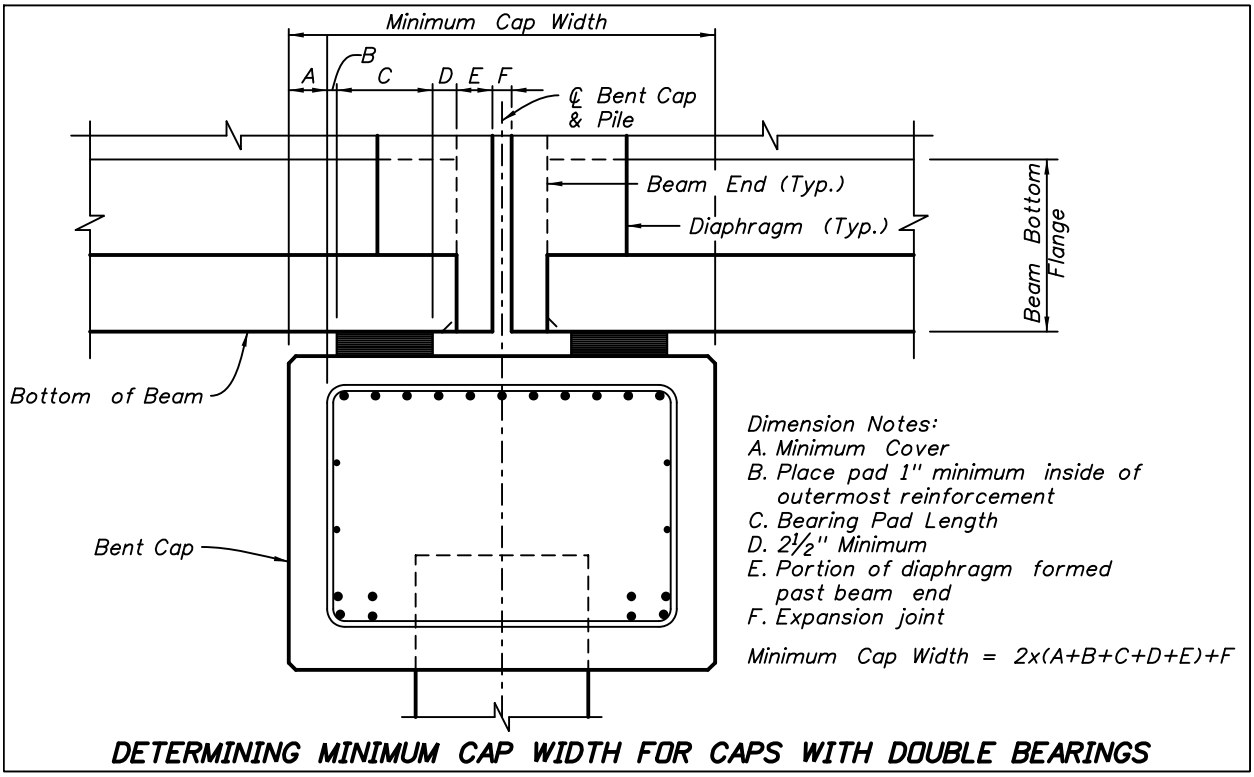
Figure 12.6-1 End Bent Cheekwall Detail at MSE Wall



F. Due to the differential settlement that will occur between the approach embankment and the end bent, do not use spread footings to support wingwalls that are attached to pile or drilled shaft supported end bents. Generally, wingwalls longer than 10' must be supported by piles or drilled shafts. Shorter wingwalls may be designed as cantilevers.

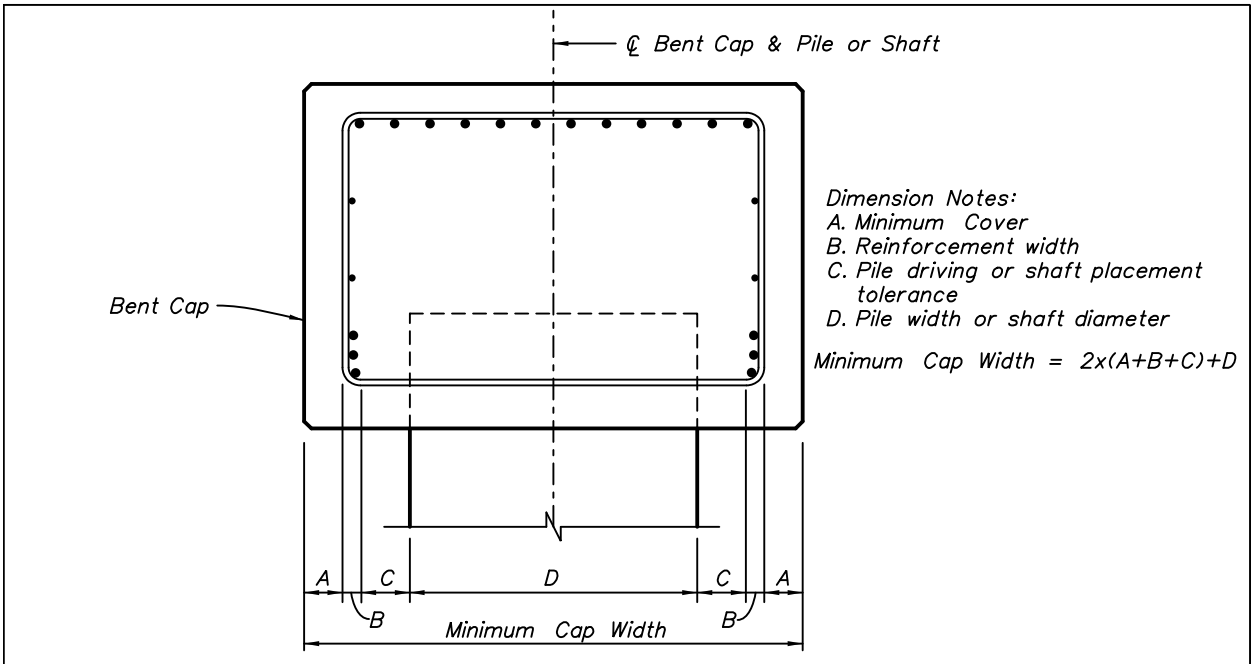
G. Size bent caps to accommodate pile driving tolerances, shaft placement tolerances, bearing pad requirements, superstructure expansion joints or any other consideration that affects bent cap width. Generally, on bents with double bearings the bearing dimension requirements will control. For bents with large piles or drilled shafts, the shaft or pile dimension plus tolerances will control. See Figure 12.6-2 and Figure 12.6-3.

Figure 12.6-2 Minimum Cap Width for Caps with Double Bearings



DETERMINING MINIMUM CAP WIDTH FOR CAPS WITH DOUBLE BEARINGS

Figure 12.6-3 Minimum Cap Width When Pile or Shaft Size Controls



DETERMINING MINIMUM CAP WIDTH WHEN PILE OR SHAFT SIZE CONTROLS

12.7 DESIGN CONSIDERATIONS - INTERMEDIATE BENT

In addition to the applicable design considerations in Section 12.5 above, the following issues should be addressed when designing intermediate bents:

A. During the preliminary design phase of water crossings, avoid placing intermediate bents in the center of the channel. This may help avoid scour issues and reduce pile lengths. Coordinate with the drainage engineer to achieve a good balance between span lengths and minimizing hydraulic issues.

B. To help reduce costs and constructability issues, minimize the use of battered piles. Be aware of conflicts battered piles may have with utilities, existing foundations or other underground obstructions.

13 SUBSTRUCTURE - PIERS

13.1 GENERAL

A. This Chapter covers multiple column, hammerhead, integral, straddle and C-piers, footings and aesthetic requirements.

B. Pier sheets will include all details necessary for the layout, reinforcement placement and quantity estimation of the pier column, cap and footing.

C. Show all views of pier and sections required to construct the pier. For some structures, this may require more details than others.

D. See ***SDM*** Chapter 4 for details related to Concrete Components.

E. To allow detailing piers at a more legible scale, detail pier caps, columns and footings for larger piers on separate sheets. Number the sheets in the order the components will be constructed, footings first, followed by columns and finally pier caps.

F. For examples illustrating the content and format of completed Pier sheets, see the ***Structures Detailing Manual Examples***.

13.2 PIER DRAWINGS AND DETAILS - GENERAL

Sections 13.2 thru 13.6 cover multi-column piers as well as other specialty types of piers such as Hammerheads, C-piers, straddle piers, integral piers, etc. Design considerations unique to these types of piers are addressed in subsequent sections. At a minimum, all Pier sheets should include the following:

A. Plan and elevation views (front and side).

B. If there is a great deal of similarity, a typical pier sheet may be used. Show all differing information such as dimensions, elevations, quantities, etc. in tabular format. If a pier is wider than can be shown clearly on one sheet, split the view into multiple sheets, while still maintaining the stacked plan and elevation format. Show matchlines on each sheet.

C. Phase construction limits. Indicate the length along the centerline pier to be constructed in each phase. Use separate details and sketches where necessary.

D. Existing structure removal limits. Indicate the length along the centerline cap to be removed. Hatch sections to be removed. Use separate details and sketches where necessary.

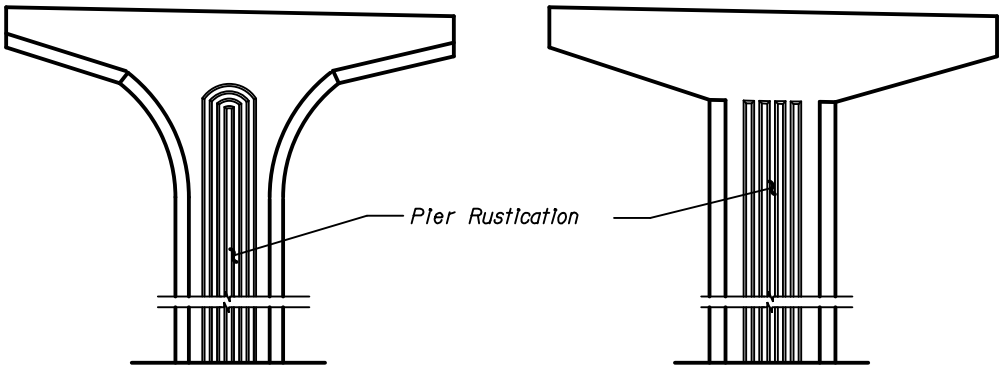
E. Pedestal spacing and dimensions. Dimension pedestal spacing with respect to the pier cap.

F. Centerline(s) of bearing. Dimension offset from face of pier to one or both centerlines of bearing.

G. Section of pier cap and pier column. Multiple sections may be required for non-prismatic members. Indicate locations where all sections are taken on the pier plan and elevation sheets.

H. Detail showing architectural treatments such as ribbing, striations or rustication. Measure concrete cover from the deepest part of relief. See Figure 13.2-1.

Figure 13.2-1 Hammerhead Pier Rustication

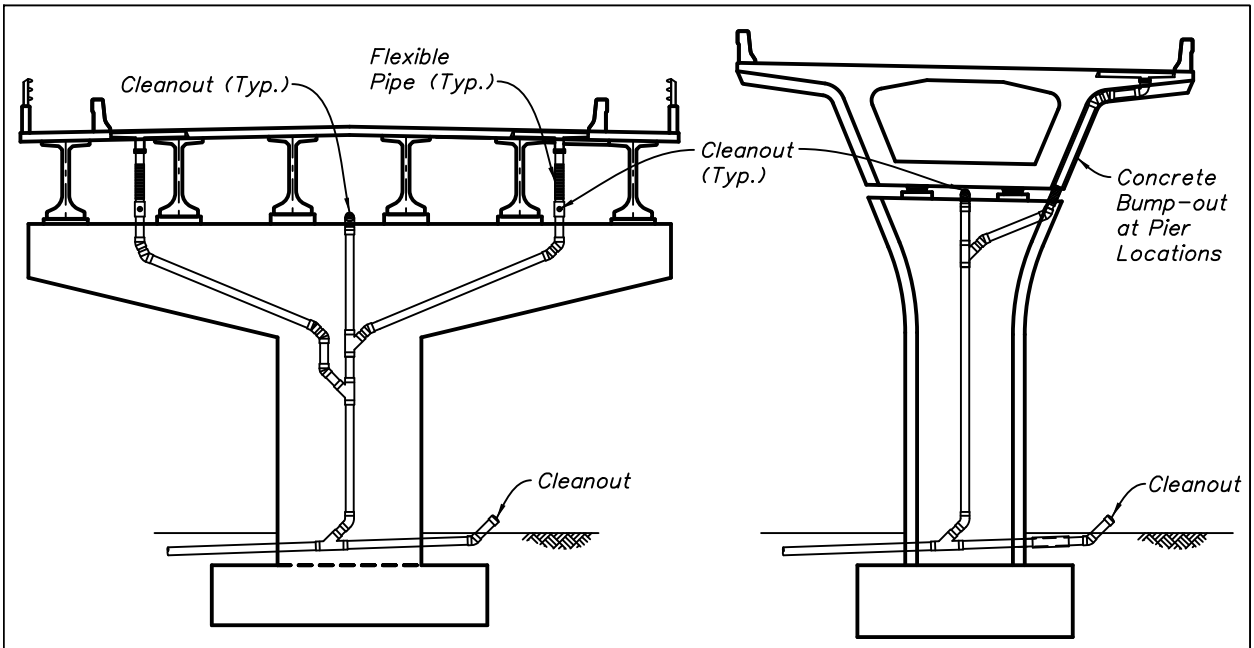


ELEVATION VIEW - HAMMERHEAD PIERS
(Flush Pier Face w/ Rustication Shown)

I. Pedestal details including reinforcement, concrete cover, preformed anchor bolt blockout locations & reinforcement embedment.

J. Drain pipes. Show connections, cleanouts, elbows and all other necessary parts. A minimum of one cleanout in each vertical and one cleanout in the lateral is required. Check for conflicts with rebar, post-tensioning ducts, anchor bolts, etc. See Figure 13.2-2.

Figure 13.2-2 Pier Drainage Details



PIER DRAINAGE DETAILS

K. Locations for anchor bolt blockouts. Indicate depth and diameter of blockouts as well as plan location.

L. All construction joints including each lift of column pours. See ***SDG*** Chapter 3 for details.

M. Estimated quantities, including mass concrete, admixture requirements, etc. consistent with the requirements stated in the General Notes. May be included on the pier plan and elevation sheet or detail sheet.

13.3 PIER DRAWINGS AND DETAILS - PLAN VIEW

In addition to the items in Section 13.2, include the following in the plan view of Pier sheets:

A. Dimensions along centerline of pier tied to the Station Line. Dimensions should be comprehensive to allow for complete layout of the pier.

B. Beam/girder centerlines. Indicate beam/girder number on each centerline. Indicate the acute angle between the beam/girder centerline and the centerline of bearing for each beam/girder or show as "Typical". Dimension beam/girder spacing with respect to the centerline pier. Tabulate if necessary.

C. Dimensions along centerline of pier tied to the Station Line. Dimensions should be comprehensive to allow for complete layout of the pier.

D. Skew angle between Station Line and centerline pier. See ***SDM*** Chapter 2 for skew definitions.

E. Direction of Stationing.

13.4 PIER DRAWINGS AND DETAILS - ELEVATION

In addition to the items in Section 13.2, include the following in the elevation view of Pier sheets:

A. Elevations at all locations critical to layout including:
   1. Pedestal/beam seat elevations.
   2. Top and bottom of pier cap at corners, ends and slope break point.
   3. Top and bottom of pier column.
   4. Finished ground.
   5. Top of footing

   Elevations may be tabulated if necessary for clarity.

B. Dashed outline of existing piers or other adjacent structures and approximate dimensions. For widenings, provide approximate dimensions and/or tie-ins between proposed and existing footings and pier caps. If the proposed bridge is built near an existing-to-remain structure, show the dashed outline of the structure and determine if dimensions are pertinent.

C. Identify a working point when required (i.e. superelevation conditions).

13.5 PIER DRAWINGS AND DETAILS - FOOTINGS

Include a separate detail for pier footings. For examples illustrating the content and format of completed Pier Footing Details sheets, see the ***Structures Detailing Manual Examples***. Generally this is a separate sheet but can be included on the pier details sheet if space permits. At a minimum, include the following footing details:

A. Footing dimensions.

B. Centerline of footing in both directions.

C. Footing reinforcement.

D. Pile or shaft locations in the footing. Dimension pile or shaft spacing with respect to the centerline of the footing. Distinguish between centerline of column and centerline of pier.

E. Outline of column(s). Show as dashed.

F. Detail of tension pile connection to footings. Include tension pile connection rebar in reinforcing bar list for the footing. See Figure 13.5-1, Figure 13.5-2 and Figure 13.5-3.

G. Footing depths below the finish grade. Footing depths should take into consideration the possibility of future widening of the roadway adjacent to the pier and the construction of additional lanes and traffic railings. Where widenings are anticipated, bury footings a minimum of three feet to top of footing. In all other locations, the minimum depth is two feet.

Figure 13.5-1 Tension Pile Details (1 of 3)

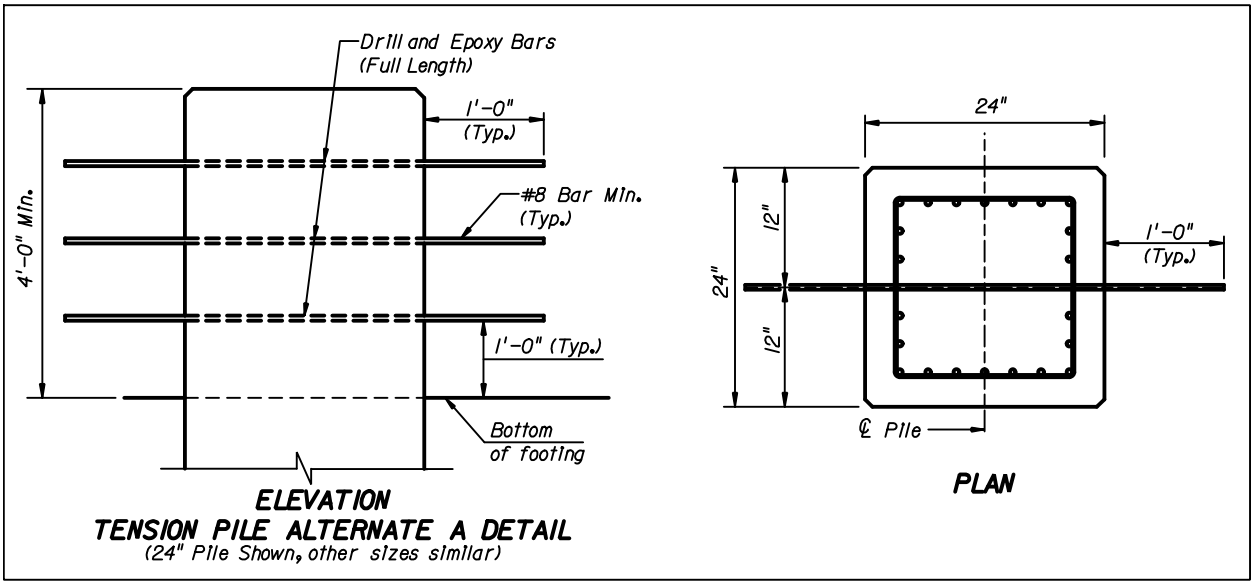
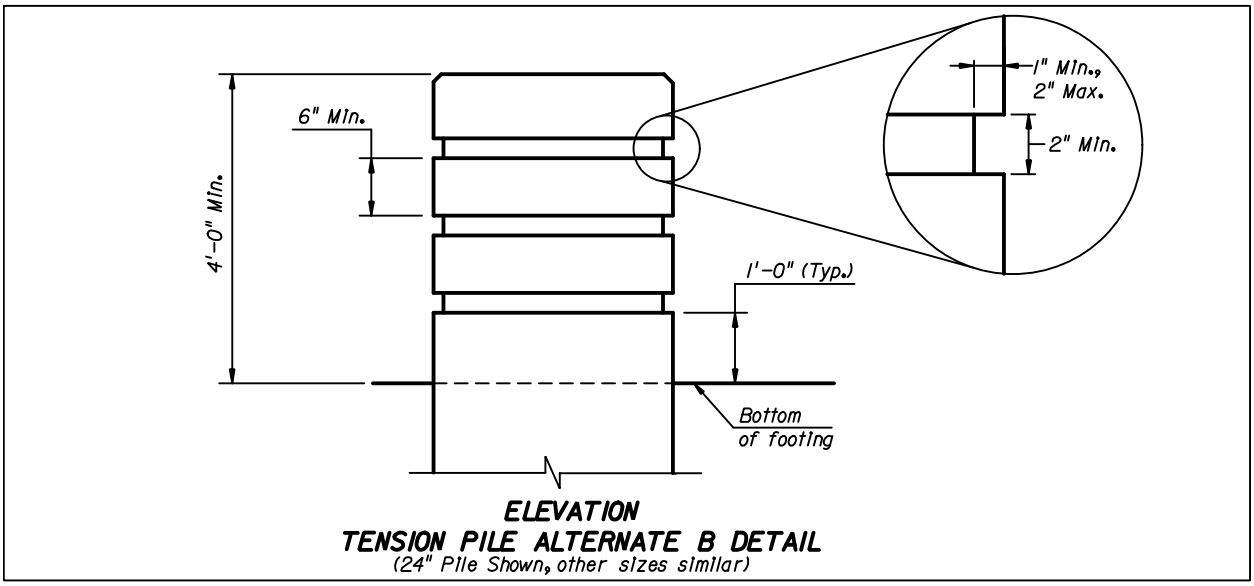
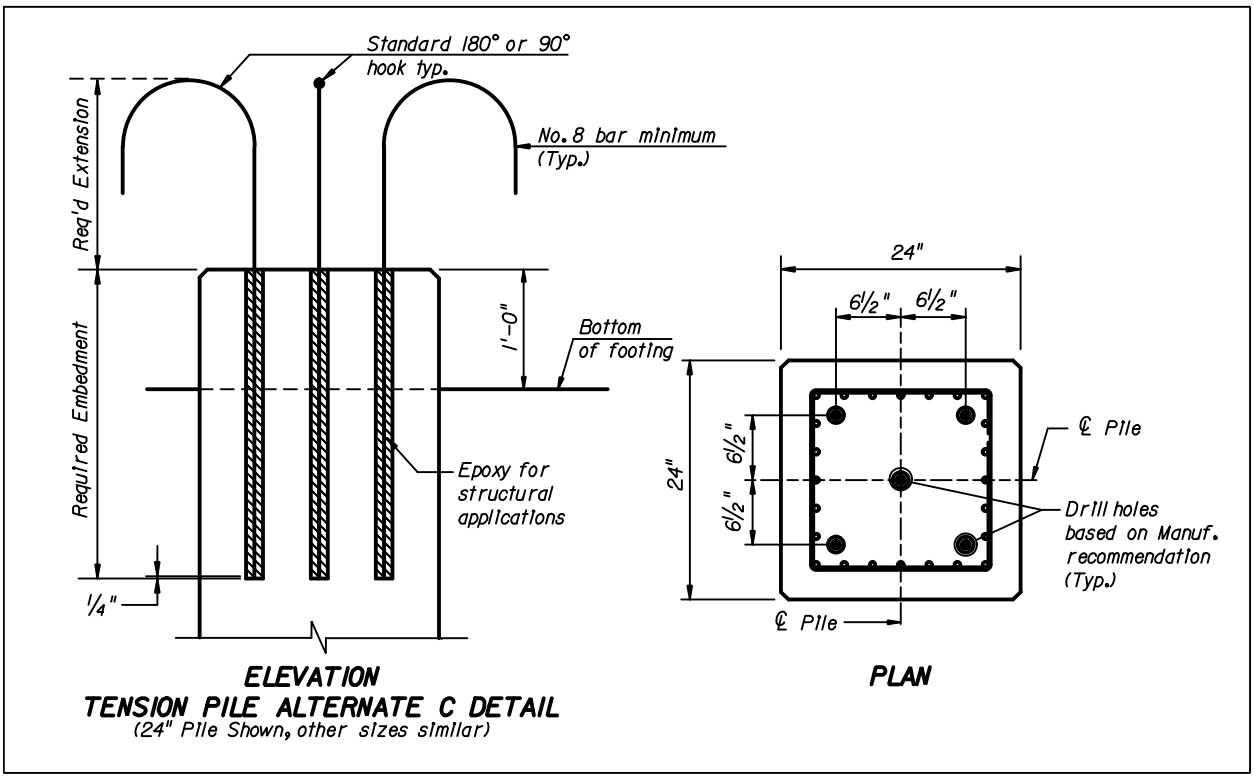


Figure 13.5-2 Tension Pile Details (2 of 3)



ELEVATION
TENSION PILE ALTERNATE B DETAIL
(24" Pile Shown, other sizes similar)

Figure 13.5-3 Tension Pile Details (3 of 3)



ELEVATION
TENSION PILE ALTERNATE C DETAIL
(24" Pile Shown, other sizes similar)

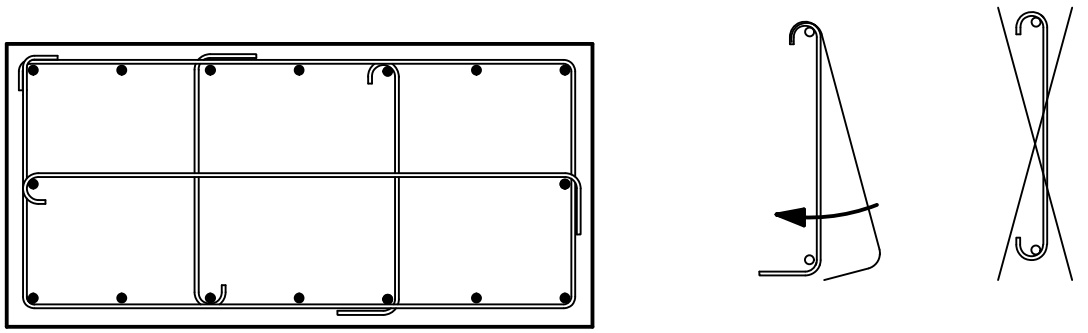
PLAN

13.6 PIER DRAWINGS AND DETAILS - REINFORCING DETAILS

Consider the following preferred reinforcing details when detailing piers:

A. Avoid column cross-ties with 180-degree hooks at each end. The preferred method is to provide a 90-degree hook at one end to allow the tie to be rotated into position. Do not use this detail when plastic hinging due to ship impact is anticipated. See Figure 13.6-1.

Figure 13.6-1 Preferred Column Tie Detail



PREFERRED COLUMN TIE DETAIL

B. When detailing variable-width pier columns, consider extending the typical pier column cage through the flared section, detailing the flare with "U" bars. Horizontal "U" bars may be detailed with a constant mark by varying lap dimensions. See Figure 13.6-2.

C. Unless plastic hinging is anticipated, standard hoops may be used on circular columns provided that the ties are rotated so the lap splice location varies throughout the length of the column. See Figure 13.6-3.

Figure 13.6-2 Variable-Width Pier Reinforcing Details

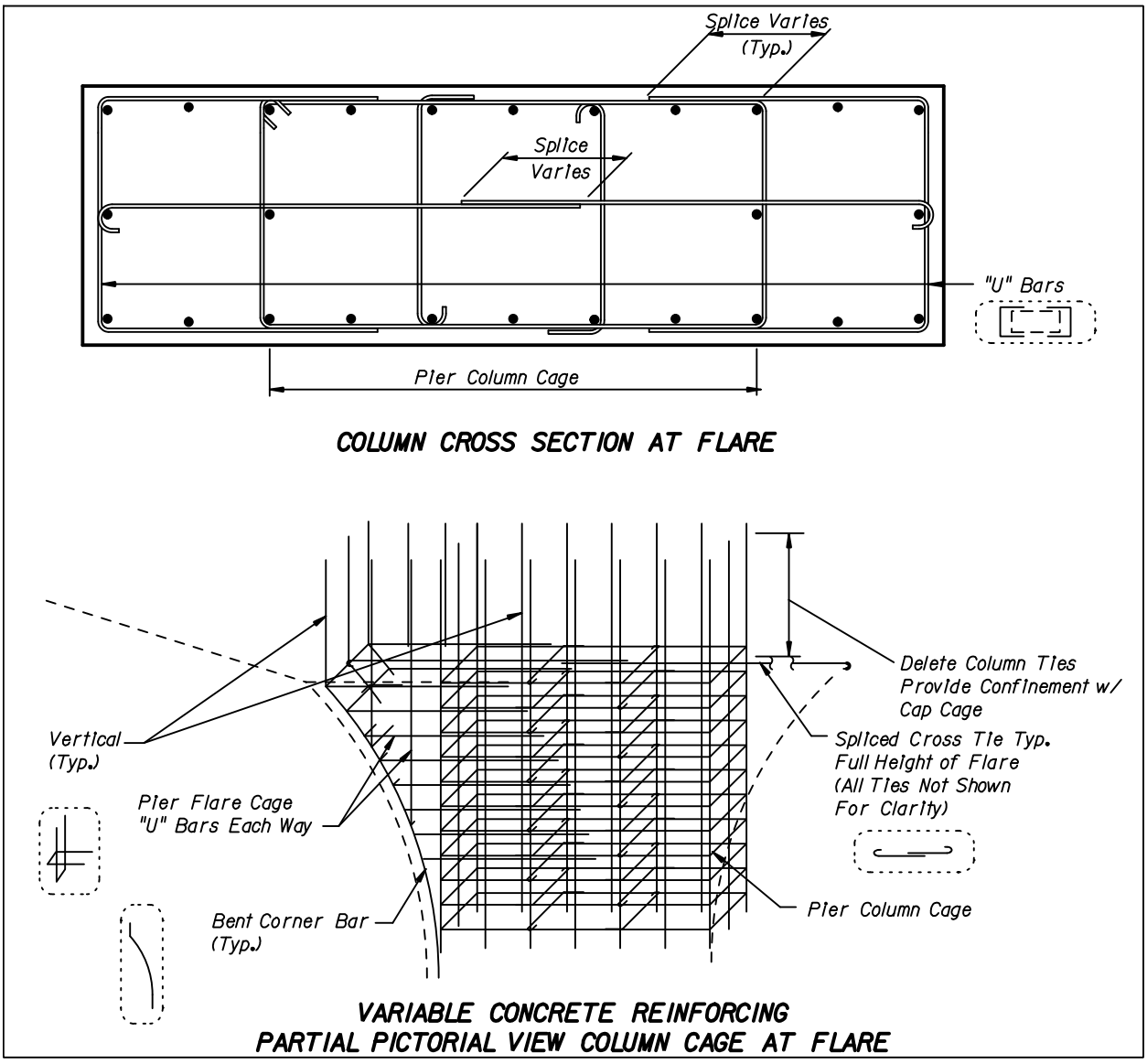
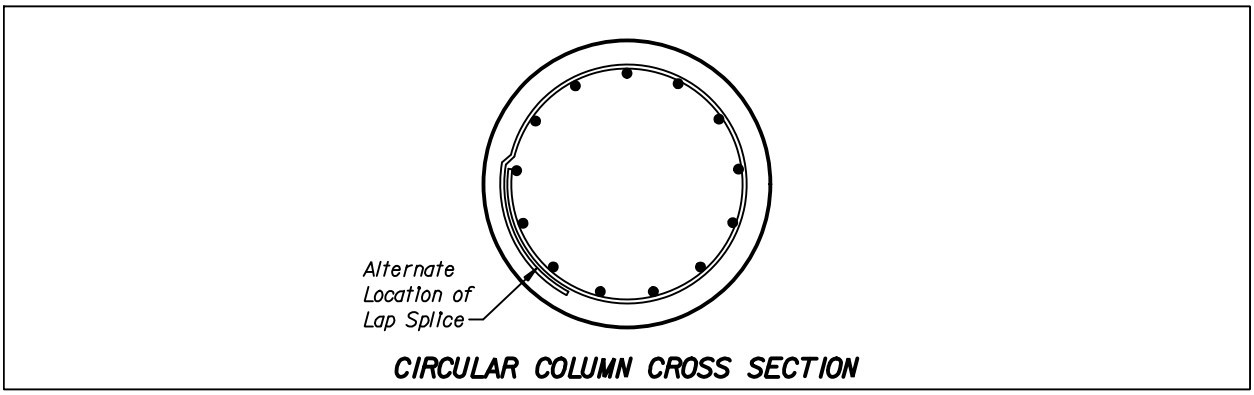


Figure 13.6-3 Circular Column Cross Section



13.7 PIER DESIGN CONSIDERATIONS - GENERAL

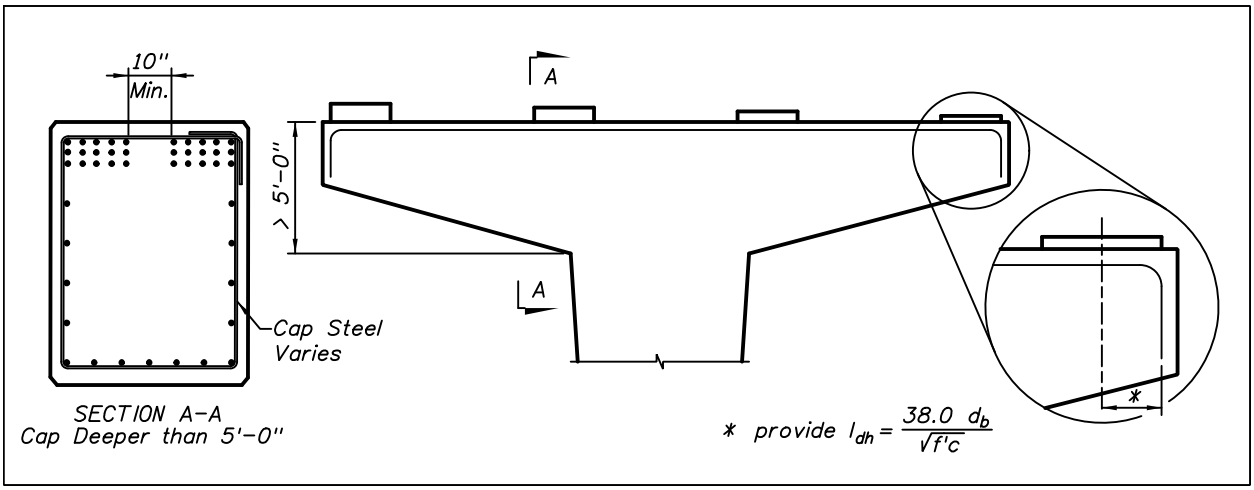
Piers should be designed with the following considerations being taken into account:

A. Detail steel to avoid conflicts between reinforcement in the cap and the column, the footing and the column, and the footing, shafts and piles. Ensure that pedestal steel does not conflict with top reinforcement in pier cap.

B. For concrete pours such as pier caps and footings that will be greater than 5 feet tall, provide a minimum 10-inch space in the main reinforcement to allow the concrete discharge chute to fit through. See Figure 13.7-1. See ***Specifications*** Section 400 for concrete discharge height limitations.

C. Main cap steel should be fully developed at cap ends in regions with large negative moments. Use "L" bars in lieu of "J" hooks in the top layer of reinforcing steel. If necessary, increase the pedestal setback dimension from edge of cap. See Figure 13.7-1.

Figure 13.7-1 Cap Steel Details



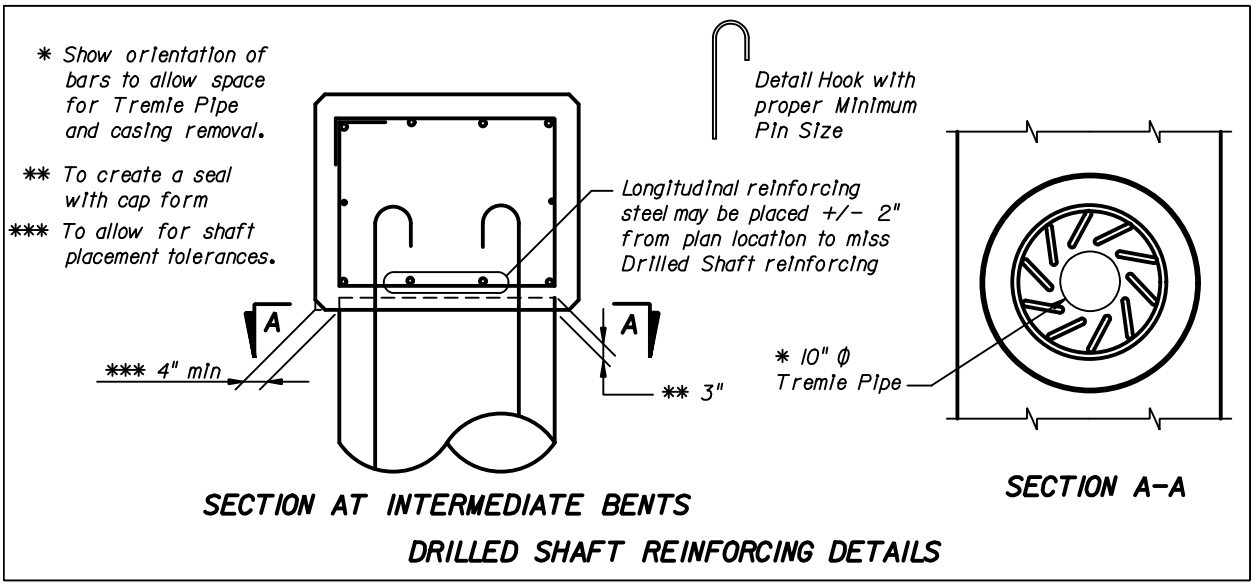
D. Fully develop column steel into pier cap and into footing. Do not include steel used in architectural details such as fillets in strength calculations.

E. Footings should be dimensioned to accommodate driving tolerances for piles and placement tolerances for shafts. Plan elevations of top of shaft should be 3" above bottom of footing to ensure there is no gap between the footing and top of shaft if constructed within tolerance. Specify a minimum embedment of 12" for piles that are not required to be developed.

F. Add details for location of main steel mats in footings giving consideration to pile driving tolerances and drilled shaft placement tolerances. Avoid conflicts between footing reinforcement and shaft reinforcement.

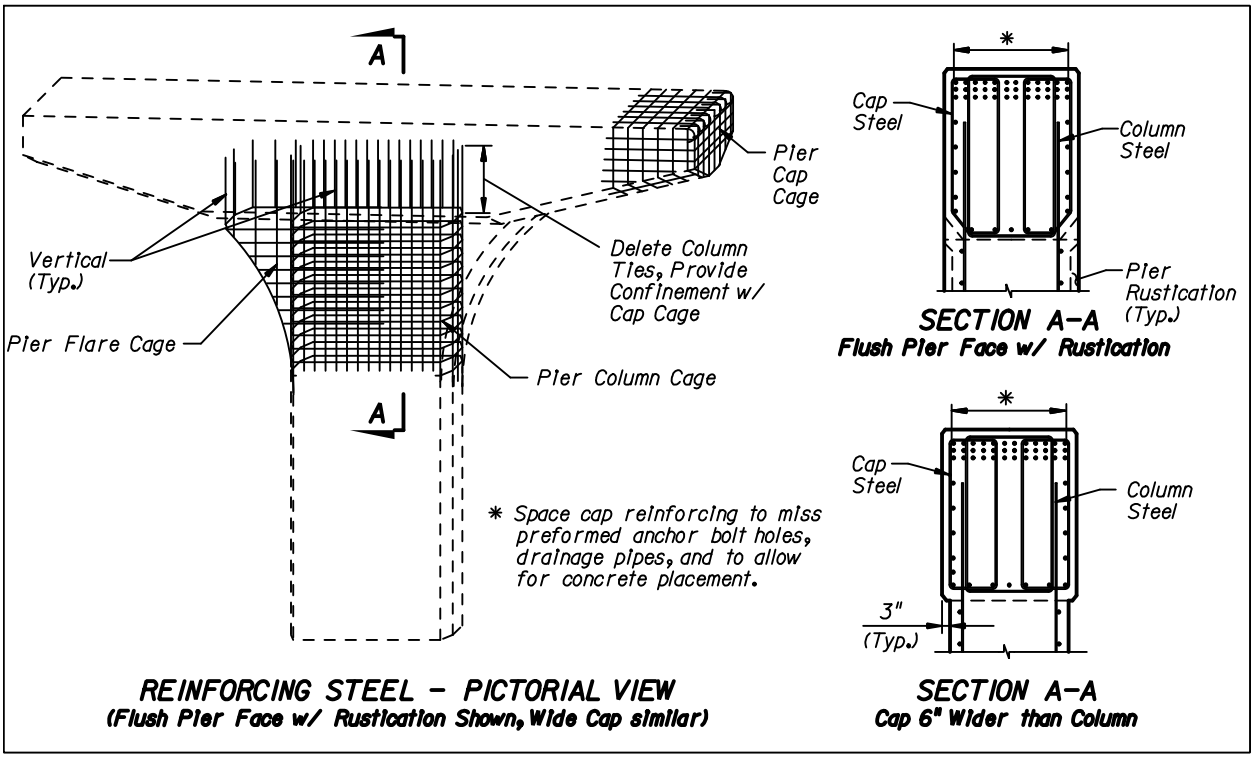
G. Detail drilled shaft reinforcement to ensure that it can be fully developed taking reinforcement cage placement tolerances into account. Whether a cap or a single column is placed on top of the shaft(s), design for the worst-case placement of the shaft reinforcement.

Figure 13.7-2 Drilled Shaft Reinforcing Details



H. Detail pier column to be stepped-in from face of pier cap by 3" on each face. This will allow column reinforcement to be inside of the pier cap reinforcement cage and will facilitate tying cap steel on the ground and lifting into the form. See Figure 13.7-3.

Figure 13.7-3 Pier Cap Reinforcing Details



I. Consider reducing column reinforcement in tall columns where allowed by the design.

J. For multi-pier bridges, only piers adjacent to roadway underneath may need to be skewed to meet horizontal clearance requirements. Consider making remaining piers normal to the bridge.

K. Verify that the dimensions of the pedestals are compatible with the bearing pad selection and skew. Place bearing pads orthogonal to the centerline of the beam wherever possible except where specifically shown to be skewed on the ***Design Standards***.

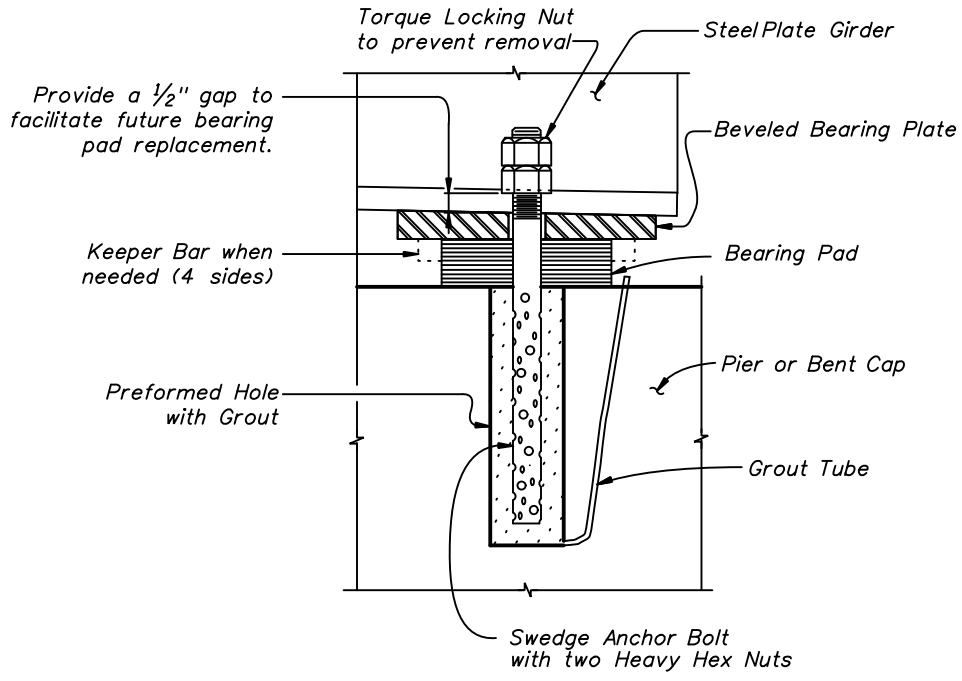
L. For caps with multiple layers of steel, place reinforcement with minimal distance between layers to maximize distance from neutral axis. Generally, 1½" clear distance between layers of reinforcement is preferred.

M. Standardize sizes and pile grid/shaft layout on pier footings throughout as much of the project as possible.

N. Design pier and column reinforcement so that it can be tied prior to erection, and not required to be tied in the air.

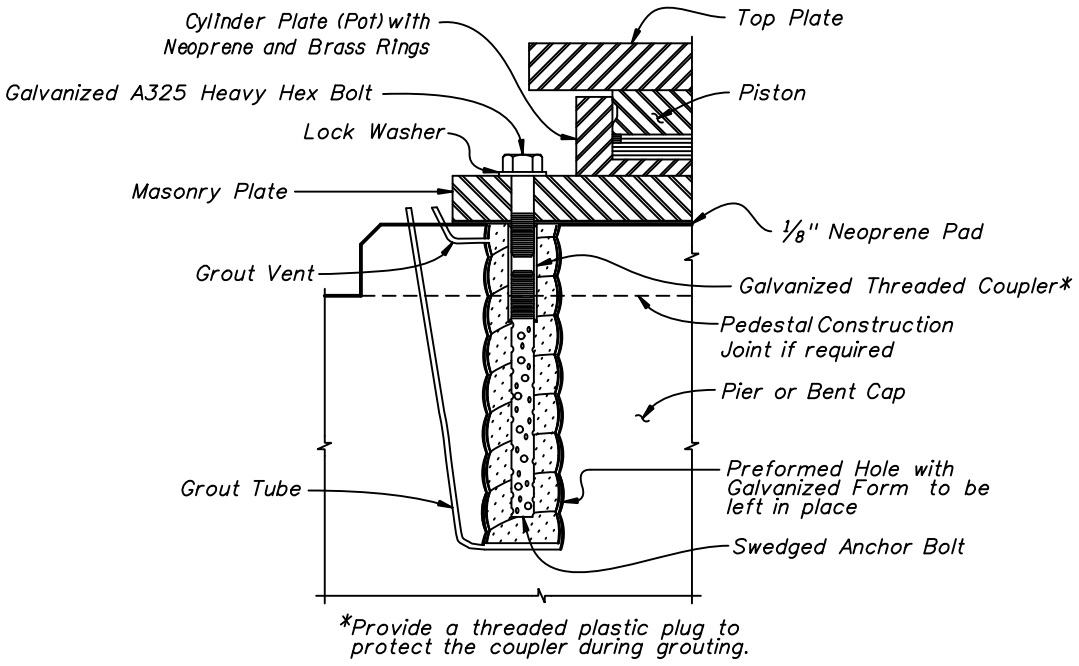
O. Preformed anchor bolt blockouts should be 4-inch diameter minimum. Completely detail blockouts and verify there are no conflicts with reinforcement. Use either a corrugated galvanized metal form that is to be left in place or a smooth removable form. See Figure 13.7-4, Figure 13.7-5 and Figure 13.7-6 for anchor bolt alternatives for both composite neoprene bearings and pot bearings.

Figure 13.7-4 Anchor Bolt Details for Composite Neoprene Bearings



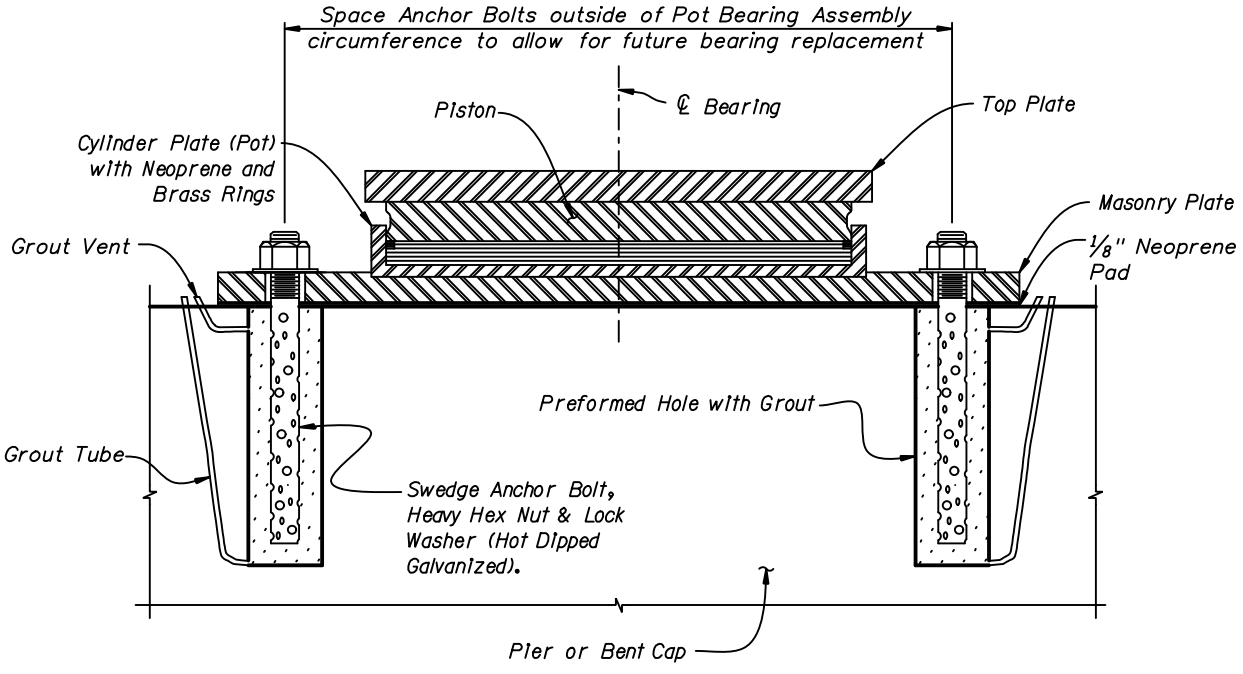
ANCHOR BOLT DETAIL FOR COMPOSITE NEOPRENE BEARINGS

Figure 13.7-5 Anchor Bolt Detail for Pot Bearings - Alternative A



ANCHOR BOLT DETAIL FOR POT BEARINGS - ALTERNATIVE A

Figure 13.7-6 Anchor Bolt Detail for Pot Bearings - Alternative B



ANCHOR BOLT DETAIL FOR POT BEARINGS - ALTERNATIVE B

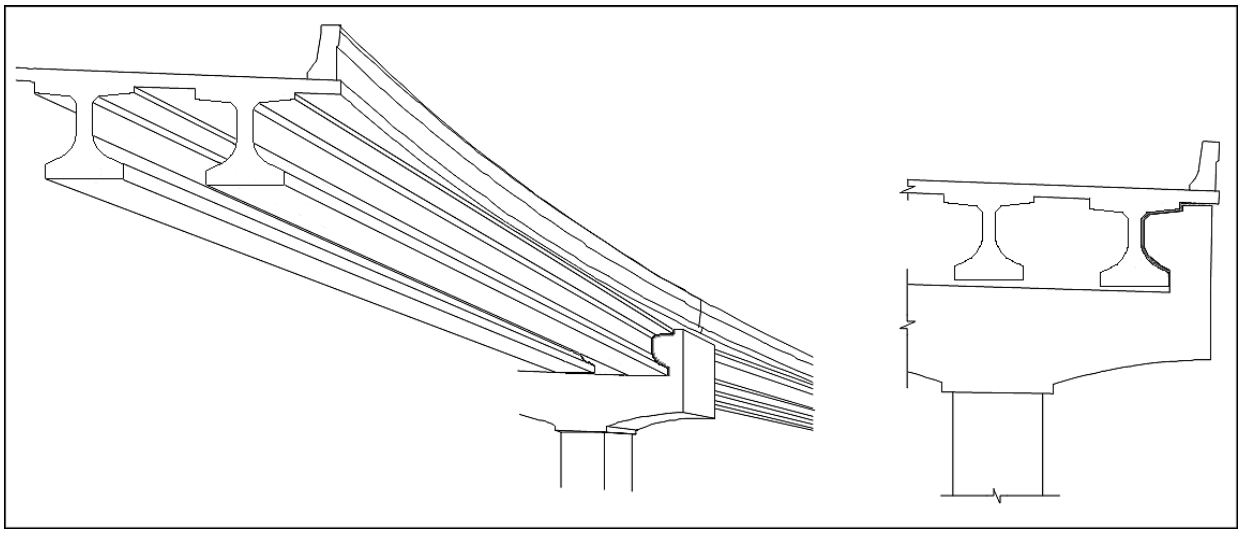
13.8 PIER DESIGN CONSIDERATIONS - PIER AESTHETICS

Aesthetics should balance efficiency, economy and elegance. For an in depth discussion of bridge aesthetics, refer to the ***Bridge Aesthetics Sourcebook*** by the Bridge Aesthetics Subcommittee of the Transportation Research Board. The following issues and suggestions can be incorporated into any project's aesthetic plans to help achieve these goals:

A. When determining pier shape and proportions, look at the tallest and shortest structures as well as minimum and maximum cross slopes. Use uniform dimensions that will result in acceptable aesthetics at all locations. This applies to multi-pier bridges as well as multi-bridge projects.

B. The use of cheekwalls on pier caps is encouraged at locations where two dissimilar beam types are to be used on adjacent spans. A curved bridge supported by straight beams is another location where pier cap cheekwalls can economically enhance a bridge's aesthetics. Cheekwalls on skewed bridges should be parallel to the skew, not normal to the pier cap. The inside face of pier cap cheek walls should be poured close to the exterior beam to prevent shadowing. Provide sufficient clearance between the cheekwall and the exterior beam to allow for jacking of the span and bearing replacement. See Figure 13.8-1.

Figure 13.8-1 Pier Cap Cheekwall Details



C. For multi-bridge projects, develop a family of pier shapes at the earliest stages of the project. Choose pier shapes that both meet the structural requirements of the bridges within a project while also providing pleasing shapes at all locations. Consider multi-column piers, hammerheads, straddle piers, C-piers or any other special pier type as the project requires. See Figure 13.8-2 and Figure 13.8-3.

Figure 13.8-2 Example Pier Shapes

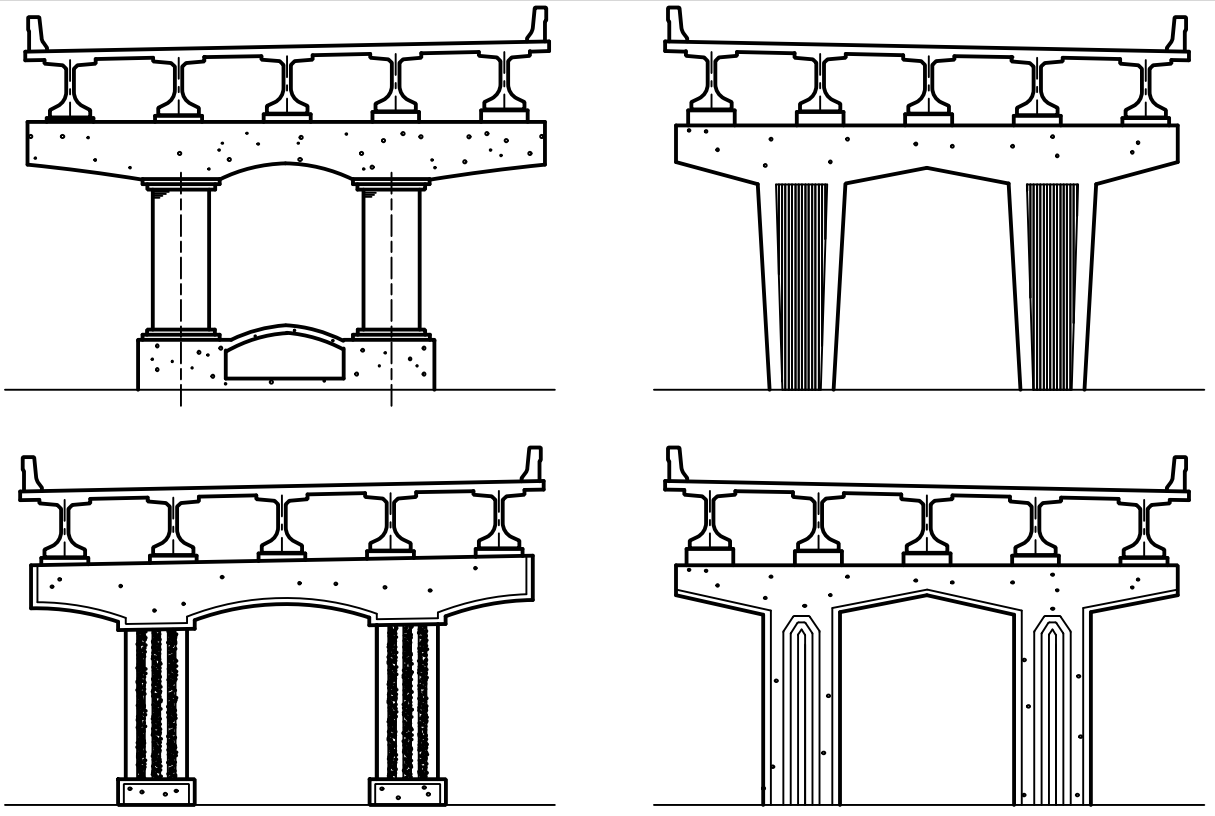
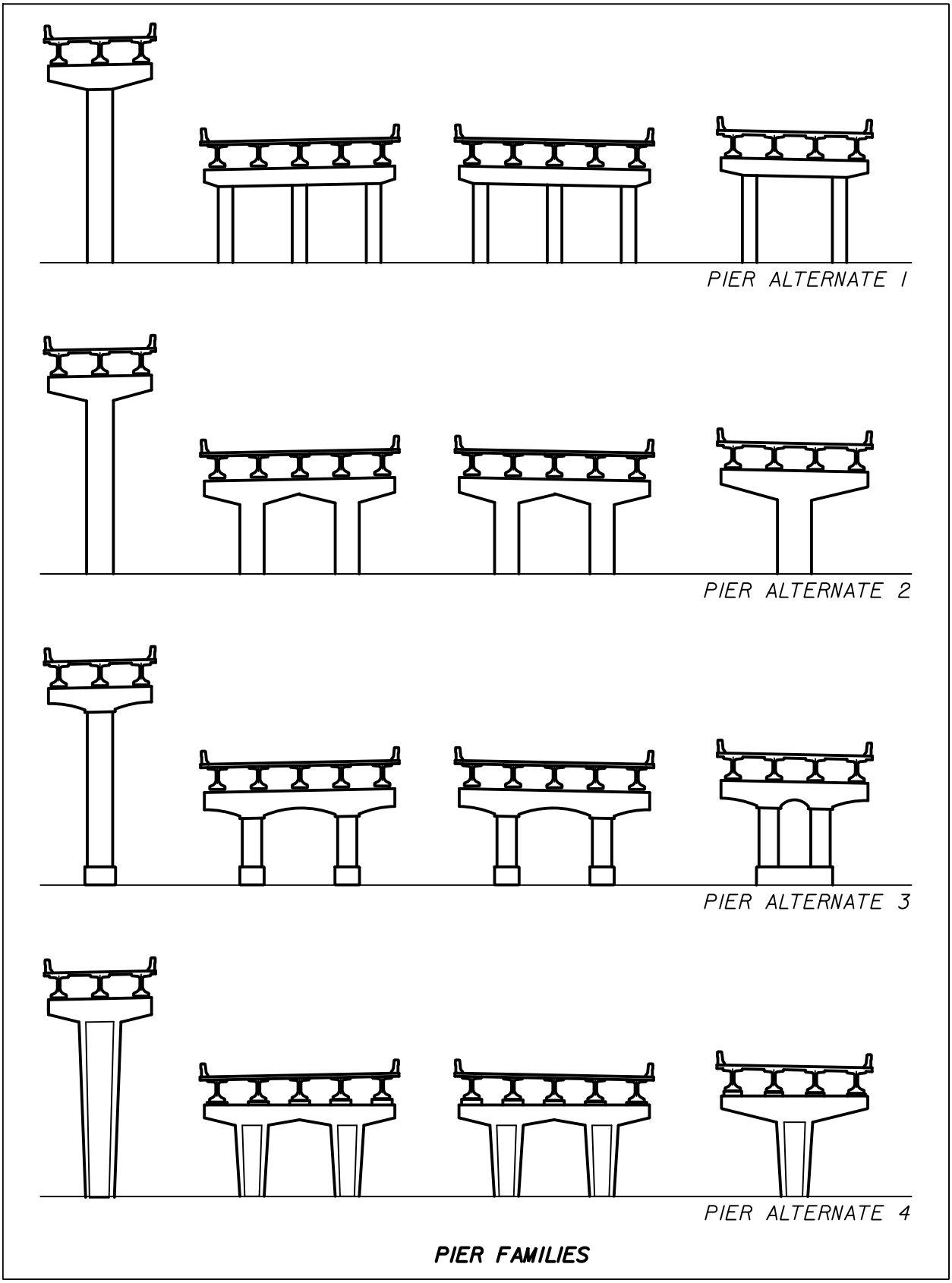
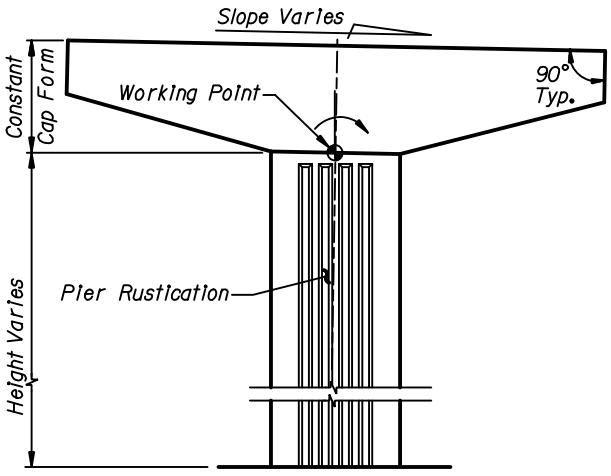


Figure 13.8-3 Pier Families



D. For piers that utilize a cap form that will be rotated to accommodate variable cross slopes, pier rustication or striation details should stop short of the pier cap in the column. This will avoid misalignment of the form liner as the pier cap rotates about the working point to make up the superelevation. See Figures 13.8-4 and Figure 13.9-1. This will also allow column steel to extend inside cap cage while maintaining concrete cover per 13.7.J above.

Figure 13.8-4 Rustication Detail At Working Point Of Rotated Pier Cap

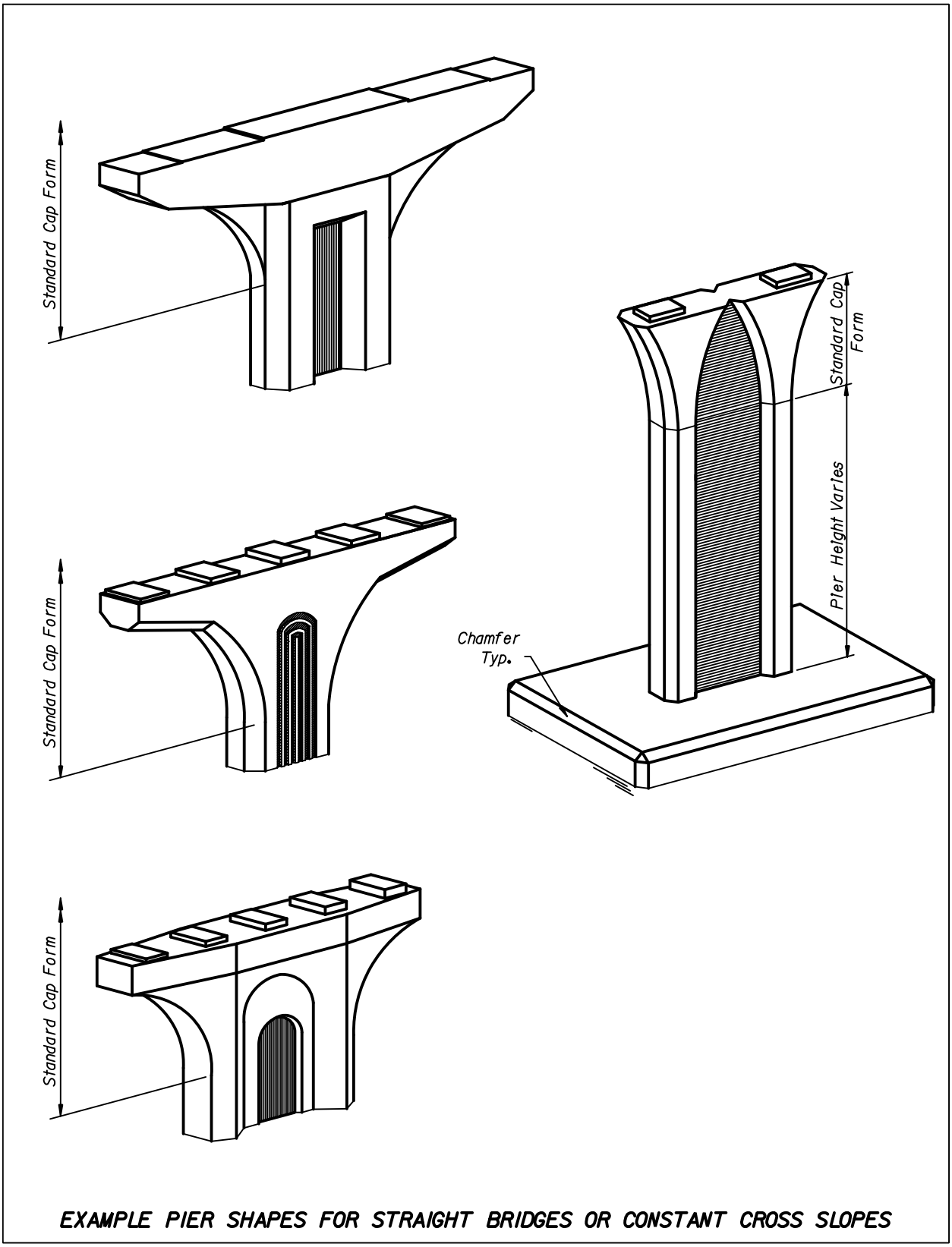


ELEVATION VIEW – HAMMERHEAD PIER
Rustication Stops Below Working Point

E. For piers with constant cross slope sections, piers that utilize flush columns and caps, or piers that utilize split cap forms flush with the column, the rustication detail may extend into the cap. Detail the pier cap so that the same form can be re-used for maximum efficiency. See Figure 13.8-1, Figure 13.9-2, Figure 13.9-3 and Figure 13.9-4.

F. Larger chamfers can help reduce the apparent size of large, bulky concrete elements. See Figure 13.8-1.

Figure 13.8-1 Example Pier Shapes for Straight Bridges or Constant Cross Slopes



13.9 PIER DESIGN CONSIDERATIONS - HAMMERHEAD PIERS

In addition to the applicable sections above, the following issues should be addressed when designing and detailing hammerhead piers:

A. It is preferable to rotate pier cap about a working point or utilize split forms to facilitate varying cross slopes. Acceptable methods are shown in Figure 13.9-1 through Figure 13.9-4. Utilize standard form shapes where practical to minimize formwork and standardize the reinforcement cage.

Figure 13.9-1 Pier Cap Details (1 of 4)

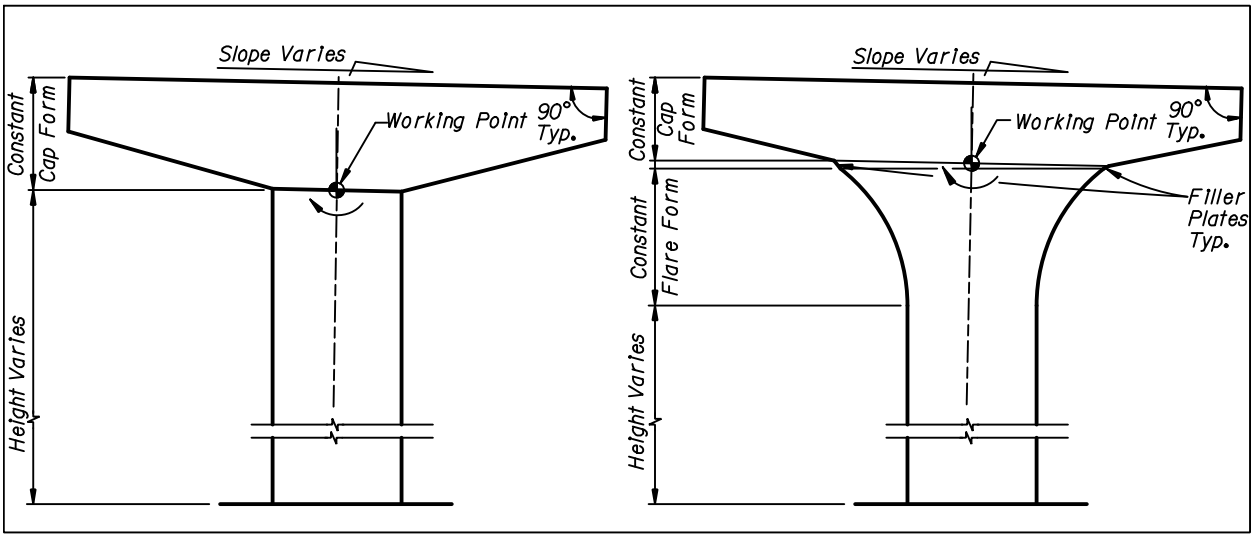


Figure 13.9-2 Pier Cap Details (2 of 4)

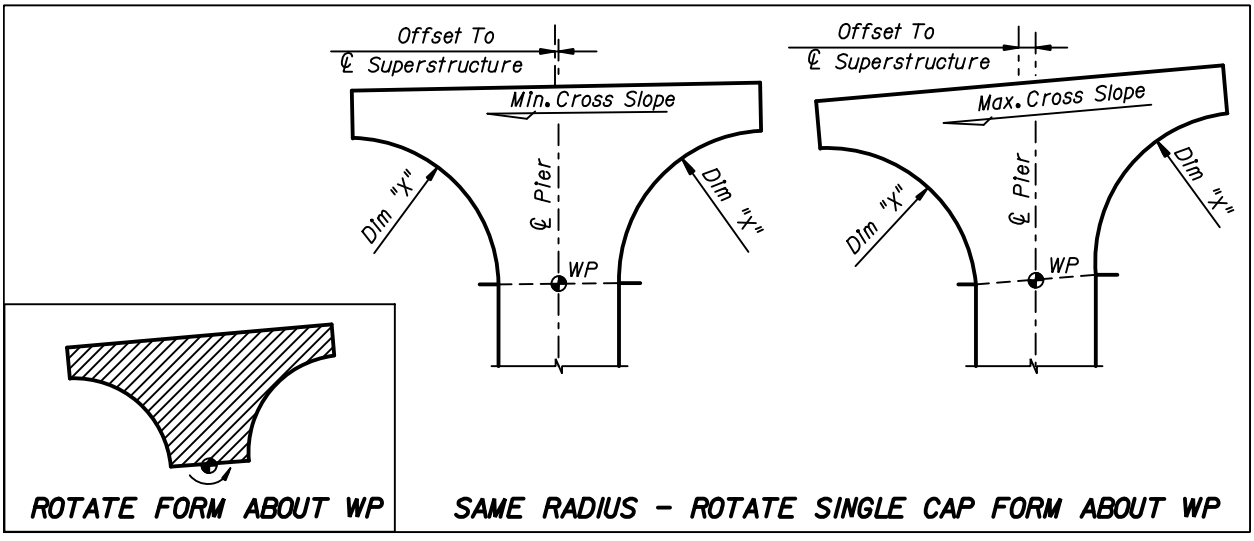


Figure 13.9-3 Pier Cap Details (3 of 4)

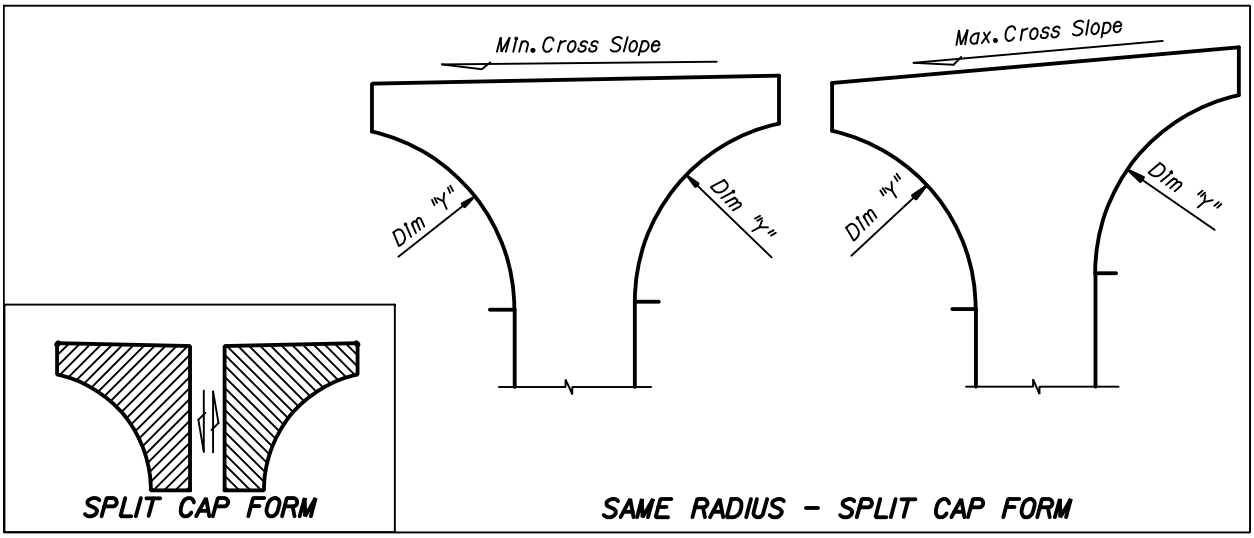
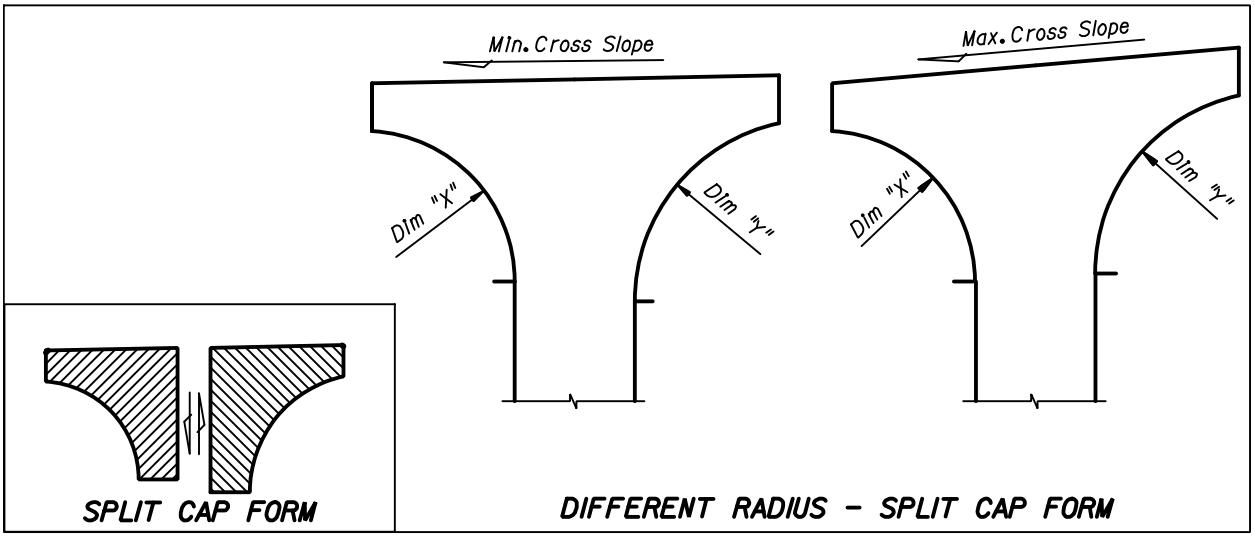
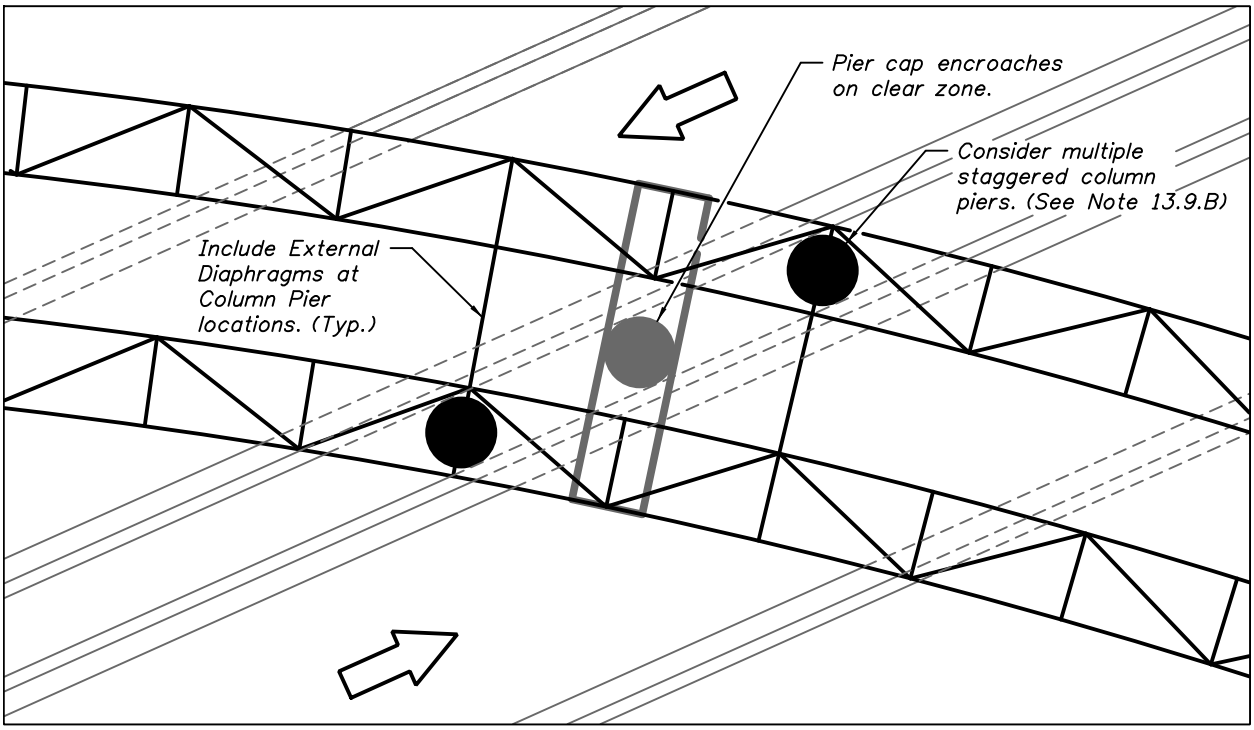


Figure 13.9-4 Pier Cap Details (4 of 4)



B. To avoid pier caps encroaching on the clear zone, consider using multiple staggered single column piers in lieu of an integral cap. See Figure 13.9-5.

Figure 13.9-5 Staggered Column Alternate



13.10 PIER DESIGN CONSIDERATIONS - INTEGRAL PIERS

Integral piers present significant design and detailing challenges. Figure 13.10-1 through Figure 13.10-3 show general details that are commonly found in integral piers.
In addition to the applicable sections above, the following issues should be addressed when designing and detailing integral piers with structural steel caps and/or columns (See Figure 13.10-1and Figure 13.10-2):

A. Show shear stud spacing and size for steel beams connected to the integral pier cap.

B. Show temporary supports.

C. Specify Grade HPS 70W steel in all integral pier caps for added toughness.

D. Completely detail openings required for drain pipes and utilities which are intended to pass through the integral cap.

E. For steel integral caps, show splice plates and bolt spacing and layout. Indicate bolt size and type.

F. Indicate weld type and size. Reference welding symbols at www.aws.org.

G. Designate all Fracture Critical Members (FCM). FCMs may be indicated in tabular format if needed.

H. For steel integral piers include a plan note requiring shop assembly where fit-up could be an issue.

Figure 13.10-1 Steel Integral Pier Cap Details

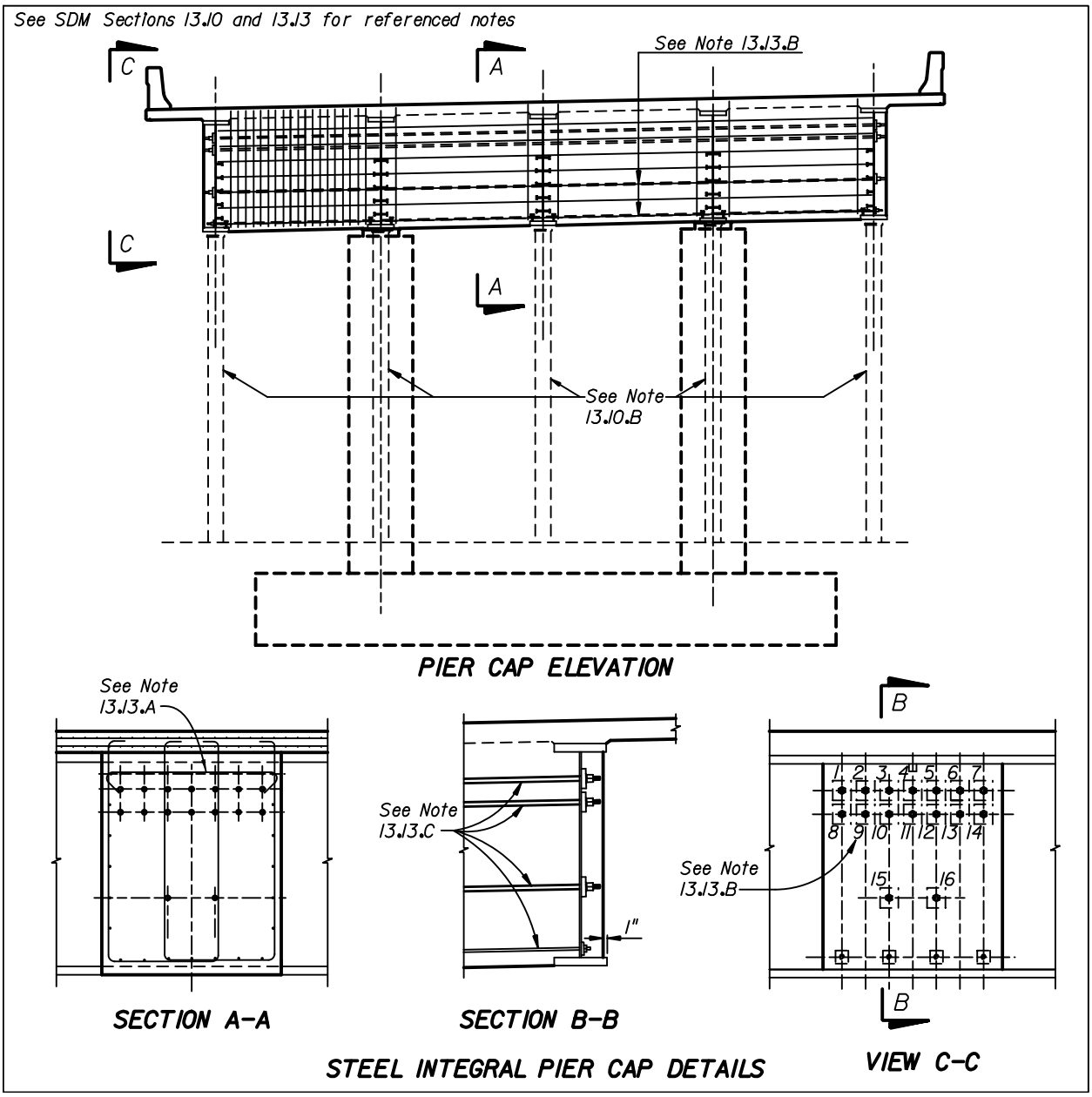
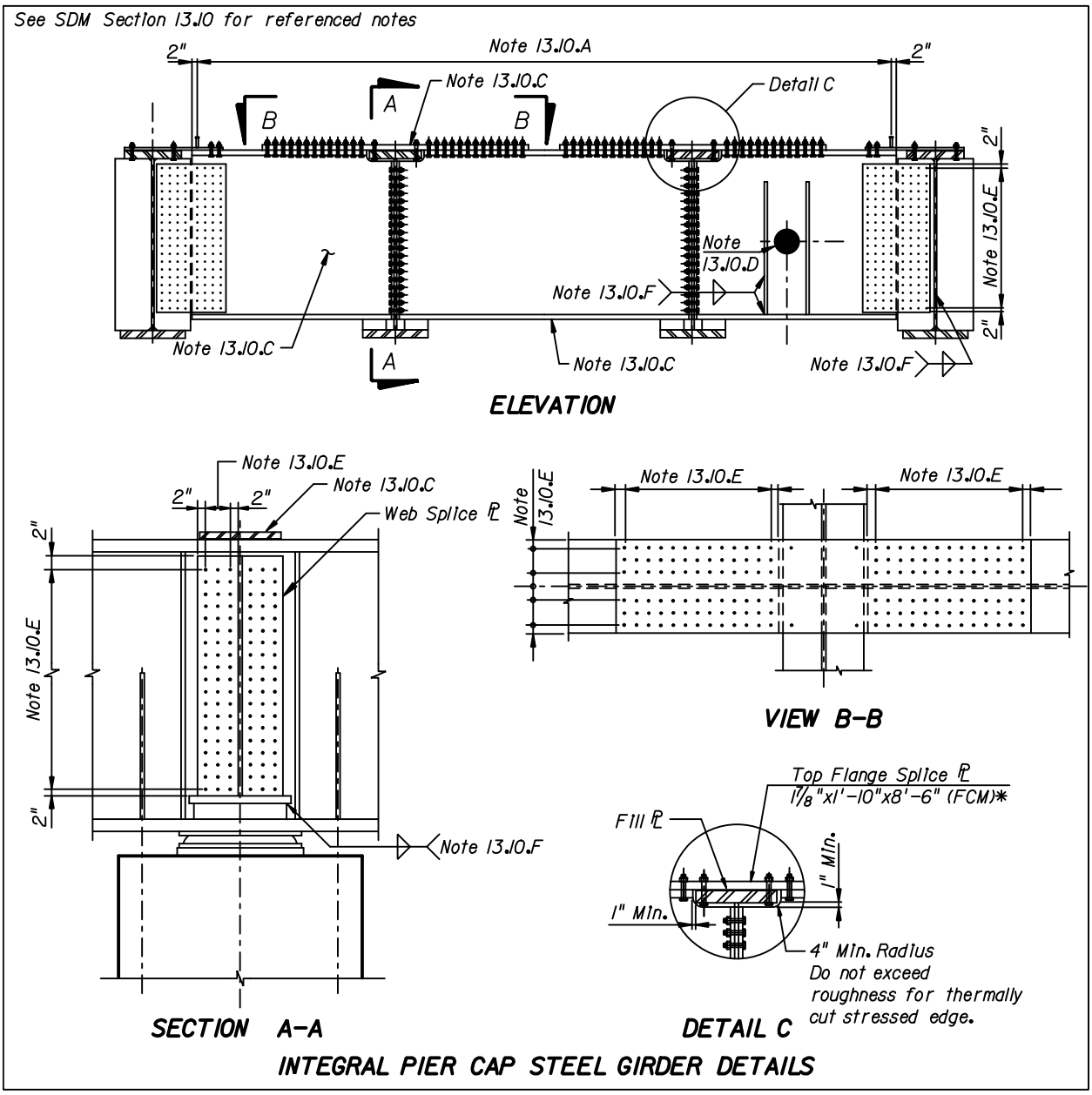


Figure 13.10-2 Integral Pier Cap Steel Girder Details



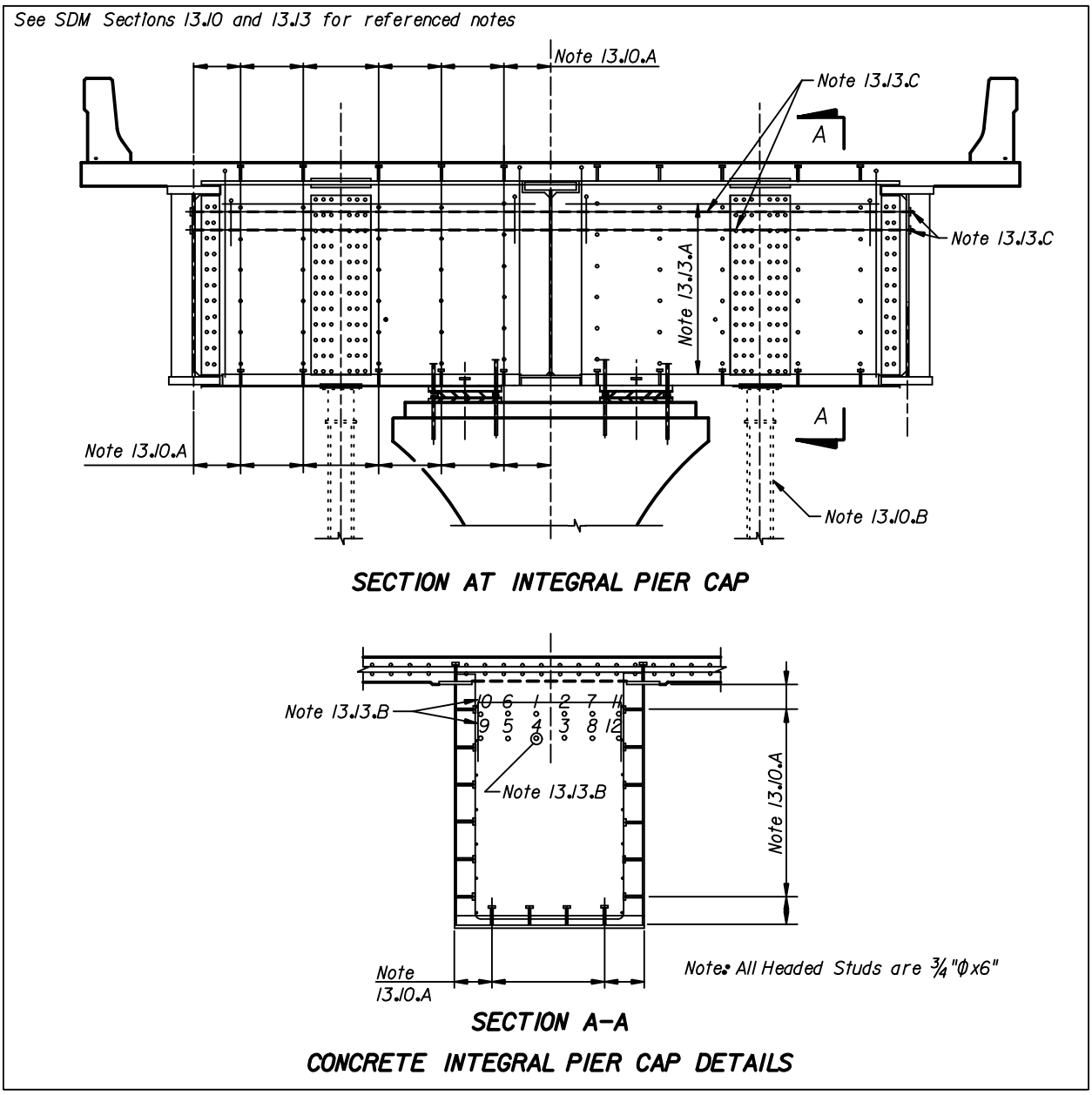
In addition to the applicable sections above, the following issues should be addressed when designing and detailing integral piers with concrete caps and/or columns (See Figure 13.10-3):

I. Generally, design concrete integral pier caps utilizing post tensioning to maximize the efficiency of the design.

J. Show temporary supports.

K. See ***SDM*** 13.13 for Post-Tensioning requirements.

Figure 13.10-3 Concrete Integral Pier Cap Details



13.11 PIER DESIGN CONSIDERATIONS - STRADDLE PIERS

In addition to the applicable sections above, the following issues should be addressed when designing and detailing straddle piers:

A. When sizing a straddle pier, take into consideration the likelihood that the roadway underneath will be widened in the future. Depending on this possibility, the straddle pier should be sized to accommodate the ultimate condition.

B. Concrete is the preferred material for straddle piers. However, if a steel pier cap is to be used, follow these guidelines:

   1. Do not place stiffeners on outside of cap.
   2. Specify Grade HPS 70W steel for straddle pier caps for added toughness.
   3. Be aware of additional considerations required for continuous beam designs due to straddle pier cap deflections.
   4. Avoid Fracture Critical Members (FCM) where possible. When unavoidable, designate all FCMs in the plans. Tabular format is acceptable.
   5. Indicate weld type and size. Reference welding symbols at www.aws.org.
   6. Inspection access must be considered during design.

13.12 PIER DESIGN CONSIDERATIONS - C-PIERS

In addition to the applicable sections above, the following issues should be addressed when designing and detailing C-piers:

A. Reasonable effort should be made to avoid tension in piles.

B. It is desirable to center the footing on the centroid of the deadload of the pier plus superstructure. This will help avoid piles in tension.

13.13 PIER DESIGN CONSIDERATIONS - POST-TENSIONING

When designing and detailing post-tensioning in piers, be aware of the following design considerations:

A. Detail reinforcement to ensure there are no conflicts with post-tensioning ducts and anchorages. Also check for conflicts with anchor bolt blockouts.

B. Clearly number post-tensioning ducts/bars for post tension stressing sequence.

C. Label PT bar size, PT nut and jacking (bearing) plate.

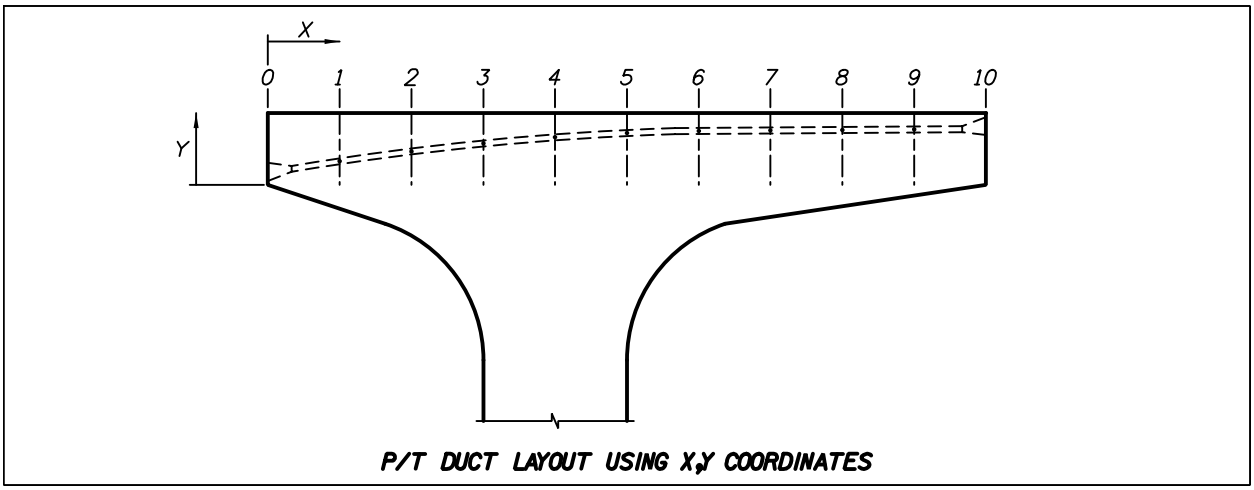
D. Include a detailed stressing sequence for PT tendons. Indicate what stage of construction each tendon will be stressed and to what level. Indicate minimum concrete strengths for concrete cap and deck before stressing operations can begin.

E. Address the placement of PT bars through the pier column. Ensure that there are no conflicts between anchorages or ducts with the reinforcement cage in the pier cap, footing or the column.

F. Create a simple P/T duct layout using x,y coordinates. Give vertical dimensions to the center of the duct. See Figure 13.13-1.

G. Reference appropriate Design Standards for additional requirements regarding post tensioning layouts and details.

Figure 13.13-1 P/T Duct Layout Using X,Y Coordinates



14 FINISH GRADE ELEVATIONS

14.1 GENERAL

A. This drawing is a typical section and schematic plan view of the superstructure that shows finish grade elevations.

B. For spans of equal length, only one schematic plan view need be shown.

C. Create equally-spaced transverse lines (T-lines) to show elevations. Turn skewed T-lines perpendicular to bridge coping at the gutterline. Maintain cross slope of bridge deck under traffic railings, raised medians and pedestrian parapets.

D. For examples illustrating the content and format of completed Finish Grade Elevations sheets, see the ***Structures Detailing Manual Examples***.

14.2 ACCURACY

Finish grade elevations are riding surface elevations (top of slab) on the bridge. To ensure accuracy, adhere to the following guidelines:

A. Space T-lines such that a linear interpolation midway between elevations does not deviate from the theoretical elevation by more than 0.005'. Use no less than ten equal spaces within any given span. For skewed straight bridges, T-lines may be either parallel to the skew or perpendicular to the bridge coping. For curved bridges, make T-lines radial. For curved bridges with a skew, use a combination of skewed T-lines and radial lines as appropriate.

B. Show these elevations to three decimal places.

14.3 FINISH GRADE ELEVATIONS

Tabulate finish grade elevations in a span-by-span or unit-by-unit fashion. These tables may be included on the Schematic sheet, Typical Section sheet or on a separate sheet. Finish grade elevations should be shown at the intersections of the following locations listed in section A and section B:

A. Along the length of the bridge:

   1. Begin/end of approach slabs.
   2. Half way point of approach slabs.
   3. Front Face of Backwall at begin/end bridge stations.
   4. Centerlines of Bearings.
   5. All T-Line locations.
   6. Centerlines of piers or intermediate bents.
   7. Along deck cut lines and/or construction joints.

B. Transversely:

1. Bridge coping.
2. Gutter line at traffic railings and traffic separators.
3. Back face of traffic railings for inboard mounted railings.
4. Inside of parapet or pedestrian railing.
5. The Station Line.
6. The PGL (if different from the Station Line). If there is more than one PGL, show elevations along each one.
7. Centerline of all beams or girders. For box beams or girders, show elevations at the centerline of each web.
8. Along deck cut lines.

14.4 FINISH GRADE ELEVATIONS - SCHEMATIC PLAN VIEW

At a minimum, show and label the following on the finish grade elevation plan view:

A. Begin/end approach slab.

B. Front Face of Backwall at begin/end bridge stations.

C. Centerlines of piers or intermediate bents.

D. All T-line locations. Label T-lines numerically.

E. Deck cut lines, construction joints and/or longitudinal joints.

F. Bridge copings.

G. All gutter lines, including traffic separators and pedestrian parapets.

14.5 FINISH GRADE ELEVATIONS - TYPICAL SECTION VIEW

The typical section views consist of sections through the bridge and approach slabs at locations showing all of the features which elevations will be given to. Clearly label all locations where elevations will be shown, cross slopes and working points for box girders. Use the exact labeling used in the Schematic Plan View.

15 SUPERSTRUCTURE - CONCRETE ELEMENTS

15.1 GENERAL

A. Superstructure plans will include all the details necessary to construct beams/girders, deck slabs, diaphragms, stay-in-place forms and all other related superstructure components.

B. For structures with prestressed concrete beams, include the Table of Beam Variables in the superstructure plans. The Table of Beam Variable can be found in the Structures Bar Menu.

C. Details on how prestressed beam standards are to be incorporated into bridge plans can be found in Volume 3 ***Instructions for Structures Related Design Standards***.

D. For structures with steel girders, see Chapter 16 for framing plan and girder detail requirements.

15.2 SUPERSTRUCTURE DRAWINGS - FRAMING PLAN

The Framing Plan shows a single, concise graphical representation of the geometry necessary for location and detailing beam framing. Include a Framing Plan for bridges with concrete beam superstructures having any of the following conditions:

1. Horizontally curved alignment
2. Skewed substructure or splayed beams
3. Intermediate diaphragms
4. Significant superelevation transition within a single span

Provide a framing plan showing at a minimum the following information:

A. The distances between intermediate diaphragms measured along the centerline of beams.

B. The distances between beams (centerlines or extensions) measured along the centerline of intermediate bents/piers or FFBW.

C. For straight beams supporting curved bridge decks, the chord lengths between the centerline of intermediate bents/piers or FFBW.

D. The distances from the Station Line to adjacent beams measured along the centerline of intermediate bents/piers or FFBW.

E. Angles between centerline of beams and the centerline of intermediate substructure or begin/end bridge line.

F. Dimension along both faces of beams measured from casting beam end to centerlines of diaphragm inserts.

G. Beam numbering including span and/or unit number.

H. Stationing and span lengths along the Station Line. Dimension spans to centerline of intermediate substructure or begin/end bridge line.

I. Angles between the Station Line and begin/end bridge line and centerline of intermediate substructure components or centerlines of bearing.

J. All dimensions to the nearest 1/8-inch and all angles to the nearest second.

K. The identification of beams on the Framing Plan, consistent with detail sheets.

L. Direction of stationing adjacent to the Station Line.

M. Stationing along the station line. Show stationing from left to right.

N. Stationing for Begin/End bridge and centerline of intermediate substructures. Indicate expansion or fixed bearings.

O. Span numbers. Number from left to right in the direction of stationing.

P. All centerlines of bearing.

Q. Angle between diaphragms and beams if not normal to the girder.

R. PC and PT locations and cross reference to horizontal curve data.

S. Longitudinal length along Station Line and copings. Show location and dimensions to PT/PC's. Show radii at copings of curved superstructures.

15.3 SUPERSTRUCTURE DRAWINGS - PLAN

The Superstructure Plan sheet shows deck reinforcement detailing information and should be worked with a detail sheet of the superstructure section. At a minimum, include the following in the Superstructure Plan sheet:

A. Plan view of the superstructure deck. Use a scale suitable for viewing the details shown on the sheet when printed. If more than one sheet is required, use appropriate matchlines.

B. Show bar callout and spacing. This can be indicated with the first and last bar in a group shown with a callout between the two.

C. Beam type and spacing. Show beam centerlines. Show flange edges and ends as dashed. Identify beam number.

D. Gutterline and coping.

E. Traffic railing joints as per the Design Standard. Indicate type of joint and spacing. Reference the applicable Design Standard.

F. Dimension of overall width, roadway width, median location, length of spans, and overall length of bridge.

G. Phase construction limits. Show dimensions and indicate which phase each portion of the superstructure is to be constructed.

H. Beams. Show beam centerlines. Show flange edges and ends as dashed. Identify beam number. This is not necessary when a framing plan is included in the plans.

I. Outlines of diaphragms with dimensions.

J. Underdeck lighting.

K. Light pole pilaster. Reference the applicable Design Standard.

L. Scuppers and deck drains.

M. Expansion joint blockouts.

N. Traffic railing longitudinal steel. Reference applicable traffic railing Design Standard.

O. Quantity block. Include breakdown for each phase of bridge for phased construction. (Note: Structural steel quantity is not to be shown.)

P. Pouring sequence. Indicate direction of pour. This can be shown on a separate sheet if necessary.

15.4 SUPERSTRUCTURE DRAWINGS - SECTION

At a minimum, include the following in the Superstructure Section sheet:

A. Section view of the superstructure. Detail all reinforcement visible in this view. Partial sections are discouraged. Include section at midspan and at bent/pier to show intermediate diaphragm and end diaphragm details respectively.

B. Deck thickness and cross slope.

C. Connection between existing and proposed slabs for bridge widenings. Indicate which phase of construction the superstructure will be constructed.

D. Vertical traffic railing reinforcement to be cast into deck. Detail longitudinal slab steel to tie in with vertical traffic railing reinforcement. Reference applicable traffic railing Design Standard for longitudinal reinforcement requirements. For bridges with a raised median traffic separator, see Index 302 for connection options.

E. Traffic separator. Show as dashed and reference the appropriate ***Design Standard*** for connection details and options.

F. Show V-groove drip edge at slab coping.

G. Beam/girder spacing. Dimension deck overhang.

H. Longitudinal closure pour locations. Include dimension(s) to a known point of reference such as a beam line, gutter line, etc.

I. Depth of girder haunch for steel girders. Do not show a dimension for prestressed beam build-up.

15.5 SUPERSTRUCTURE DRAWINGS - CONCRETE DIAPHRAGMS

At a minimum, include the following in the Diaphragm sheet:

A. Sections through diaphragms at piers and end bents. Detail reinforcement, expansion joints, construction joints and control joints. Include compressible material (such as two or more layers of 30lb felt paper) between adjacent diaphragms at interior supports. Show dimension between adjacent diaphragms at expansion joints. Coordinate with expansion joint details.

B. Section through intermediate diaphragms. Detail reinforcement to avoid conflicts with slab steel.

C. Jacking locations. Show jacking service loads in tabular format.

15.6 SUPERSTRUCTURE DRAWINGS - DETAILS

The Superstructure Details sheets show detailing information required to construct the superstructure. This will include reinforcement details, form placement details, expansion joint information and other ancillary details such as utility hangers, traffic signals and under deck lighting. For an example illustrating the content and format of a completed Superstructure Detail sheet, see the ***Structures Detailing Manual Examples***. At a minimum, include the following in the Superstructure Details sheets:

A. Section views of the superstructure as required. Detail all reinforcement visible in this view.

B. Utility hanger details. Include details showing utility passing through diaphragms. Show expansion sleeve details where required.

C. Expansion joint details.

D. Stay-in-place form details. See Figure 15.9-1, Figure 15.10-2, Figure 15.10-3, Figure 15.10-4 and Figure 15.10-5.

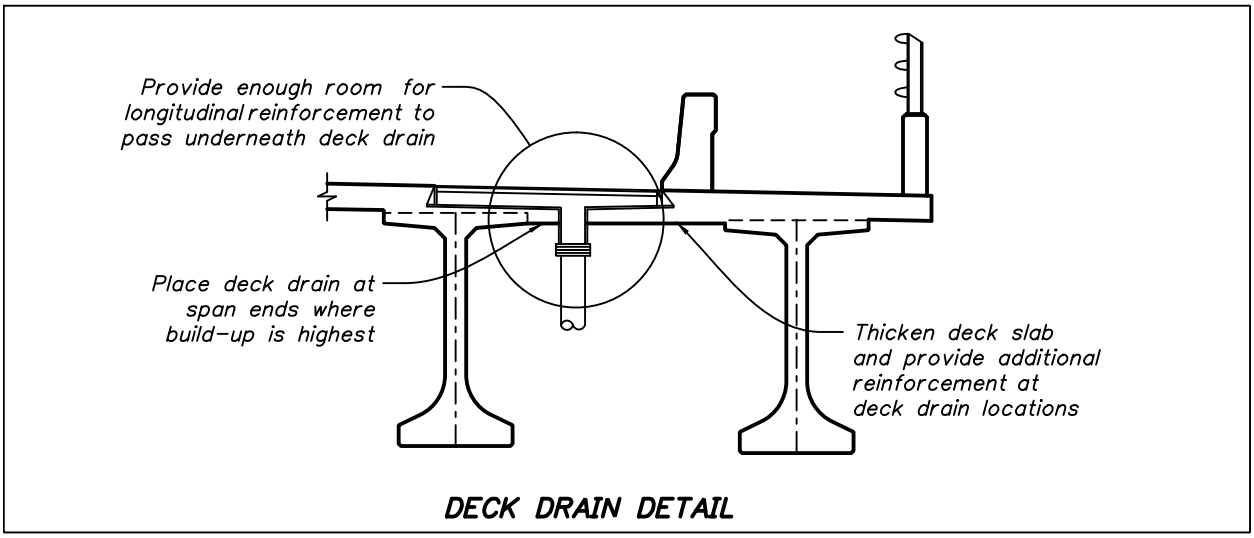
15.7 DECK DRAINS

It is preferable to avoid the use of deck drains on bridges. In situations where the use of deck drains cannot be avoided, take into account the following design considerations:

A. Open deck drains (scuppers) must not discharge directly on the supporting beams or girders, substructure embankments at end bents, lower roadways and sidewalks, or other areas (water or land) where not permitted. Show pipes drained to a location recommended by the State/District Drainage Engineer.

B. Locate required deck drain near pier supports and, when practical, use a single drain sized to drain the entire span. Provide a flexible or semi-rigid connection with the drain that accommodates differential movement between the superstructure and substructure. See Figure 15.7-1.

Figure 15.7-1 Deck Drain Detail



C. Additional reinforcement is required in the bridge deck at closed deck drain locations. Detail additional reinforcing around drains. The reinforcing requirement is dependent on the drain size and beam spacing.

D. Detail the closed drain systems utilizing deck drains that are prefabricated steel drain boxes with anchor studs, hot dip galvanized after fabrication with removable grates, welded steel plates and bars with anchors. Design closed drains for wheel loads or pedestrians (ADA compliant) based on their locations on the bridge deck.

E. Use PVC, Schedule 80 UV-Resistant or fiberglass pipe, encased in the pier concrete. Provide a minimum of one cleanout located at the top of the structure and one cleanout located at or near the ground lateral.

F. A note allowing specific alternate ferrous castings in lieu of prefabricated steel drain boxes may be included. Do not list proprietary names of items or vendor names.

15.8 SUPERSTRUCTURE DESIGN CONSIDERATIONS - GENERAL

The bridge superstructure requires a considerable amount of design and detailing to be easily constructed. The following design considerations should be incorporated to produce a more constructible, economical design:

A. Maximize the clear distance between mats of steel by using No. 5 rebar and tightening spacing, if possible. This must be balanced with reinforcement spacing minimum requirements.

B. For cross sections with a slope break point, do not specify a straight bar in the bar list with an indication to field bend as necessary. Instead, calculate the angle and call for the appropriate bar type in the bar list.

C. Design bridge deck reinforcement so that transverse main steel is on the exterior of each mat. Distribution steel should be on the interior of each mat.

D. Detail closure pours where required considering differential deflection between phases/adjacent beams. Closure pours should be a minimum of 2 feet wide.

15.9 SUPERSTRUCTURE DESIGN CONSIDERATIONS - PRESTRESSED BEAMS

In addition to the applicable sections above, the following issues should be addressed when designing and detailing superstructures supported by prestressed concrete beams:

A. Avoid conflicts between diaphragm reinforcement and slab reinforcement. Detail these areas in the plans and on the bar list to ensure that there are no conflicts.

B. To facilitate jacking procedures, detail the bottom of end diaphragms as flat from corner to corner of adjacent beams, i.e. no steps or haunches. Generally, end diaphragms should be 12" thick. See Figure 15.9-1.

C. Add a note when intermediate diaphragms are required to alert fabricator to see Framing Plan sheet(s) for insert spacing and layout.

D. Deck pour sequencing details will be required for decks supported by continuous beams or for large decks that cannot reasonably be poured monolithically. Show positive regions poured first and locate construction joints at points of dead load moment contraflexure.

E. Show stay-in-place (S.I.P) form details. See Figure 15.9-2 for S.I.P. details related to prestressed-beam superstructures.

Figure 15.9-1 Concrete Diaphragm Detail

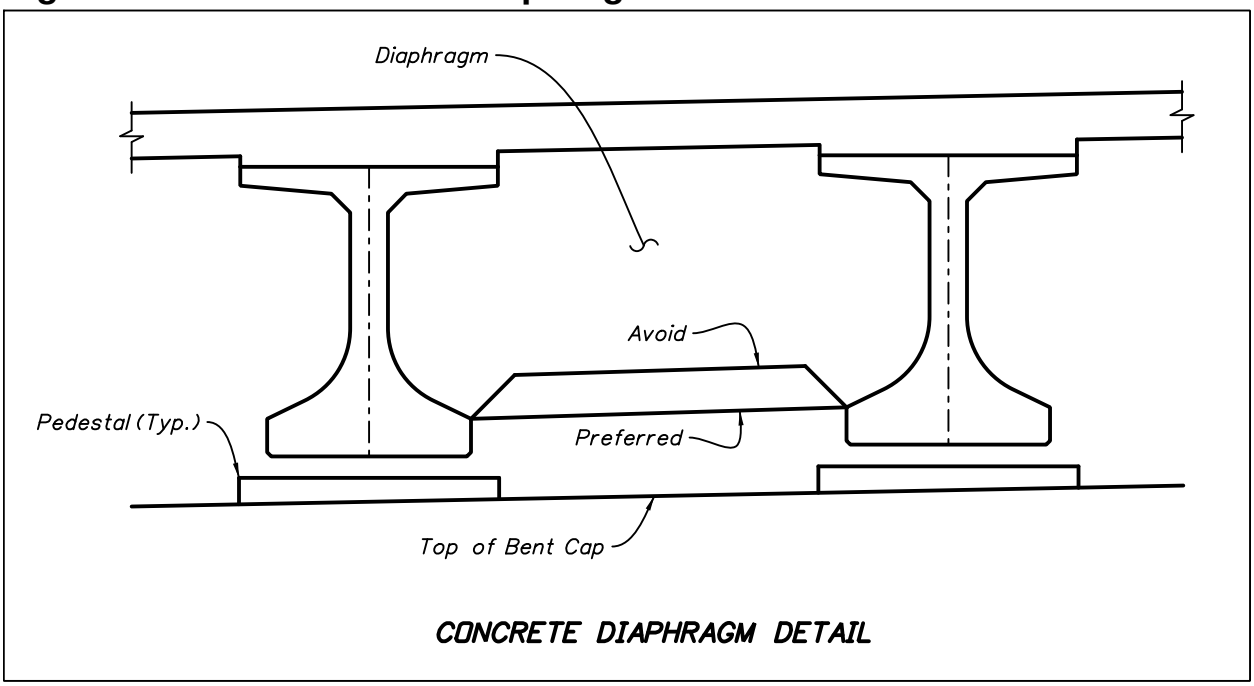
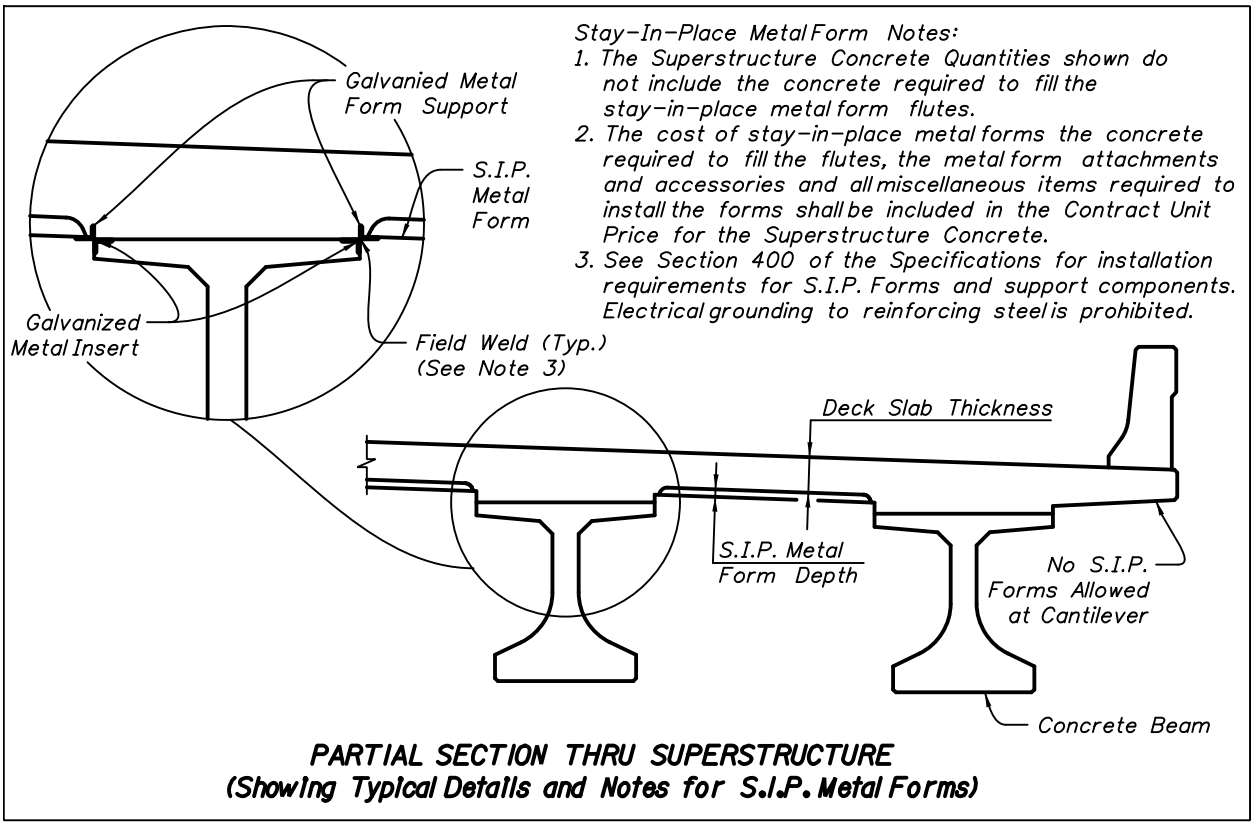


Figure 15.9-2 S.I.P. Form Details for Prestressed Concrete Beams

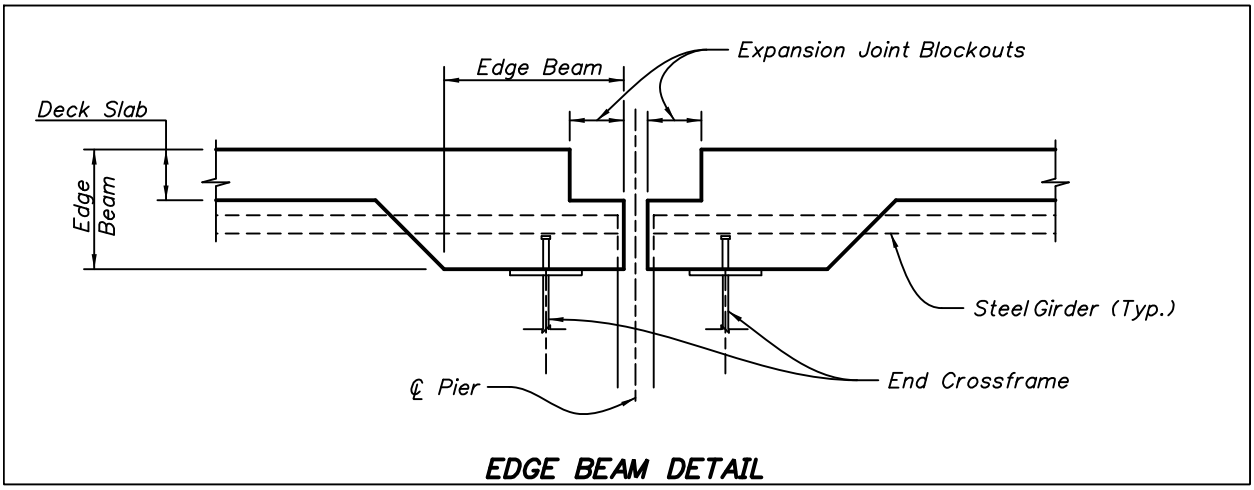


15.10 SUPERSTRUCTURE DESIGN CONSIDERATIONS - STEEL GIRDERS

In addition to the applicable sections above, the following issues should be addressed when designing and detailing superstructures supported by steel girders:

A. Design edge beam at ends of spans or continuous units to transmit wheel loads into end crossframe. See Figure 15.10-1.

Figure 15.10-1 Edge Beam Detail



B. Detail superstructure with a constant-depth haunch. This depth, in conjunction with the cross slope, must be taken into account when designing shear stud height. Show the dimension from the top of web to the top of slab in the Superstructure Details.

C. Show S.I.P form details. See Figure 15.10-2 and Figure 15.10-3 for I-girder S.I.P. form details and Figure 15.10-4 and Figure 15.10-5 for box girder S.I.P. form details. Stay-in-place forms are required for the interior portion of box girders.

Figure 15.10-2 S.I.P. Form Details for Steel I-Girders (Section)

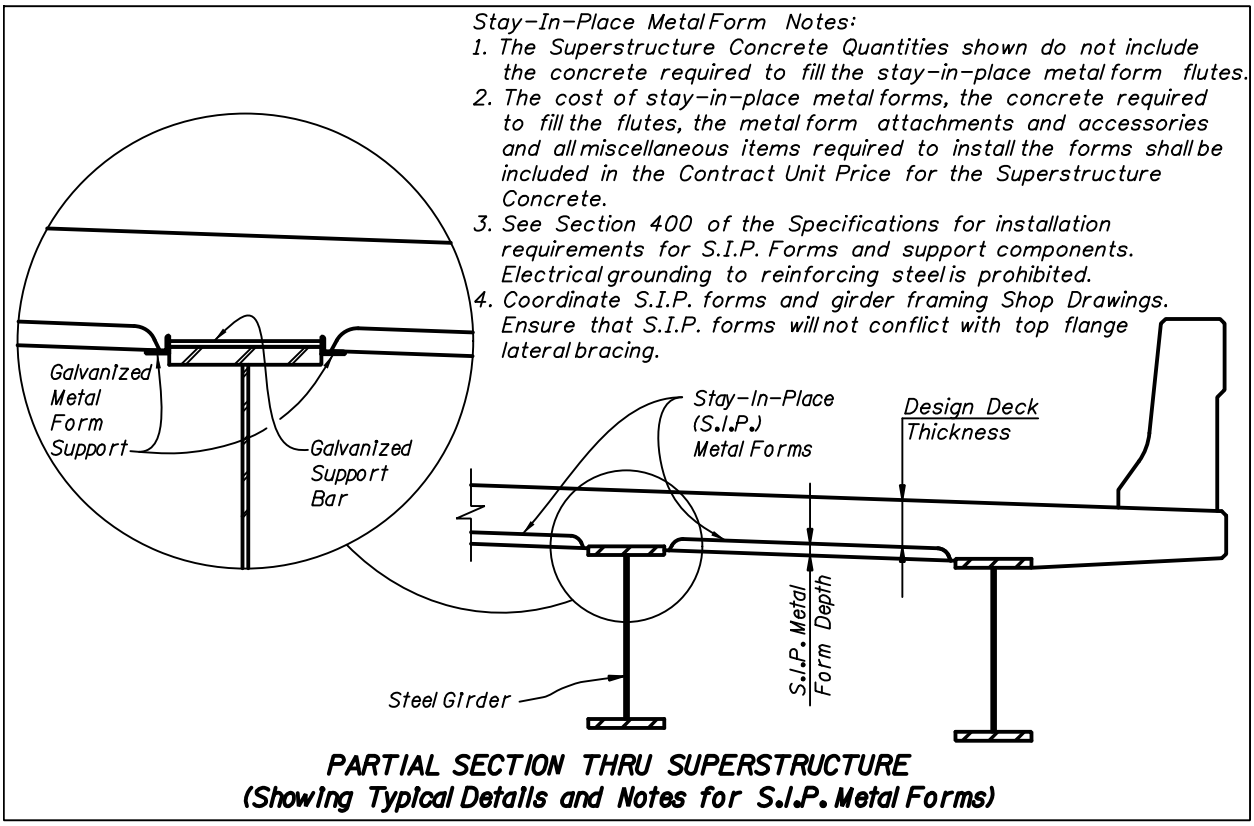


Figure 15.10-3 S.I.P. Form Details for Steel I-Girders (End Detail)

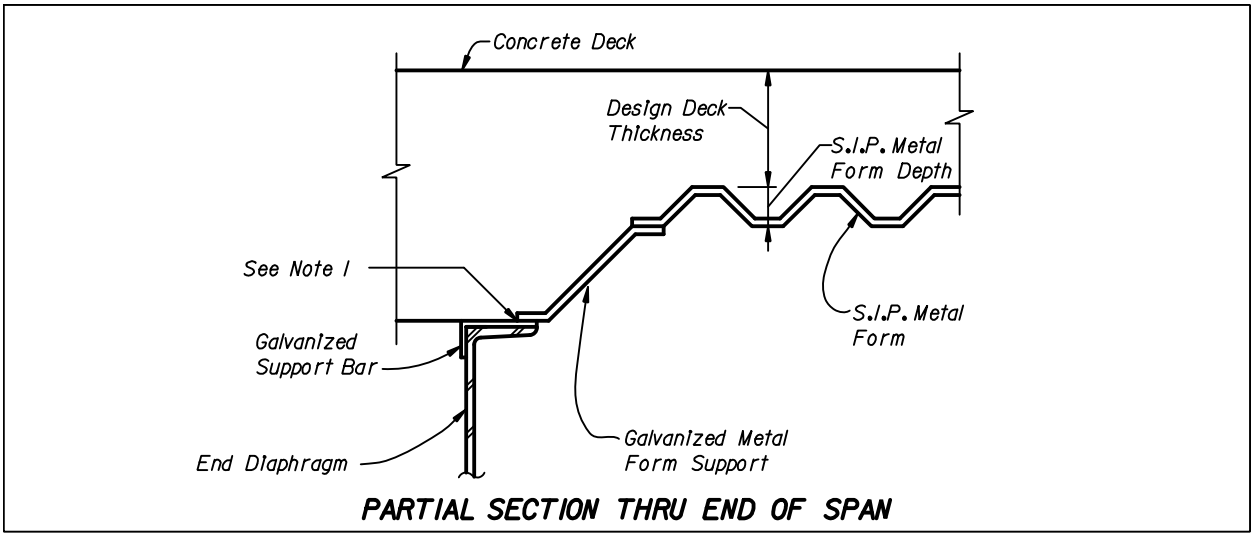
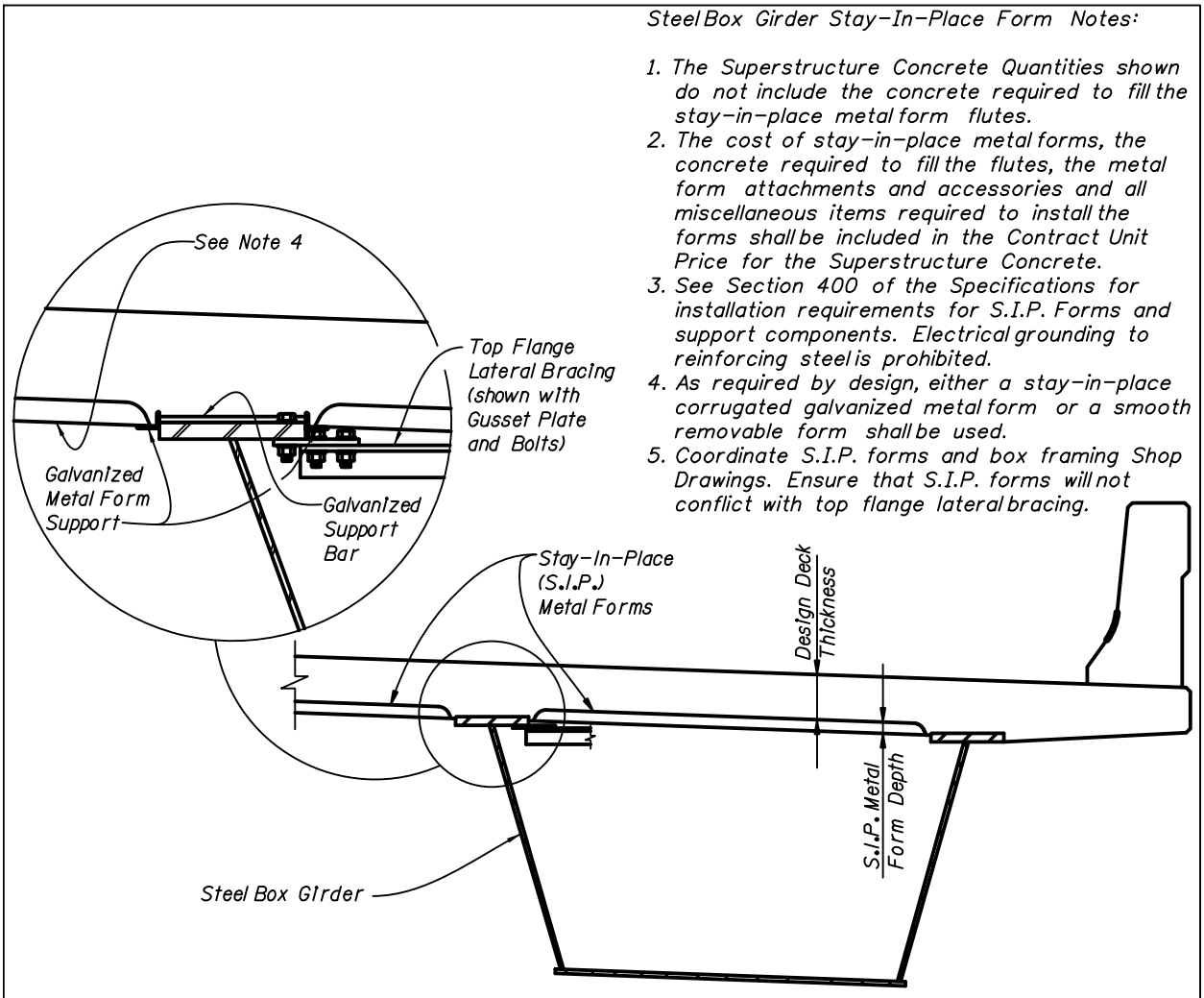
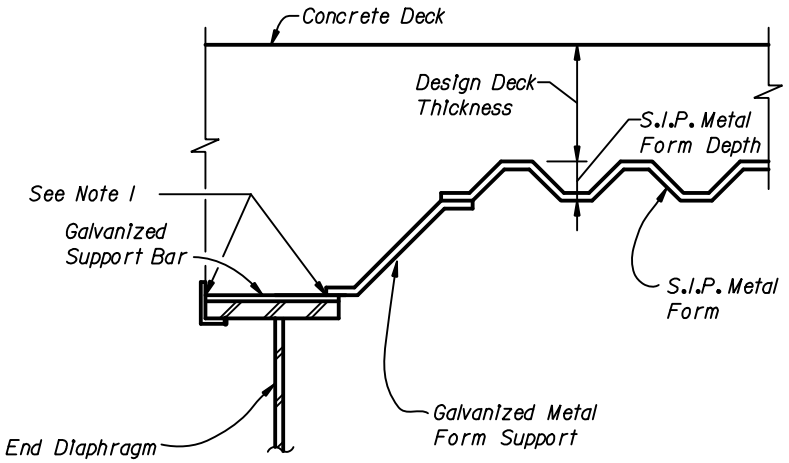


Figure 15.10-4 S.I.P. Form Details for Steel Box Girders (Section)



PARTIAL SECTION THRU SUPERSTRUCTURE
(Showing Typical Details and Notes for S.I.P. Metal Forms)

Figure 15.10-5 S.I.P. Form Details for Steel Box Girders (End Detail)



PARTIAL SECTION THRU END OF SPAN

16 STRUCTURAL STEEL GIRDERS

16.1 GENERAL

A. Structural steel drawings and details will be used by fabricators and contractors for the production and erection of structural steel members. This chapter does not cover movable bridges.

B. Refer to the following ***AASHTO/NSBA*** Steel Collaboration Standards (available at http://www.steelbridges.org/) with exceptions as detailed in this chapter:

1. G 1.2 - 2003 Design Drawing Presentation Guidelines
2. G 1.4 - 2006 Guidelines for Design Details
3. G 12.1 - 2003 Guidelines for Design for Constructability

C. Check AISC on-line database of available structural steel shapes before specifying a particular steel shape and size. Preference should be given to shapes and sizes with multiple producers due to increased availability and lower cost.

16.2 SHOP DRAWINGS

Refer to the following ***AASHTO/NSBA*** Steel Bridge Collaboration Standards for guidance on review and approval of steel shop drawings (available at http://www.steelbridges.org/):

1. G 1.1 - 2000 Shop Detail Drawing Review/Approval Guidelines
2. G 1.3 - 2002 Shop Detail Drawing Presentation Guidelines
3. ***Plans Preparation Manual (PPM)*** - Volume 1, Chapter 28
4. ***FDOT Standard Specifications for Road and Bridge Construction***- Section 5

16.3 FRAMING PLAN DRAWINGS AND DETAILS - STEEL I-GIRDERS

Framing plans are required for all bridges supported by a steel superstructure. For examples illustrating the content and format of completed Framing Plan sheets, see the ***Structures Detailing Manual Examples***. Provide a framing plan for steel I-girder superstructures showing the following information (see Figure 16.3-1, Figure 16.3-2, Figure 16.3-3 and Figure 16.3-4):

A. Transverse stiffener spacing. Show transverse stiffeners on one side of girder only. Show on inside of exterior girders.

B. Lateral bracing spacing.

C. The distances between girders (centerlines or extensions) measured along the centerline of bearing.

D. The distances from the Station Line to adjacent girders measured along the centerline of bearing

E. Dimension to field splices along centerline girder from centerlines of bearing. Number field splices from left to right. Clearly label optional field splices.

F. Girder radius of curvature for each girder, tabulate if necessary for clarity.

G. Crossframe location. Indicate type of crossframe.

H. Temporary bracing required for construction.

I. Distance from centerline bearing to the FFBW.
J. Girder numbering including span and unit number.
K. Distance between centerlines of bearing.
L. Location of PC/PT along centerline of girders.
M. Direction of stationing.

Figure 16.3-1 I-Girder Framing Plan

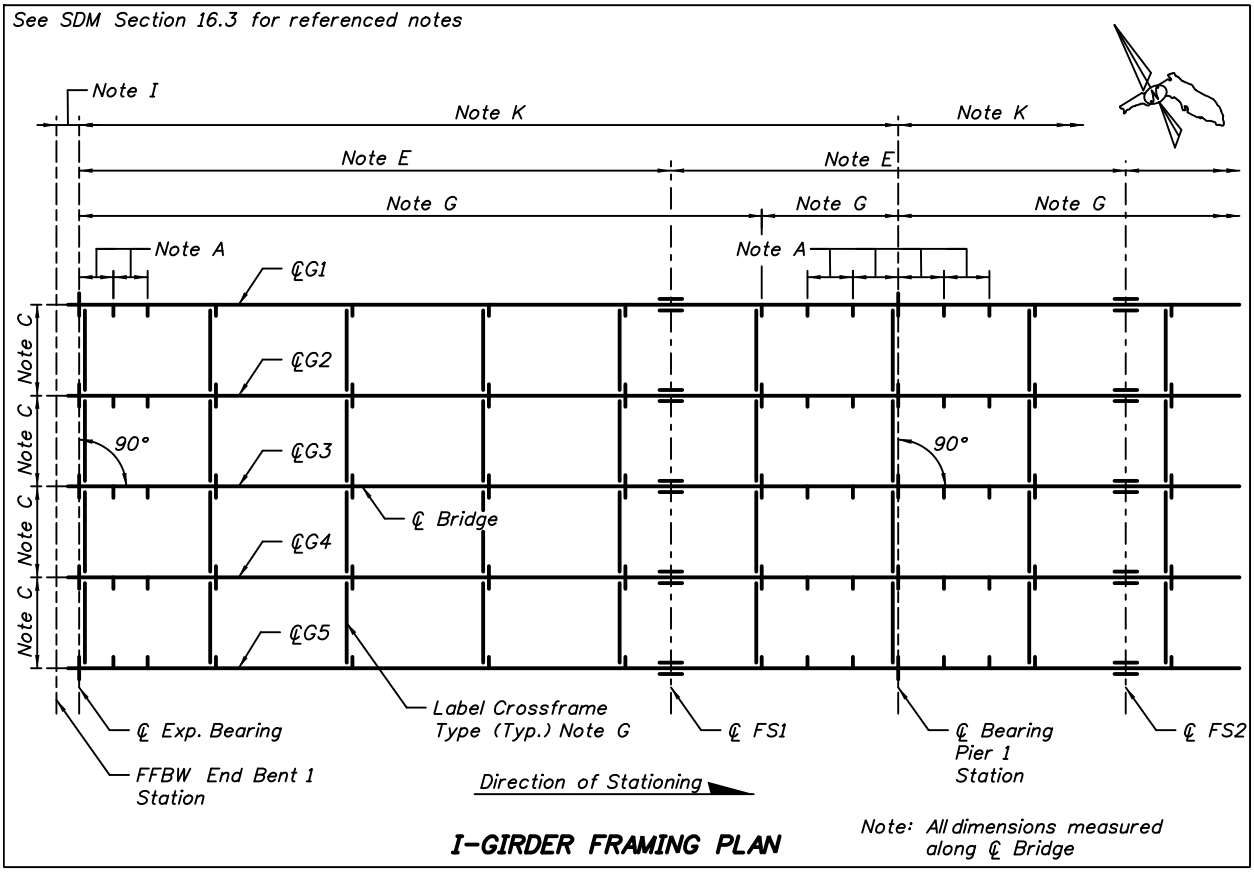


Figure 16.3-2 Framing Plan Details - Large Skews

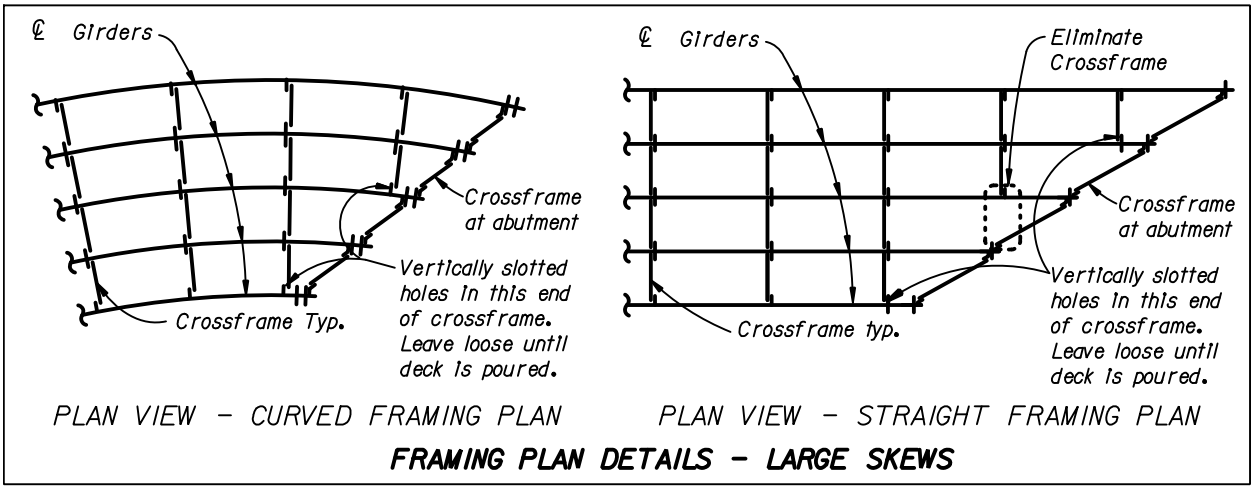


Figure 16.3-3 Framing Plan at Gore Area

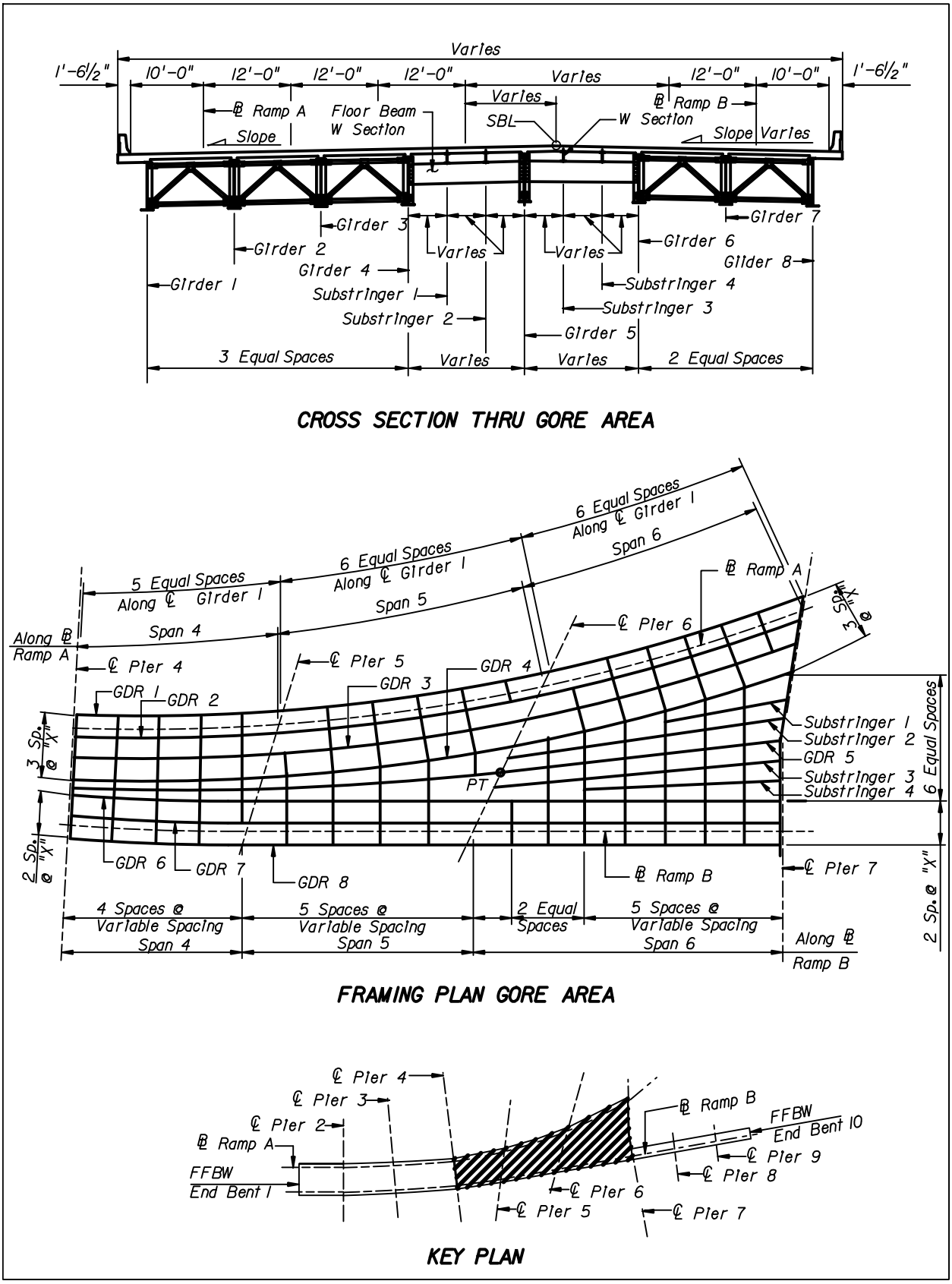
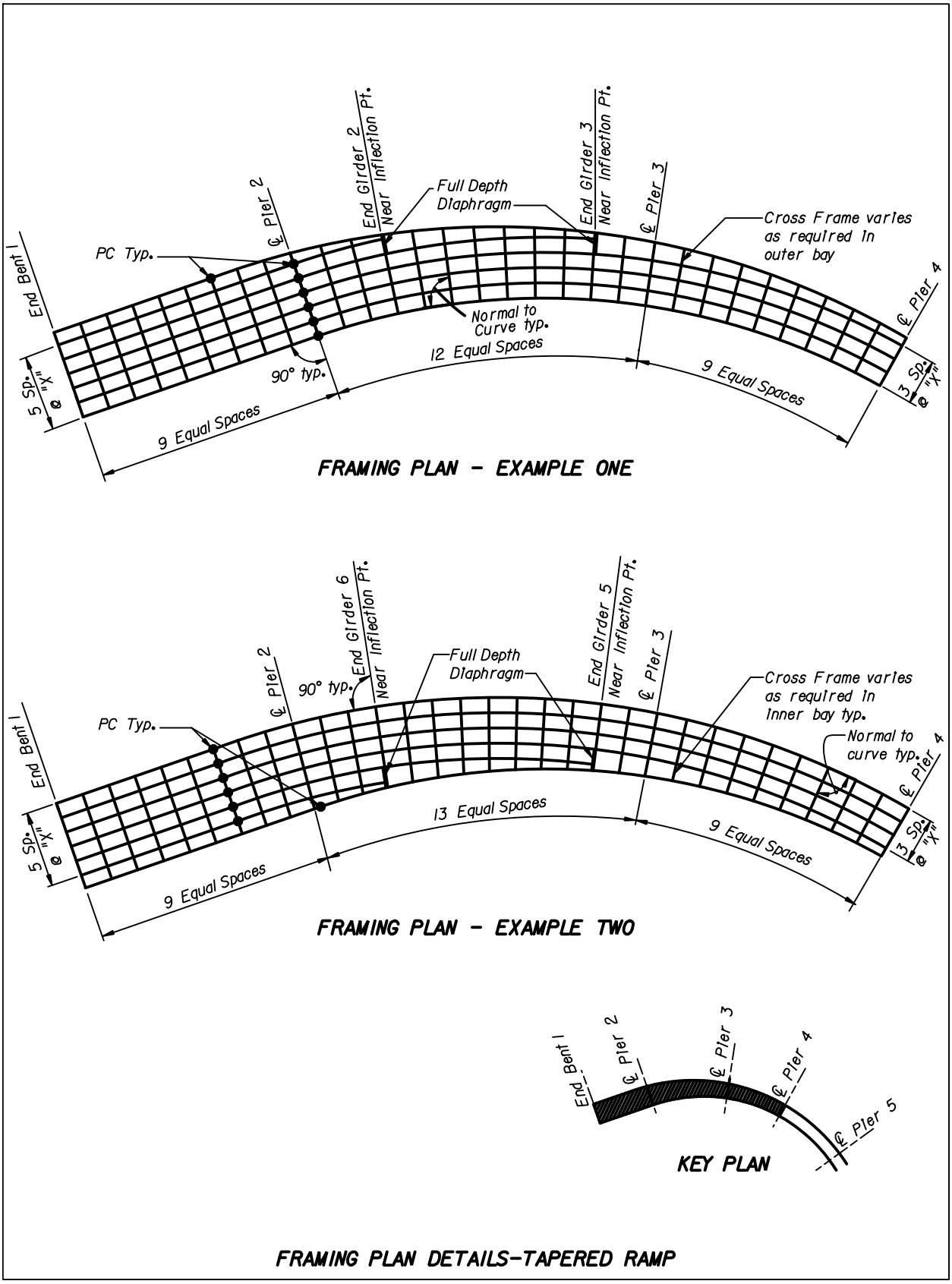


Figure 16.3-4 Framing Plan at Tapered Ramp



16.4 FRAMING PLAN DRAWINGS AND DETAILS - STEEL BOX GIRDERS

Provide a framing plan for steel box superstructures showing the following information (see Figure 16.4-1):

A. All applicable information shown in ***SDM*** 16.3.

B. Access opening location and spacing.

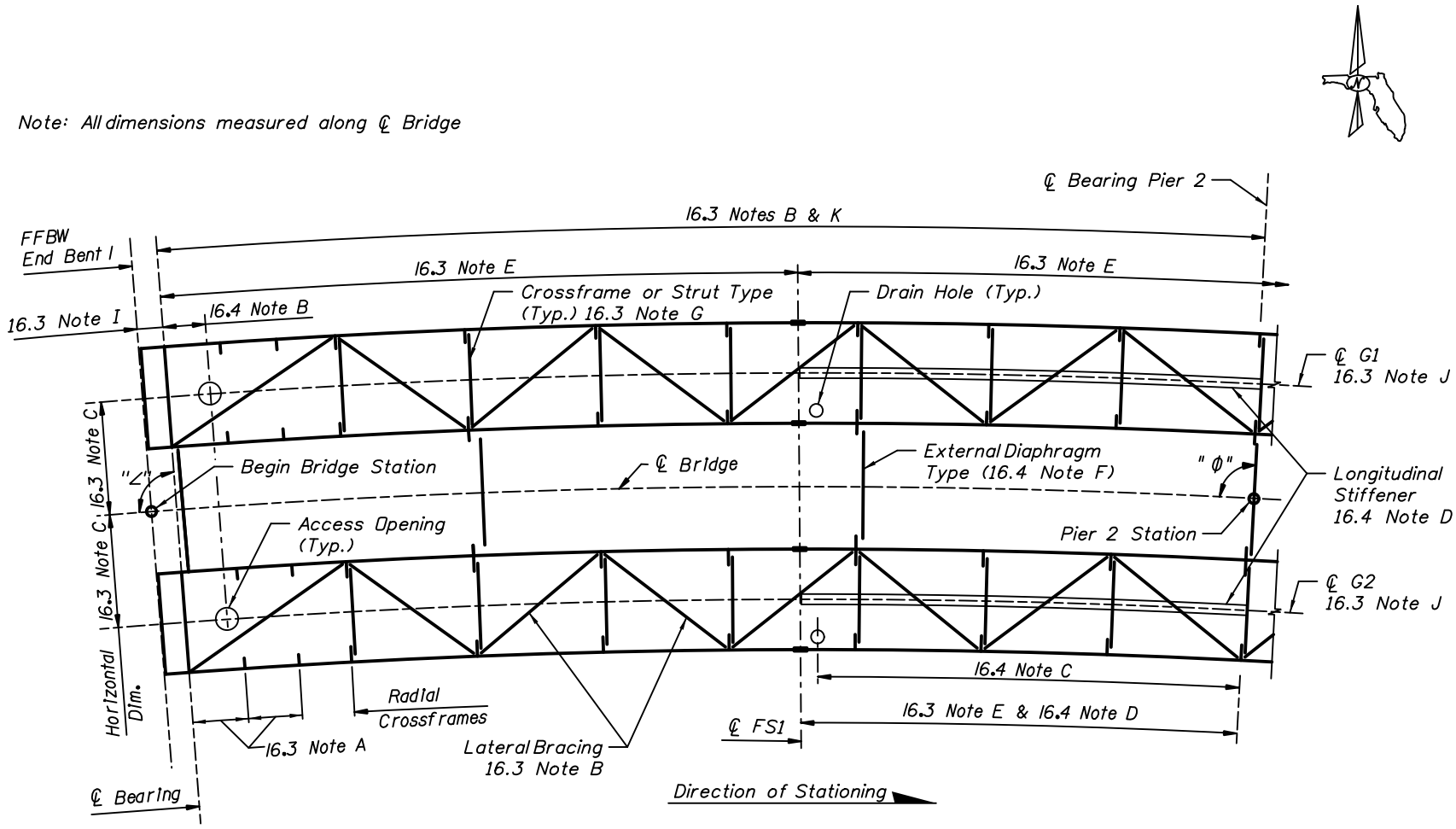
C. Drain hole location and spacing.

D. Show limits of longitudinal stiffeners.

E. In plan view, show centerline of top of web. (Do not show width of top flange.)

F. Permanent/temporary external diaphragm/crossframe locations.

Figure 16.4-1 Curved Box Girder Framing Plan



CURVED BOX GIRDER FRAMING PLAN

16.5 GIRDER ELEVATION

Girder Elevation sheets are required on all bridges utilizing a steel superstructure. The detailer may include the girder elevation on the Framing Plan, but this may be a separate sheet. Much of the information presented here can be shown in tabular format, as required. Detail girder elevation upstation left to right. At a minimum, include the following on the Girder Elevation sheet:

A. Elevation view of girder. Vertical scale may be exaggerated for clarity. Provide suitable matchlines for girders that require more than one sheet.

B. Shear connector spacing along centerline of girder (centerline of box for box girders).

C. Flange plate sizes.

D. Web plate size.

E. Weld sizes and types. Reference welding symbols at www.aws.org.

F. Field splices. Number splices sequentially left to right. Designate optional splices as required.

G. Plates to be Charpy V-Notch (CVN) tested (use legend). Indicate where Charpy V-Notch testing is required along top and bottom flange dimension lines or by indicating web locations with note/arrow. Indicate Fracture Critical Members by notation (FCM).

H. Dimensions for length along centerline girder as follows:

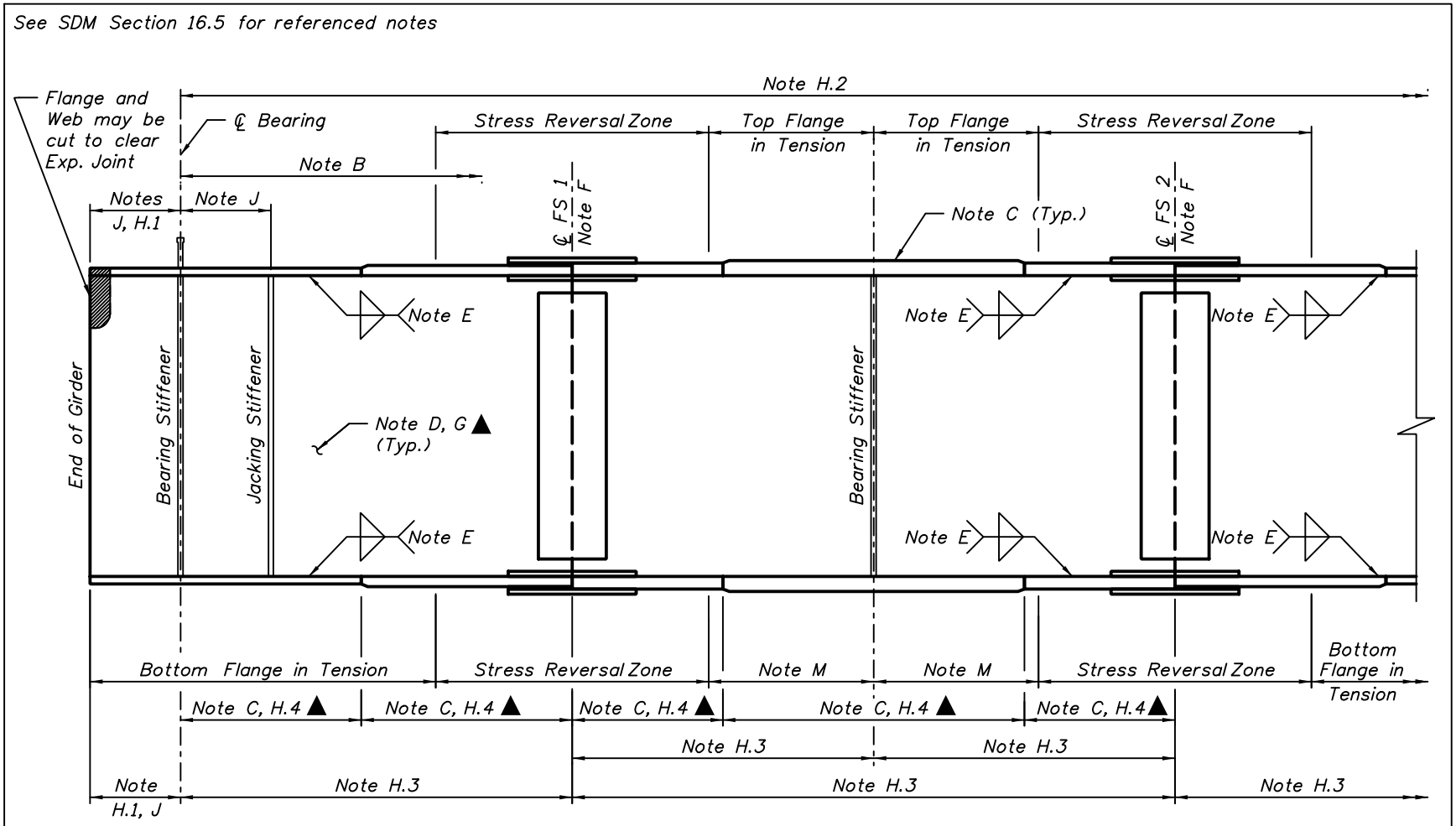
   1. From girder end to centerline of bearing at the end of the unit.
   2. Between centerline piers and/or centerline of bearing at the end of the unit.
   3. Between centerline(s) of bearing and field splice(s).
   4. Girder section changes. Show this dimension for top flange, bottom flange or web section changes.
   5. Limits of flange tension and stress reversal zones.

I. Bearing stiffener and jacking stiffener spacing from girder end and/or centerline bearing at intermediate supports. Bearing and jacking stiffeners should be spaced not less than 8" apart.

J. Show and dimension any penetrations in the web for drainage pipes, post tensioning, sign/signal attachments, etc.

K. Material designation requirements for hybrid designs.

Figure 16.5-1 Typical Girder Elevation



TYPICAL GIRDER ELEVATION

16.6 I-GIRDER STIFFENER DETAILS

Stiffener Detail sheets should include details for fabrication and placement of bearing/ jacking stiffeners, intermediate stiffeners and crossframe connection plates. At a minimum, include the following on the Stiffener Details sheet (see Figure 16.6-1, Figure 16.6-2,Figure 16.6-3 and Figure 16.6-4):

A. Section of girder or diaphragm showing stiffener plate dimensions. Show enough sections to adequately address all stiffener scenarios. Do not double label a single section.

B. Plate sizes and dimensions for all stiffeners and connection plates. Size stiffeners taking into account bolt patterns and associated tolerances.

C. Weld type and dimension. Use appropriate welding symbols.

D. End conditions for stiffeners. Use "Finish to Bear" at the bottom of jacking and bearing stiffeners only.

E. Corner clip dimensions. Generally, a clip height of 3" is preferred. Maintain the same clip dimensions throughout.

F. Tab plate and cut back details.

G. Weld termination detail.

H. Show jacking and bearing stiffeners normal to bottom flange for vertical grades up to 4%. All connection plates and intermediate stiffeners should be detailed normal to web.

I. Intermediate stiffeners on one side only and inside only for exterior girders.

Figure 16.6-1 Weld Termination Detail

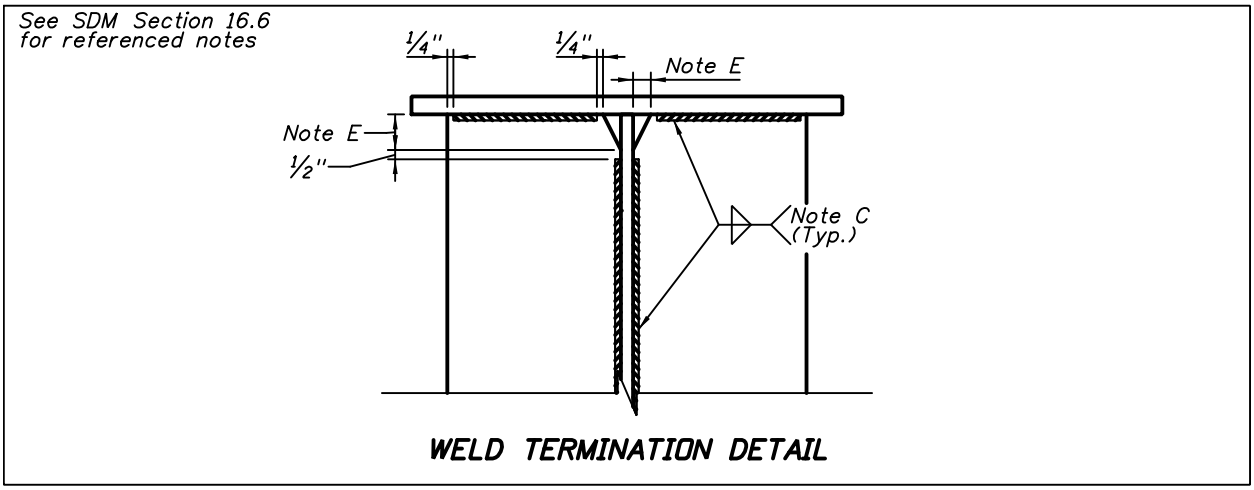


Figure 16.6-2 Tab Plate Detail

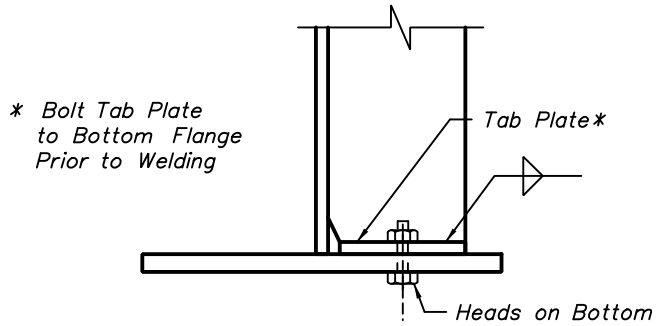


Figure 16.6-3 Standard Clip Options

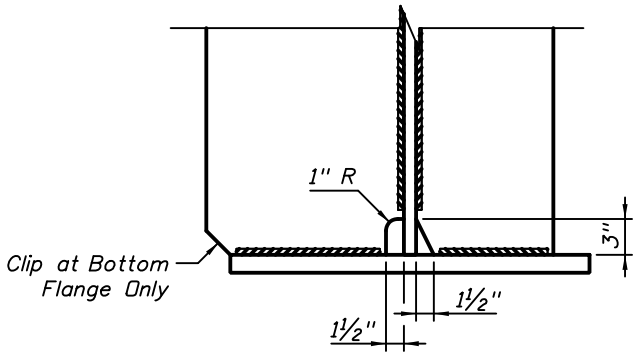
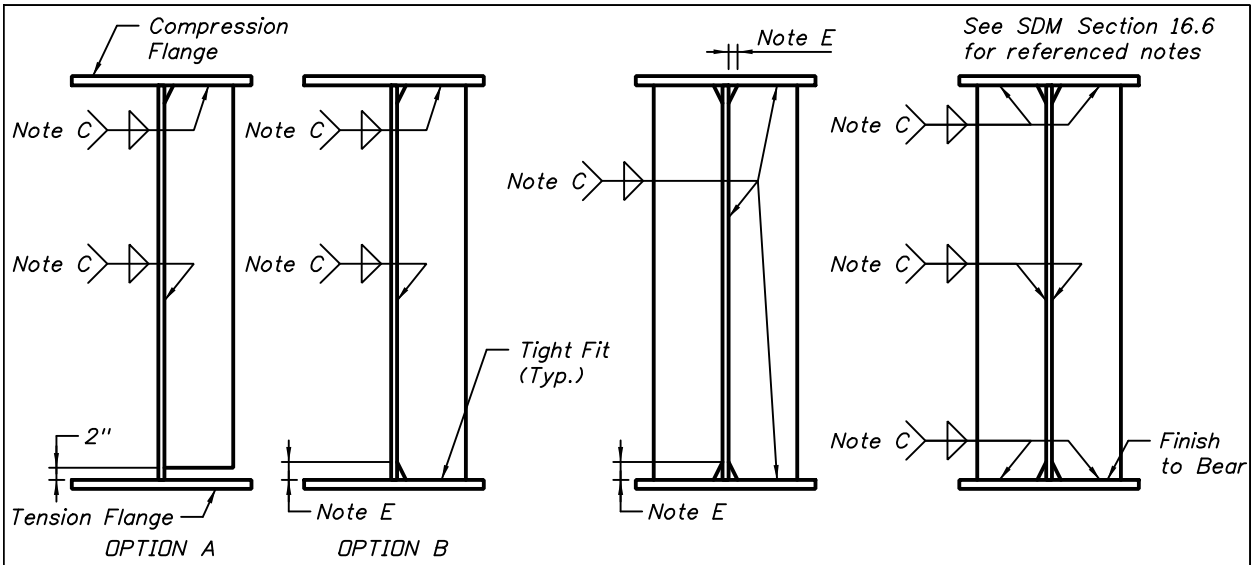


Figure 16.6-4 Girder Details



16.7 CROSSFRAME DETAILS - I-GIRDERS

Crossframe Detail sheets should include details for fabrication and placement of intermediate and end crossframes. At a minimum, include the following on the Crossframe Details sheet (see Figure 16.7-1 and Figure 16.7-2):

A. Partial section of superstructure showing typical bay. Show at least one section for each type of end crossframe and intermediate crossframes.

B. Bolt layout and spacing. Maintain minimum edge distances taking tolerances into account. Detail bolt patterns normal to girders.

C. Plate thickness for gusset plates. Keep plates rectangular.

D. Member sizes (angles, C-channels, WT-sections, etc.). Use standard shapes for all members. Show angles with horizontal leg on the upper side of member.

E. Shear connector spacing for end crossframes. Be aware of spaces required to bolt-up end crossframes.

F. Weld detail with dimensions. Weld termination details at all gusset and connection plates. Generally, ¼" fillet welds are preferable. Terminate welds ½" from edge.

G. Bolt hole sizes when oversized or slotted holes are required. Field-drilled connections are preferred at closure pour bays.

H. Partial plan view of crossframe showing bent plates at skewed supports.

I. Details for temporary crossframes.

J. Deck drainage piping conflict details.

K. Dimension between top of web and top of top chord of crossframe (channel for end crossframes).

Figure 16.7-1 Typical I-Girder Crossframe Details

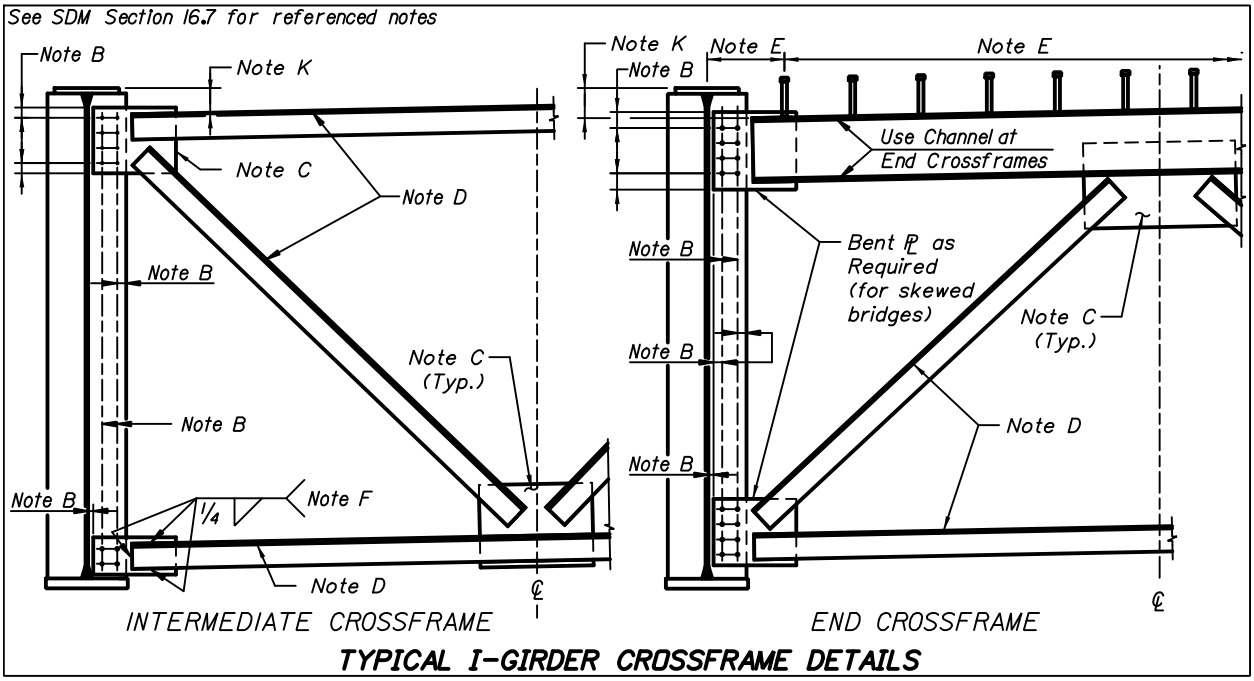
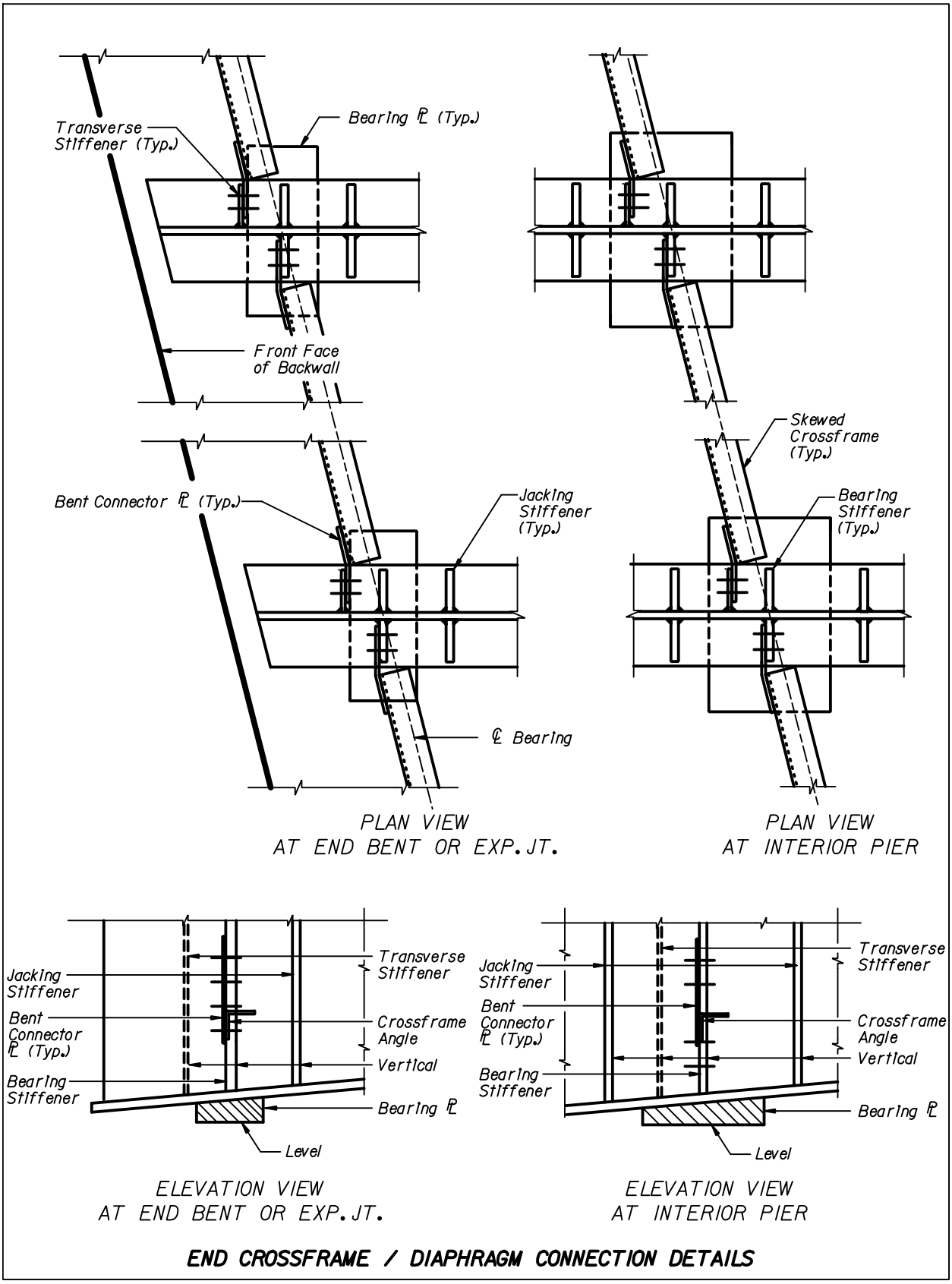


Figure 16.7-2 End Crossframe / Diaphragm Connection Details



16.8 CROSSFRAME & DIAPHRAGM DETAILS - BOX GIRDERS

Crossframe and Diaphragm Detail sheets should include details for fabrication and placement of internal crossframes and diaphragms in box girders. This includes details for internal crossframes, internal lateral bracing, end diaphragms, external diaphragms and external crossframes. At a minimum, include the following on these detail sheets (see Figure 16.8-1, Figure 16.8-2, Figure 16.8-3, Figure 16.8-4 and Figure 16.8-5):

A. Partial section of superstructure showing typical bay. Show at least one section each for all crossframes and diaphragms, internal and external.

B. Bolt layout and spacing dimensions. Maintain minimum edge distances taking tolerances into account. Detail bolt patterns normal to girder web.

C. Plate thickness for gusset plates.

D. Member sizes (angles, C-channels, WT-sections, etc.). Use standard shapes for all members. Show angles with horizontal leg on the upper side of member.

E. Shear connector spacing for end diaphragms. Be aware of spaces required to bolt-up external end crossframes.

F. Weld detail with dimensions. Weld termination details at all gusset and connection plates. Generally, ¼" fillet welds are preferable.

G. Top flange lateral bracing for box girders. The use of lateral gusset plates is permitted to aide bracing fit-up. Use fill plates where required to avoid conflict with formwork or diaphragm top flange. Include a plan view of lateral bracing connection details.

H. Tab plate details. Include weld sizes and termination details.

I. Details for temporary diaphragms.

J. Diaphragm details.

   1. Plate sizes.
   2. Location and sizes of stiffeners. Detail stiffeners normal to box bottom flange and vertical.
   3. Locations of access holes and utility holes.
   4. Cut out dimensions and connection details for longitudinal stiffeners.
   5. All applicable details listed above.

Figure 16.8-1 Steel Box Girder Cross Section Basic Geometry

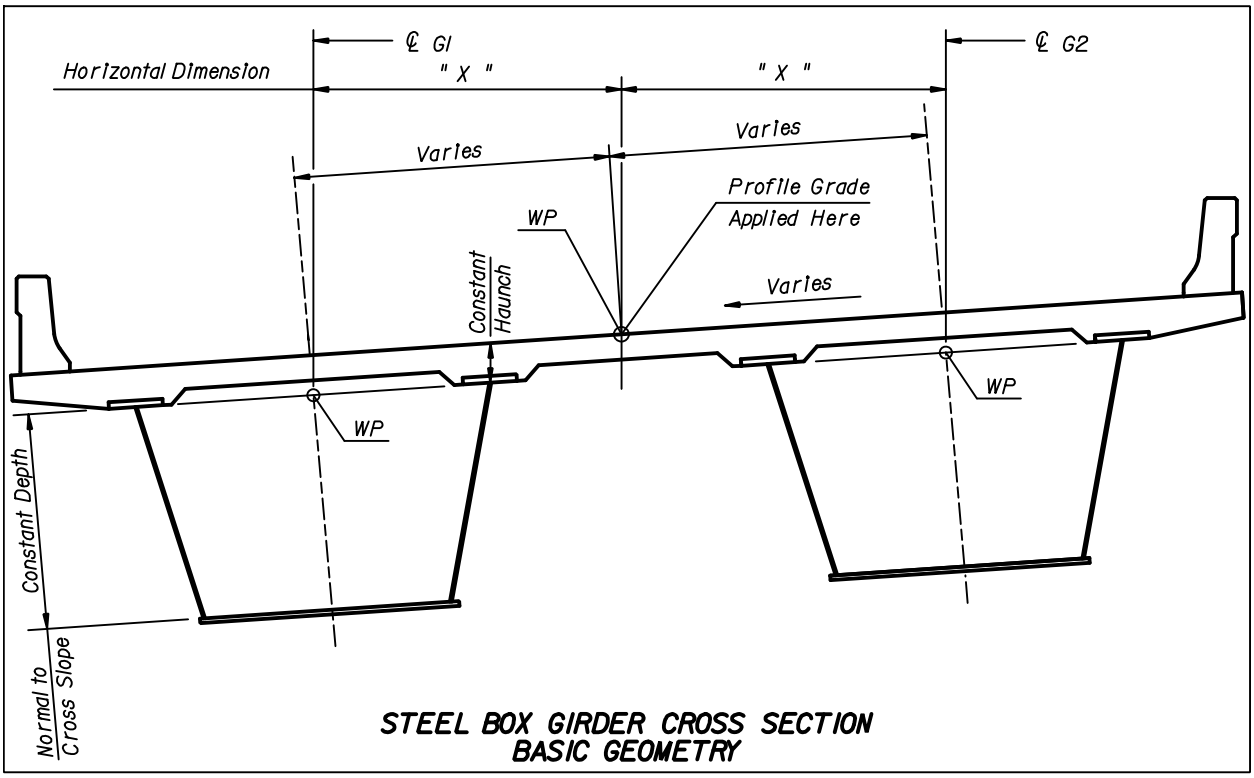


Figure 16.8-2 Steel Box Girder Pier Diaphragms

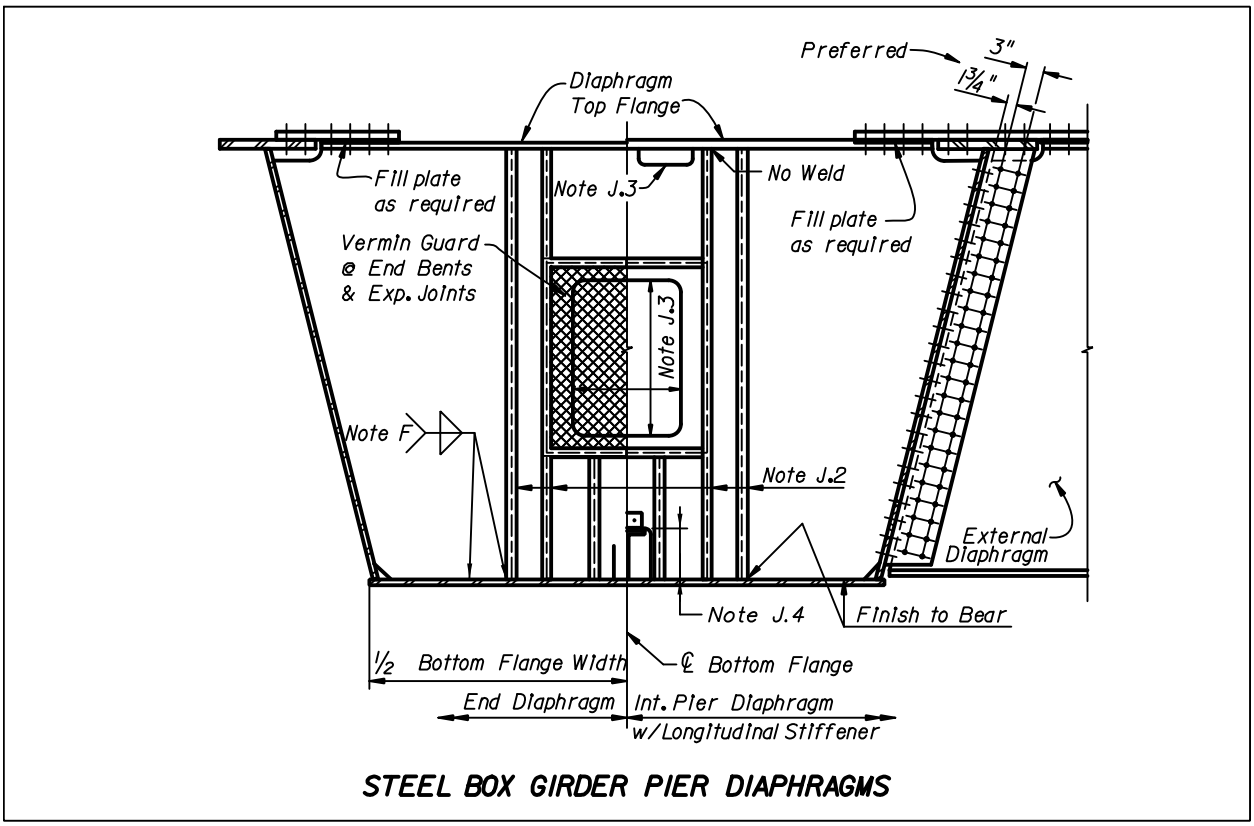
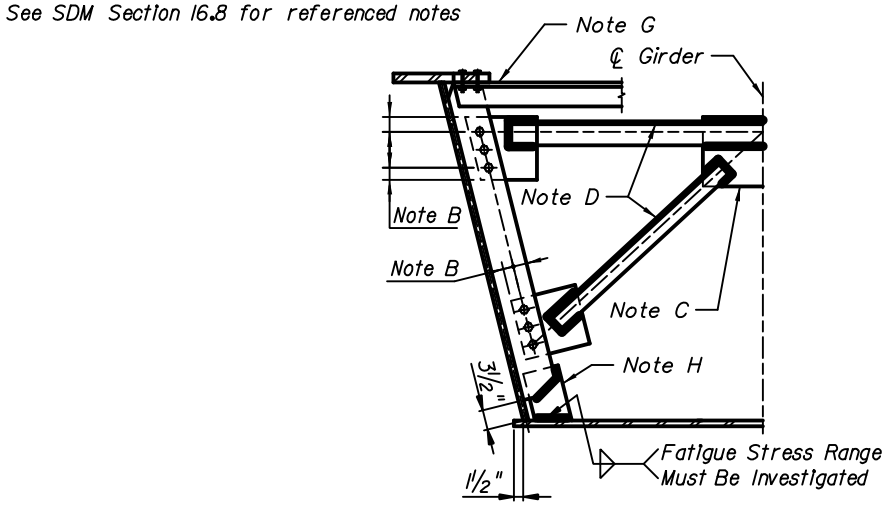
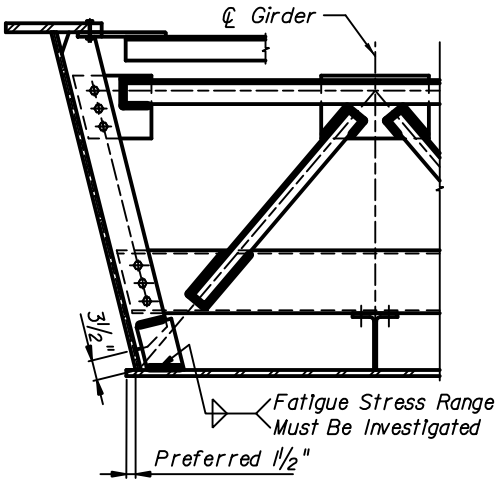


Figure 16.8-3 Steel Box Girder Internal Crossframe



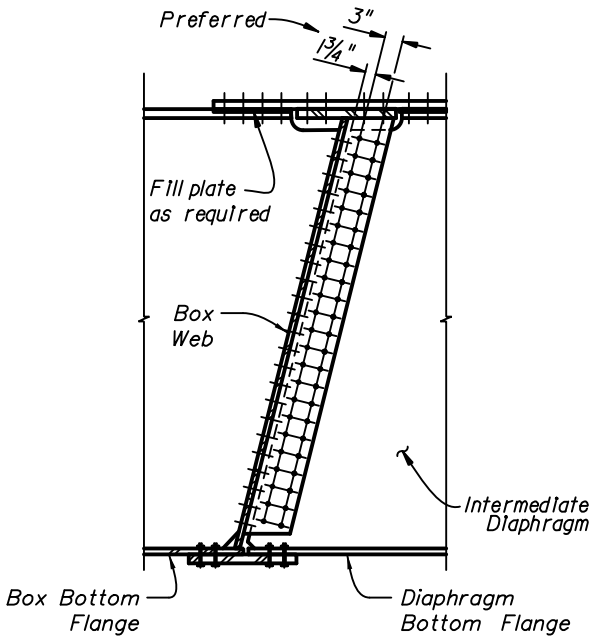
STEEL BOX GIRDER INTERNAL CROSSFRAME

Figure 16.8-4 Steel Box Girder Crossframe with Longitudinal Stiffener



STEEL BOX GIRDER CROSSFRAME WITH LONGITUDINAL STIFFENER

Figure 16.8-5 Steel Box Girder Intermediate Diaphragm Connection



INTERMEDIATE PERMANENT EXTERNAL DIAPHRAGM CONNECTION TO BOTTOM FLANGE

16.9 GIRDER DEFLECTION AND CAMBER DIAGRAMS

Girder Deflection and Camber sheets are required for all steel superstructures. These sheets should provide sufficient geometric reference for girder fabrication and deck placement. For examples illustrating the content and format of completed Camber Diagram sheets, see the ***Structures Detailing Manual Examples***. Typically, camber and deflection ordinates are shown in tabular format. Show total dead load deflection along the centerline of the box for box girders, and net camber along the centerline of the girder for I-girders. At a minimum, include the following on the Girder Deflection and Camber sheet (see Figure 16.9-1 and Figure 16.9-2):

A. Line diagram showing a graphical representation of the following:
   1. Total camber including vertical curve camber*.
   2. Span number and length.
   3. Horizontal increment ordinate locations.
   4. Label centerline bearing.

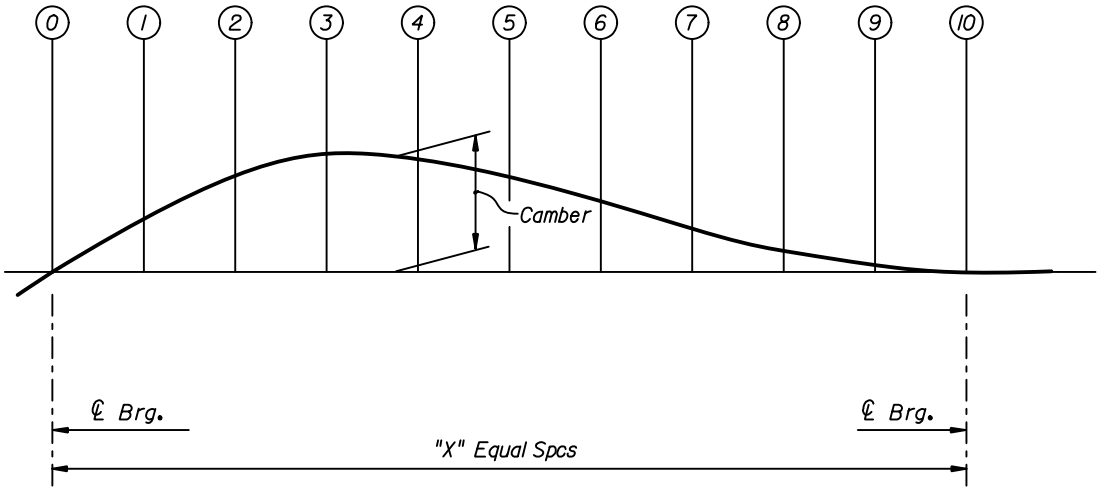
B. Tabulated camber ordinates for the following:
   1. Steel deadload deflection.
   2. Slab deadload deflection.
   3. Superimposed deadload deflection.
   4. Total deadload deflection.
   5. Vertical curve*. If not applicable, omit this row from the table.
   6. Total required camber including vertical curve camber*.

   * vertical curve ordinate not included for box girders.

C. Miscellaneous camber notes:

1. Indicate downward deflection as positive.
2. Indicate upward camber as positive.
3. Slab deadload deflection should be based on deck casting sequence. Include note in the plans that changes in casting sequence will require re-calculation of slab deadload deflections.

Figure 16.9-1 I-Girder Camber Diagram

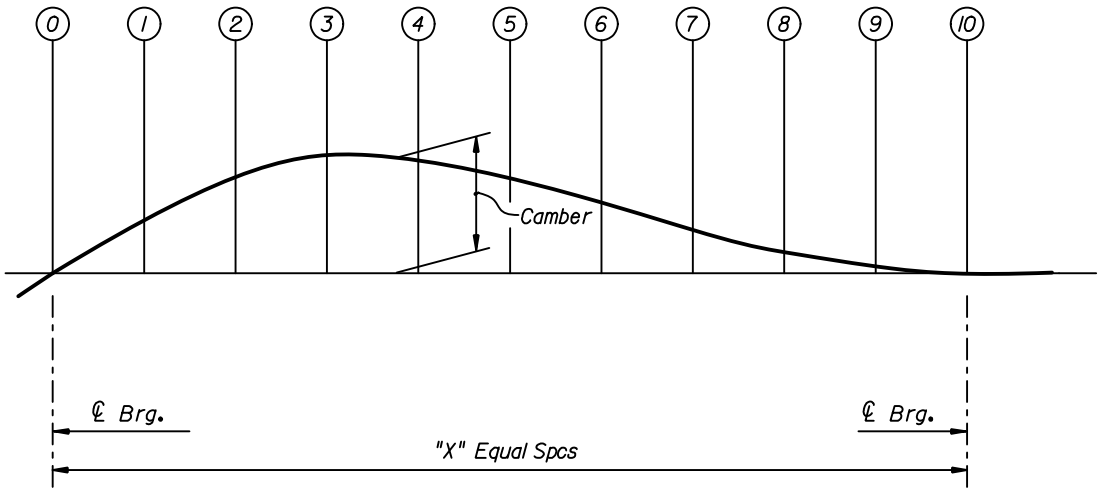


LINE		0	1	2	3	4	5	6	7	8	9	10
GI	STEEL DL	0										0
	CONCRETE DL	0										0
	SUPERIMPOSED DL	0										0
	TOTAL DL	0										0
VERTICAL CURVE	VC	0										0
	REQUIRED CAMBER	0										0

Cambers can be given in fractions, decimal of a foot or decimal inches.
Upward camber is positive. Downward deflection is positive.

I-GIRDER CAMBER DIAGRAM

Figure 16.9-2 Box Girder DL Deflection Diagram



LINE		0	1	2	3	4	5	6	7	8	9	10
G1	STEEL DL	0										0
	CONCRETE DL	0										0
	SUPERIMPOSED DL	0										0
	TOTAL DL	0										0

Cambers can be given in fractions, decimal of a foot or decimal inches.
Upward camber is positive. Downward deflection is positive.

BOX GIRDER DL DEFLECTION DIAGRAM

16.10 WELDS

A. Avoid using details that cause stress concentrations in the weld and a decrease in the basic allowable stress range.

B. It is the designer's responsibility to design the connections; however the detailer should be familiar with Table 6.6.1.2.3-1 and Figure 6.6.1.2.3-1 in ***LRFD***. This table indicates that welds cause reductions in allowable fatigue strength and the reductions are governed by the magnitude of discontinuities in the welds.

C. Avoid the following welds defined as Category E weld types in ***LRFD*** Table 6.6.1.2.3-1:

   1. Intersecting welds: Do not use this type detail.
   2. Longitudinal weld terminations: Plates on cover-plated beams should terminate at the ends of beams. Welds should be continuous. When attachments require longitudinal welds on beams or girders, refer to ***LRFD***.

D. Any detail, except as noted above, can be used provided it is properly accounted for in the design and details. The simplest detail consistent with the stress requirements will generally be the most desirable from the standpoint of design, fabrication and economics.

16.11 MISCELLANEOUS DETAILS

Steel girder details should be complete to provide sufficient information for fabrication and erection of the girders. These miscellaneous details can be shown on a separate sheet or incorporated in any of the previous sheets, as space permits. The following details are typical for most steel girder applications:

A. Transverse shear connector spacing detail (weld symbol not required in this detail as it is covered by Specification 502). See Figure 16.11-3.

B. Anchor bolt details.

C. Field splice detail. Include the following (see Figure 16.11-2):

   1. Plan view of both top and bottom flange. Include thickness and/or width transition detail. It is not necessary to transition top flange widths at field splices where the section changes since the top flange will not be visible.
   2. Section view.
   3. Elevation view.
   4. Plate sizes and thicknesses, including filler plates.
   5. Bolt layout and spacing. Provide dimension from top of web to uppermost row of bolts in the web splice. Maintain minimum edge distance requirements.

D. Stay-in-place form details. Detail forms inside box girders to avoid conflicts with the top flange lateral bracing system. See ***SDM*** Chapter 15 for typical SIP form details.

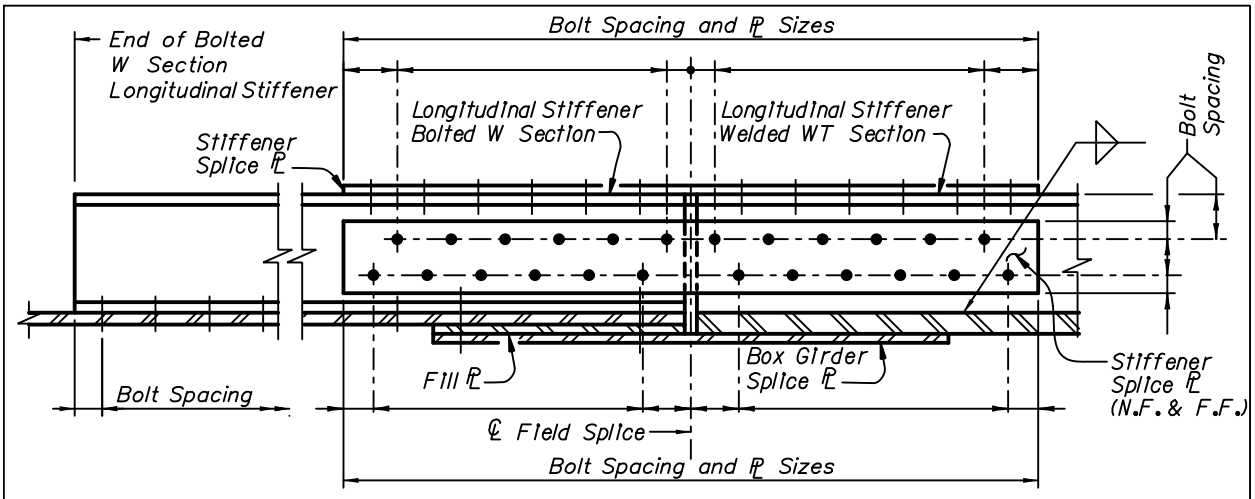
E. Box girder details including the following:

   1. Access opening details.
   2. Vermin guards.
   3. Diaphragm access opening details.
   4. Drain holes with screen covers. Drain holes should be a minimum of 5' from centerline pier or front face of backwall. See Figure 16.11-4.
   5. Electrical access holes.
   6. Longitudinal stiffener details. Detail longitudinal stiffener termination as shown. See Figure 16.11-1.

F. Details for shop splices. Show minimum distances to field splice, section change locations, nearest stiffeners, etc. Detail welds as full penetrations welds. See Figure 16.11-5.

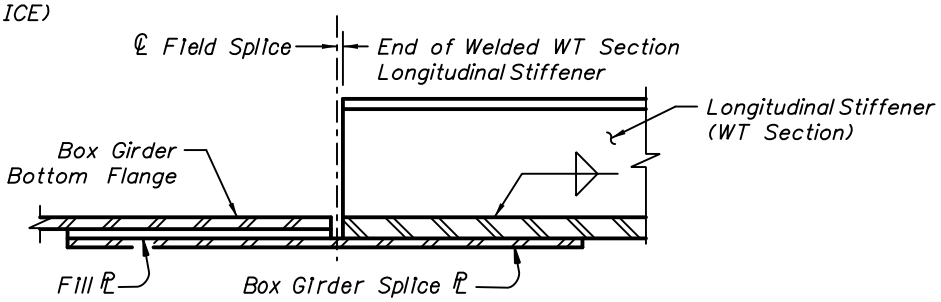
G. Details for closure pours when phased construction is required. Show a section through the closure pour bay detailing the Phase I bolt holes in the connector plates as slotted and Phase II bolt holes in the connector plates to be field drilled. See Figure 16.11-6.

Figure 16.11-1 Box Girder Bottom Flange Stiffener Transition and Termination Details

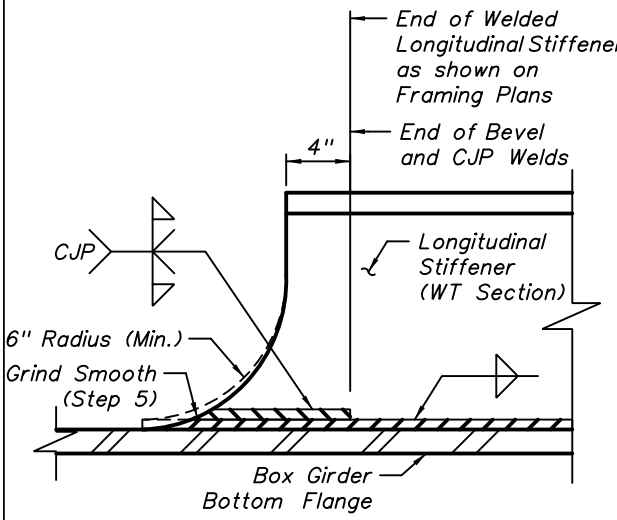


BOLTED LONGITUDINAL STIFFENER TERMINATION
(BEYOND FIELD SPLICE)

LONGITUDINAL STIFFENER TRANSITION AT FIELD SPLICE



WELDED LONGITUDINAL STIFFENER TERMINATION (AT FIELD SPLICE)



WELDED LONGITUDINAL STIFFENER TERMINATION (BEYOND FIELD SPLICE)

BOTTOM FLANGE LONGITUDINAL STIFFENER TERMINATION WELDING SEQUENCE NOTES:

1. Bevel Portion of Stiffener to be Groove Welded. Grind or Gouge Notch at End of Bevel to a smooth rounded contour.
2. Cut Radius, Leaving enough material to permit a satisfactory Groove Weld where the Cut Edge becomes tangent to the Bottom Flange.
3. Manually Groove Weld the end portion, starting at the End of the Bevel and moving toward the end of the Stiffener. Do not use a Back-up Bar; instead, Back Gouge as required to achieve a satisfactory Complete-Penetration Groove Weld.
4. Place Submerged Arc Fillet Welds on both sides of the Stiffener for its entire length, including the end portion already Groove Welded.
5. Grind the Weld and the Stiffener material to a smooth contour where the Radiused Stiffener End becomes tangent to the Bottom Flange.

BOX GIRDER BOTTOM FLANGE STIFFENER TRANSITION & TERMINATION DETAILS

Figure 16.11-2 Web Splice Detail

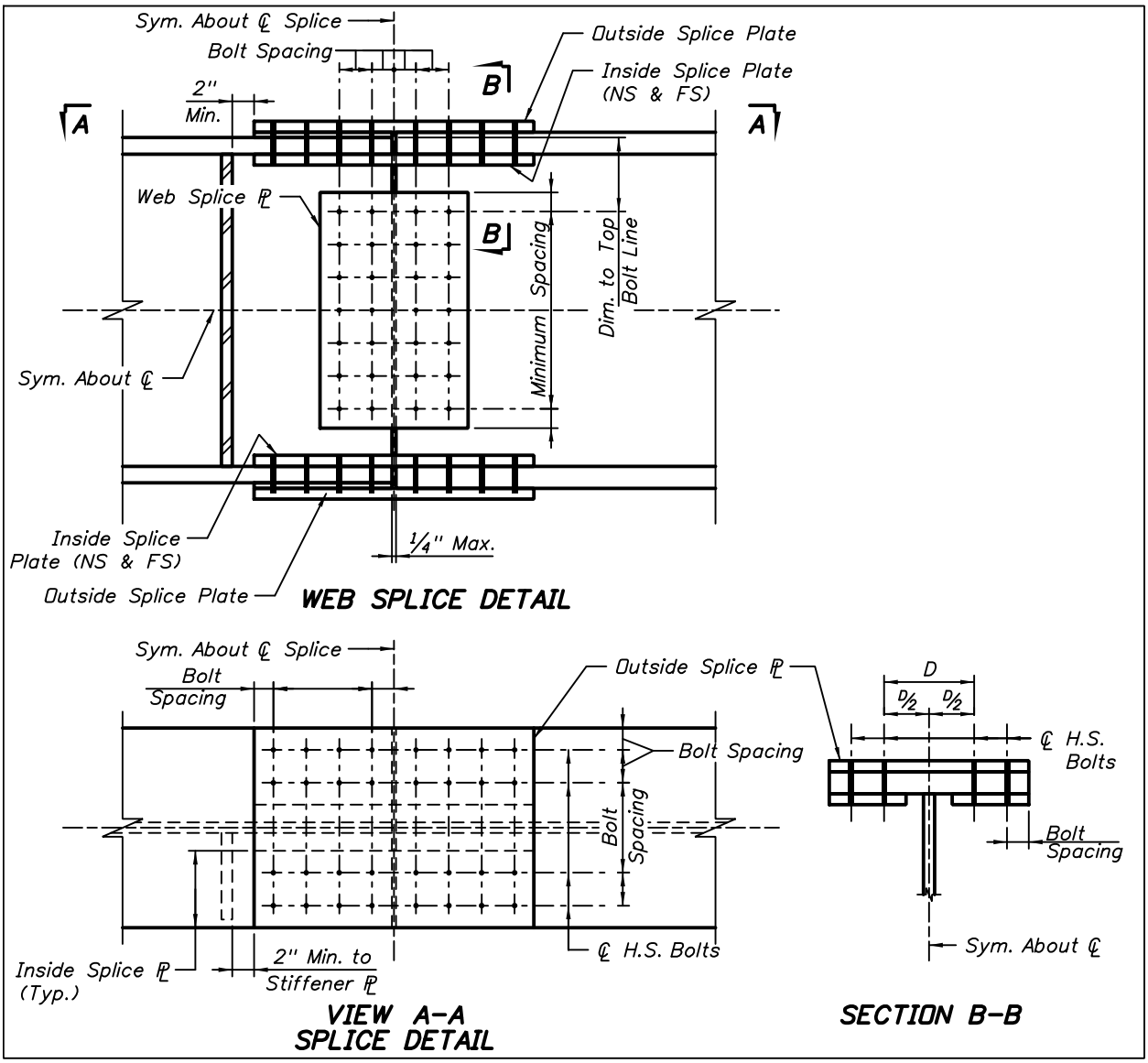


Figure 16.11-3 Top Flange Shear Connector Detail

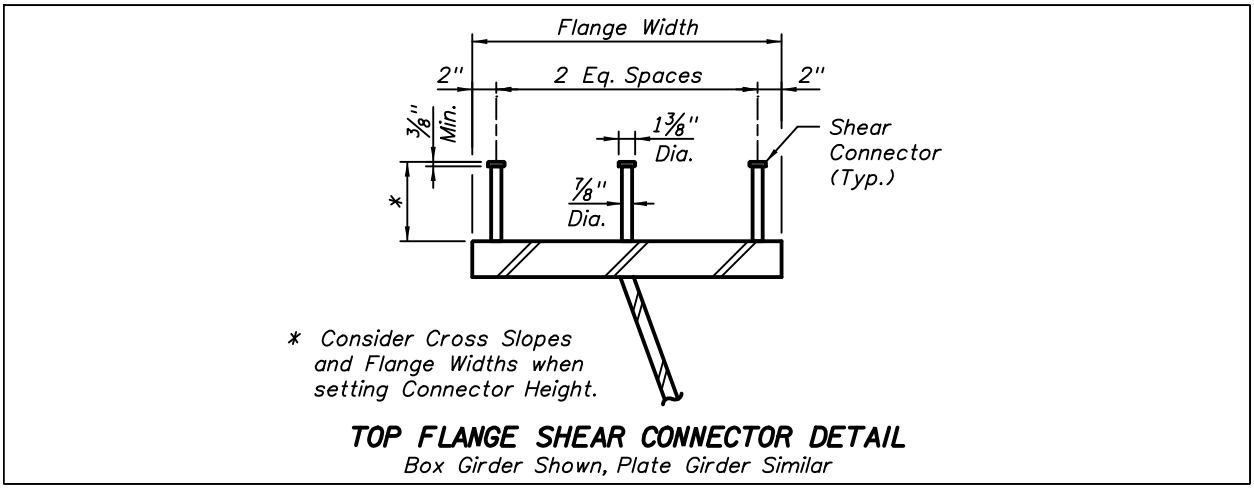


Figure 16.11-4 Box Girder Section with Drain Hole Detail

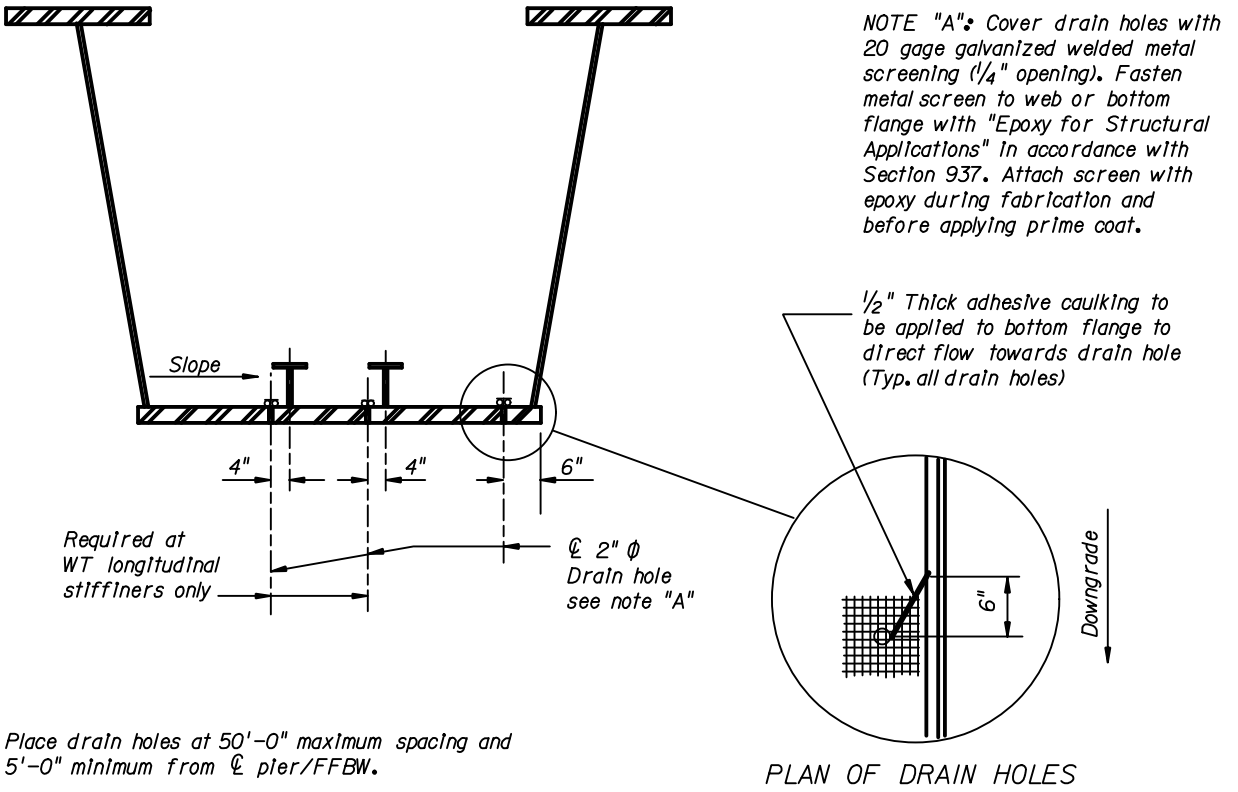


Figure 16.11-5 Shop Splice Details

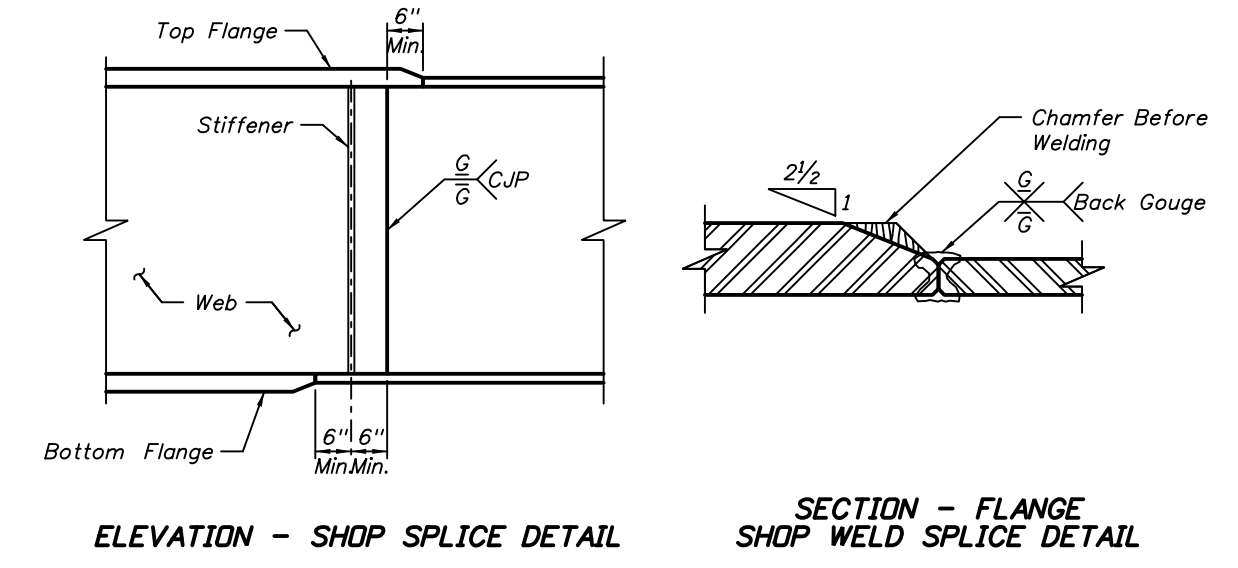
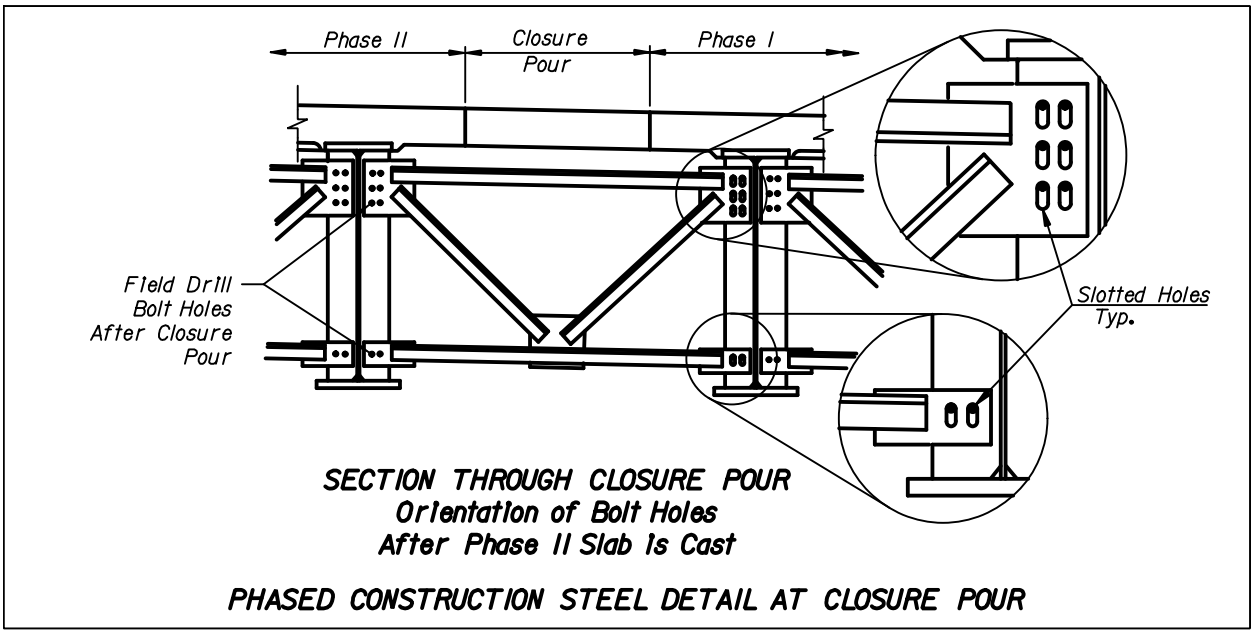


Figure 16.11-6 Phased Construction Steel Detail at Closure Pour



17 TYPICAL SECTION

17.1 GENERAL

A. The purpose of the Typical Section sheet is to show the dimensions for the bridge deck, beam spacing and roadway configuration. Reinforcement details belong on the Superstructure Section sheet(s).

B. The Typical Section sheet can be combined with the Pier Elevation sheet. This sheet should be titled Bridge Section sheet.

C. For an example illustrating the content and format of a completed Typical Section sheet, see the ***Structures Detailing Manual Examples***.

17.2 TYPICAL SECTION - DRAWING AND DETAILS

At a minimum, include the following in the Typical Section sheets:

A. Section of the bridge deck. Use a suitable scale for the drawing. If there are two bridges, show both sections. Break lines are allowed in the dimension and the drawing.

B. Right-of-way lines when bridge is being built on a new or shifted alignment. Show Temporary Construction Easement lines if applicable.

C. Station Line. Label PGL.

D. Dimensions for traffic railing widths, shoulder widths, lane widths, bike lane widths, sidewalks, median width and overall width. Include dimensioning of future lanes.

E. Existing bridge structure for widenings. Draw existing superstructure as dashed. Hatch or shade portions that are to be demolished.

F. Beam type, size and spacing. If spacing varies, indicate minimum and maximum dimensions. Reference appropriate ***Design Standard***.

G. Overhang distance. If overhang varies, indicate minimum and maximum dimensions.

H. Traffic Railing type. Reference the applicable ***Design Standard***. If more than one traffic railing is used (median, pedestrian, etc.) indicate each applicable ***Design Standard***.

I. Traffic Separator. Reference appropriate option in the ***Design Standards***.

J. Slab thickness.

K. Construction joints. Label construction phases consistently with the TCP and other sheets in the plan set.

L. Cross slope. If cross slope is in transition, show as "Varies".

M. Pier elevation. If pier elevation is combined with superstructure sheet, label sheet as Bridge Section.

18 ADA REQUIREMENTS

18.1 GENERAL

A. This chapter provides graphical representations of the preferred methods for compliance with the Americans with Disabilities Act (ADA) and Florida Accessibility Code. In general, special attention should be paid to sidewalks on bridges when:

   1. Bridge or bridge approach grades are greater than 5%.
   2. Drainage grates or scuppers are placed within the limits of the sidewalk.
   3. Expansion joints are placed within the limits of the sidewalk.
   4. Bridge cross slopes exceed 2%.

B. When required, include all details in the plans necessary to build the ramps and handrails based on accessibility code compliance.

C. See ***ADA Standards for Transportation Facilities*** for other facilities not covered in this chapter.

D. These guidelines are meant to apply to a broad range of situations. For situations where these guidelines will be difficult to implement, consult the DSDE for guidance.

18.2 RAMPS AND HANDRAILS - GRADES GREATER THAN 5%

Sidewalks on bridges must comply with ADA and Florida Accessibility Code. The following details are intended to address sidewalks with grades steeper than 5% and/or cross slopes greater than 2%. The following guidelines apply to sidewalks on bridges that meet these criteria:

A. For grades greater than 5%, ramps will be required. Provide landings and maximum grades as outlined in the ***ADA Standards for Transportation Facilities*** and the details shown in Figure 18.2-1.

B. Where ramps are required, handrails are required on both sides of the sidewalk. Handrails should be placed at a constant distance from the landing/ramp surface. For aesthetic purposes, the pedestrian/traffic railing should be constant throughout the bridge. Modify the height of the pedestrian/traffic railing to hide the ADA handrail. See Figure 18.2-2 and Figure 18.2-3.

C. Handrail assemblies, brackets and attachments are required to withstand a minimum point load of 250 pounds applied vertically down and horizontally but not simultaneously at the top of the rail. Additionally, the rail system design must be in compliance with current ***ADA*** requirements and all applicable state and local authorities.

D. Show rails continuous over a minimum of three wall brackets. Space splices at 40'-0" centers maximum. Locate the center of a splice near the edge of a wall bracket.

E. Indicate with plan notes that the Contractor is required to submit the following to the Engineer for approval prior to fabrication:

   1. Shop drawings with complete details including rail bracket and expansion joint locations. Indicate component details, materials, finishes, connections, and joining methods and the relationship to the adjoining work.

   2. Summary of the materials for the proposed rail system, including mill analysis with certification by the producer that the parts are the alloys specified and meet the specifications called for. See ***Specification*** 965.

   3. The manufacturer's engineering design and data for the rail system and components signed and sealed by a Professional Engineer registered in the state of Florida.

   4. The manufacturer's installation instructions and product data.

F. Materials and Finishes: The following materials and finishes requirements should be indicated on the plans:

   1. Rails and splice assemblies are to be fabricated from extruded aluminum pipe, Alloy 6061-T6 or 6063-T52 ASTM B 221. Provide Schedule 40 rails with nominal size of 1½ inches, 0.145 minimum wall thickness with a mill finish.

   2. Provide wall brackets of extruded aluminum alloy6061-T6 or 6063-T52 ASTM B 221 with a mill finish.

   3. When directed by the DSDE, galvanized steel may be substituted for aluminum pipe and wall brackets.

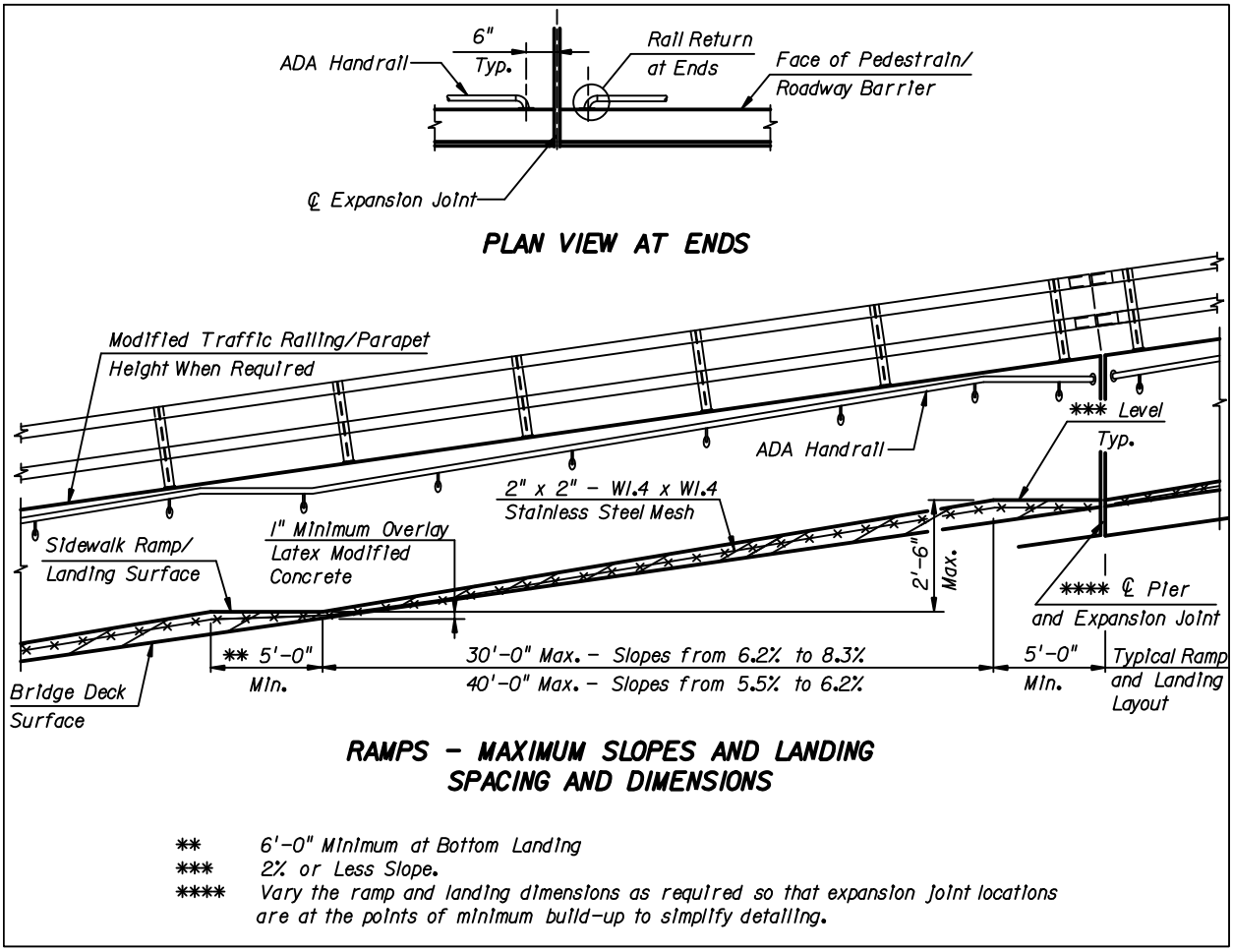
   4. Provide mechanical fasteners of the type and size required by the manufacturer's specification and design calculations.

      a. Provide anchor bolts for the brackets in accordance with ASTM A36 or ASTM F1554, Grade 36. Anchor bolts, nuts and washers must be hot-dipped in accordance with ***Specification*** 962.

      b. Use stainless steel, ASTM F-593, Alloy Group 2 (316) for all fasteners and washers used at the splice assemblies and to mount the rails to the brackets.

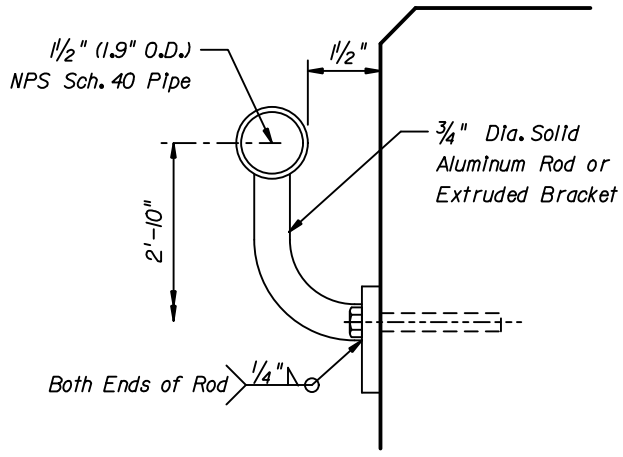
G. Erect rails parallel to and 2'-10" above the top of pedestrian ramp as shown in Figure 18.2-3.

Figure 18.2-1 Maximum Slopes and Landing Spacing and Dimensions for Ramps



** 6'-0" Minimum at Bottom Landing
*** 2% or Less Slope.
**** Vary the ramp and landing dimensions as required so that expansion joint locations are at the points of minimum build-up to simplify detailing.

Figure 18.2-2 ADA Handrail Detail



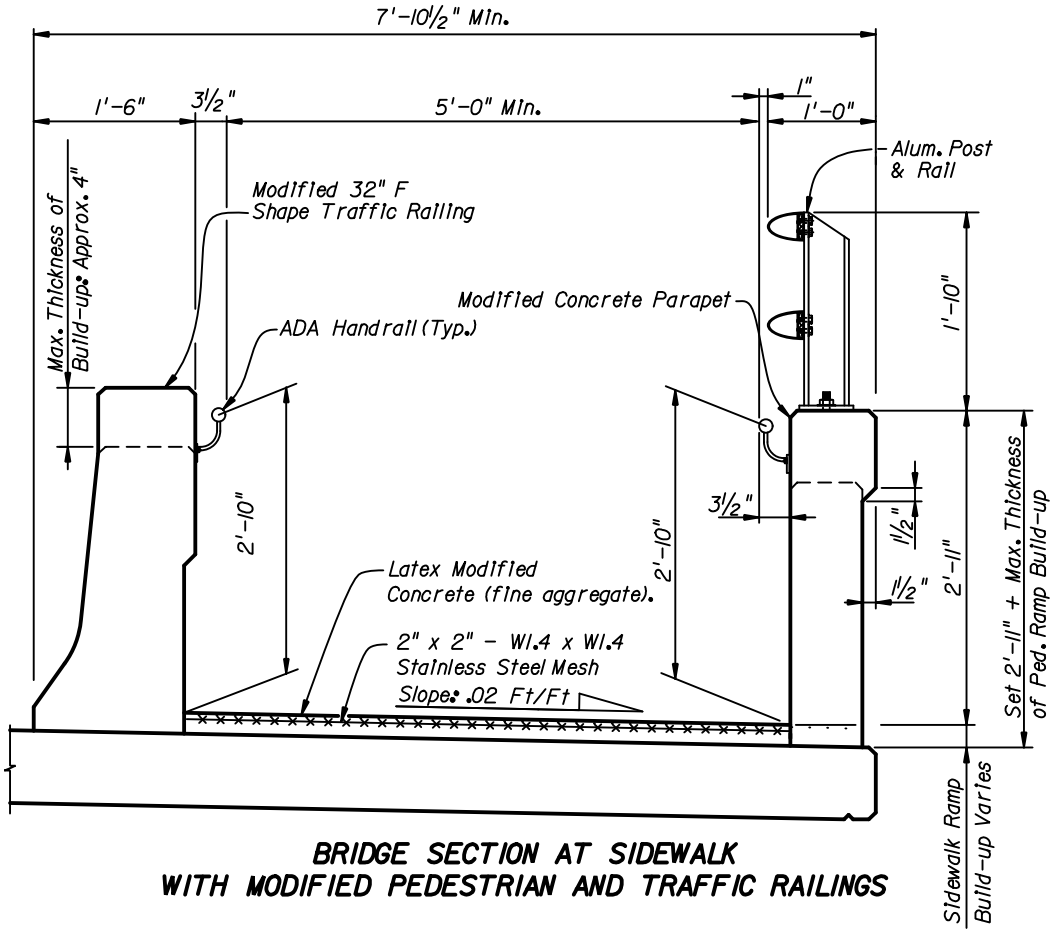
RAMPS

ADA ramps, landings and ADA handrails shall be provided within the sidewalk limits where bridge and bridge approaches have grades greater than 5%.

ADA handrails are required on both sides of the sidewalk in areas where ramps are required. The ADA handrails shall be placed a constant distance from the landing/ramp surface. It is aesthetically desirable to maintain a constant height pedestrian/roadway barrier for the full bridge length. The height of the pedestrian/roadway barrier should be sufficient to hide the ADA handrail.

A minimum clear distance of 5'-0" between ADA handrails shall be provided.

Figure 18.2-3 Bridge Section At Sidewalk With 32" "F" Shape Barrier (Modified)



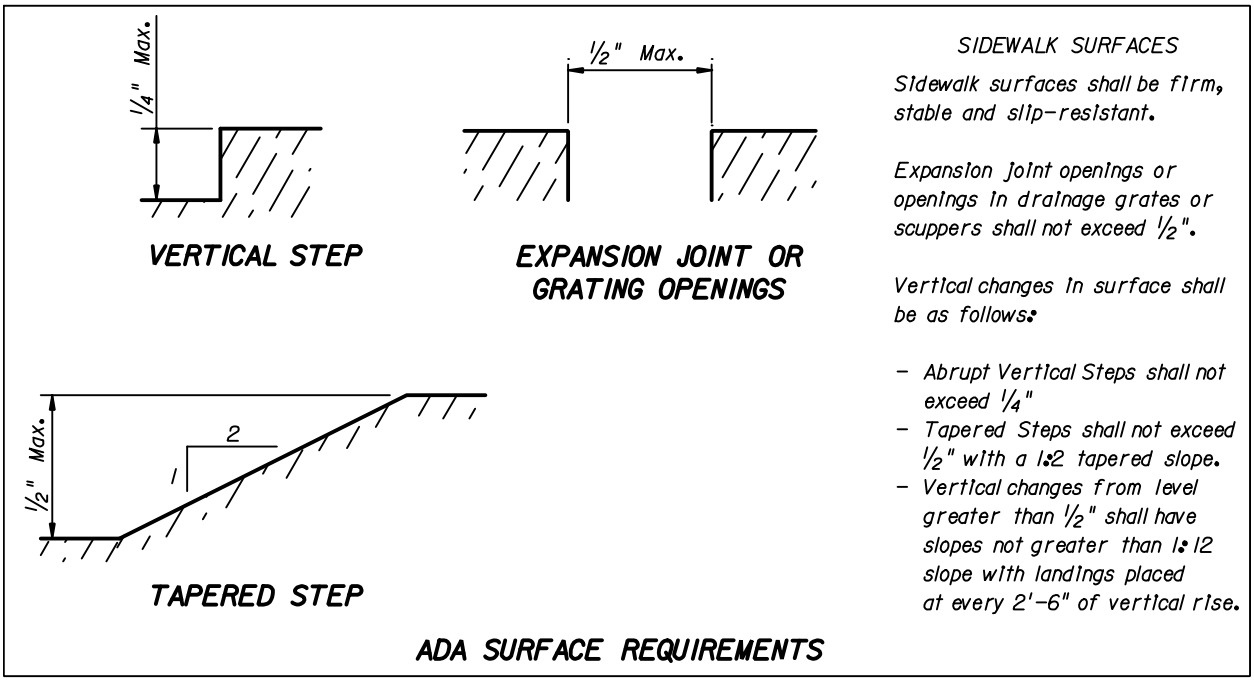
SIDEWALK RAMP BUILD-UP NOTES

1. Sand-blast or water-blast deck surface within limits of the sidewalk.
2. Remove all loose concrete and laitance in accordance with the manufacturer's recommendations.
3. Apply an approved epoxy bonding agent to the exposed concrete surface.
4. Overlay with latex modified concrete to the dimensions indicated for the special pedestrian ramp and landings.
5. Finish deck surface in accordance with Section 522.

18.3 DRAINAGE GRATES OR SCUPPERS

Drainage grates or scuppers that lie within the limits of the sidewalk are required to conform to ***ADA Standards for Transportation Facilities***. At a minimum, openings in gratings are to be no more than ½" wide in one direction. For surface requirements, see Figure 18.3-1.

Figure 18.3-1 ADA Surface Requirements



18.4 EXPANSION JOINTS ON BRIDGES

Expansion joints on bridges with sidewalks are addressed in ***Design Standard*** 21100 (Strip Seal Joints) and 21110 (Poured Joint with Backer Rod). Similar methods are required for bridges with large joints such as modular joints or finger joints. See Figure 18.3-1 for additional surface requirements.

18.5 CROSS SLOPES GREATER THAN 2%

The maximum cross slope for sidewalks on bridges is 2%. For bridges with a cross slope greater than 2%, the sidewalk must be modified to be in compliance. This can be achieved either by using a tapered build-up in the sidewalk portion of the bridge or by providing a change in cross slope at the gutterline. See Figure 18.5-1, Figure 18.5-2 and Figure 18.5-3.

Figure 18.5-1 Section at Sidewalk With Bridge Cross Slope Greater Than 2% (1 of 3)

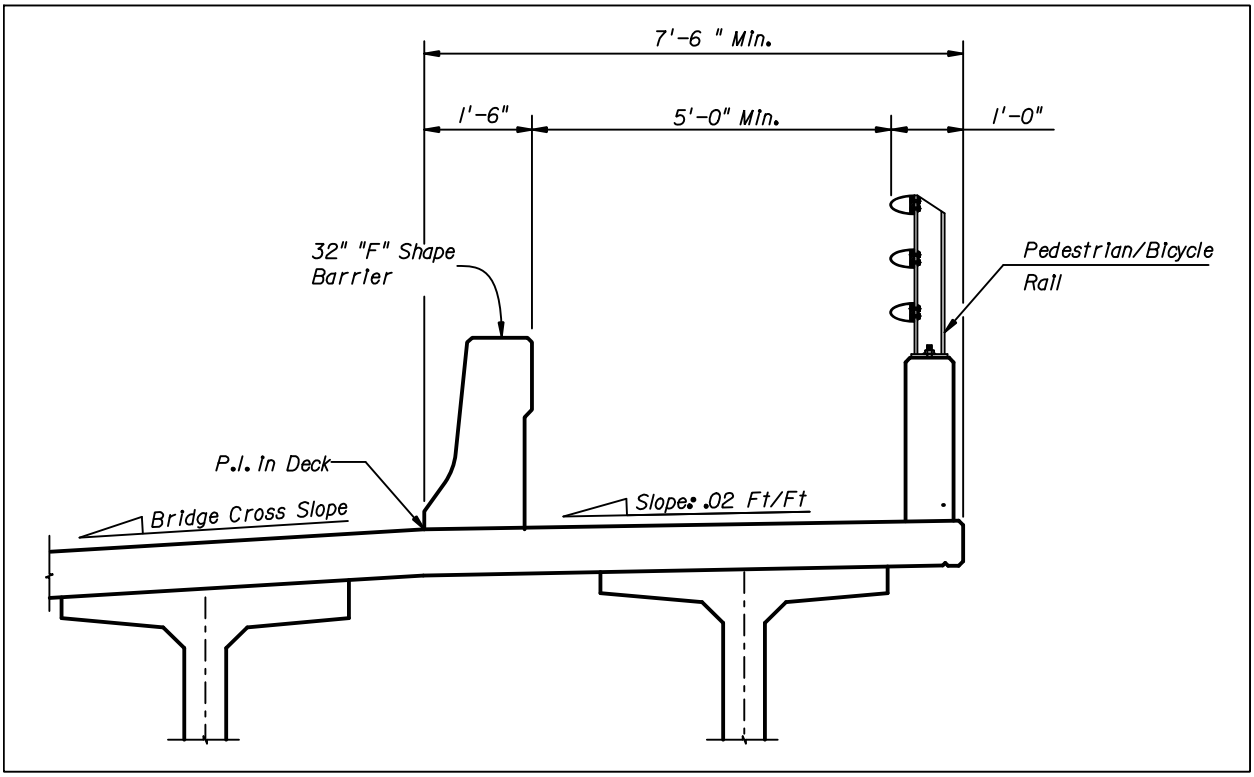


Figure 18.5-2 Section at Sidewalk With Bridge Cross Slope Greater Than 2% (2 of 3)

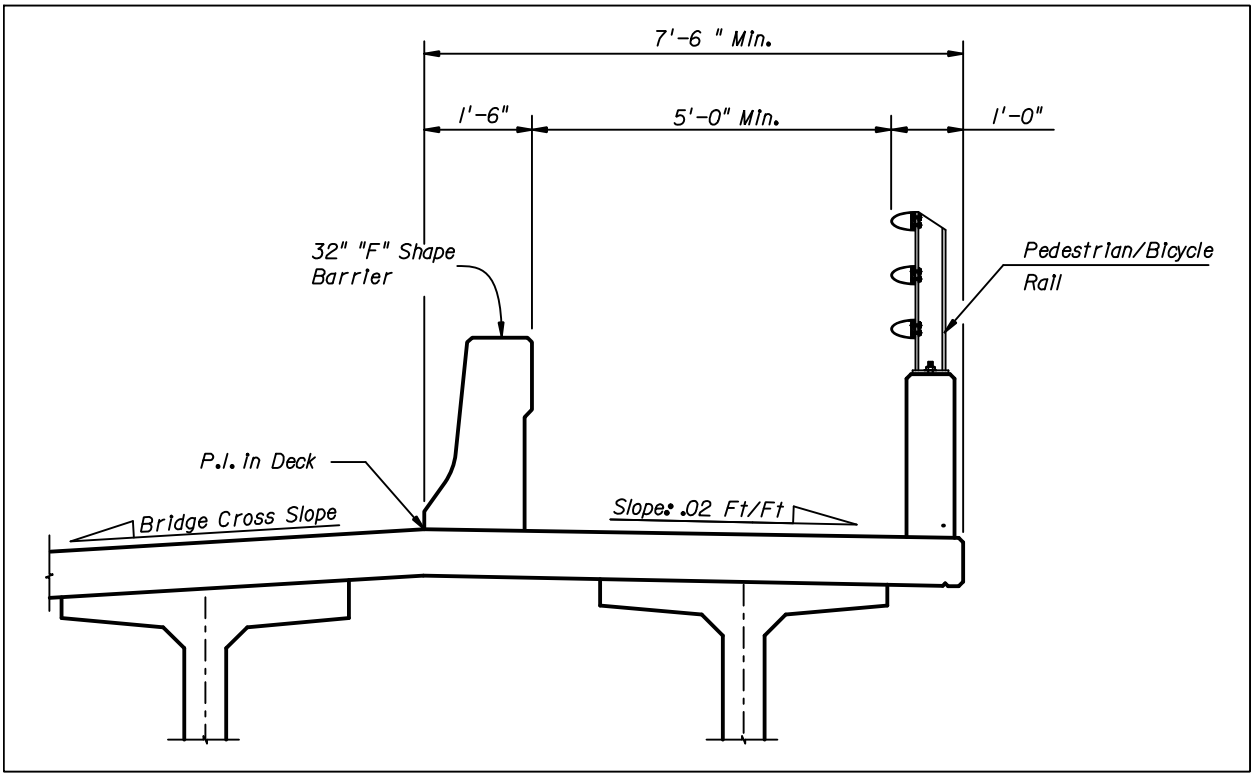


Figure 18.5-3 Section at Sidewalk With Bridge Cross Slope Greater Than 2% (3 of 3)