

Session 23

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Research Findings for LRFD Deck Design

Topic Description

FDOT and TxDOT jointly sponsored research into safety shaped bridge parapets on cantilevered decks. LRFD deck thickness requirements suggest current deck thicknesses used by FDOT might be inadequate. Research and crash tests don't support this notion.

Speaker Biography

Dean Alberson

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Dr. Dean C. Alberson is an associate research engineer and program manager of the Crashworthy Structures Program with Texas Transportation Institute. He obtained his Ph.D. in Civil Engineering Structures at Texas A&M University. He originally came to TTI in 1992 to work with the Engineering Factors Program in the Safety Division. During that time, he was actively involved in the full-scale testing and evaluation of numerous highway safety features and was subsequently promoted to Program Manager of Engineering Factors. From 1996 to February of 2000, he was employed as a private consultant to several roadside safety hardware manufacturers. During this time, Dr. Alberson worked closely with manufacturers in the support of existing products, helped them improve their products, and developed new products. These tasks were accomplished through computer modeling, prototype tests, and full-scale developmental tests. Dr. Alberson is a registered professional engineer.



Research Findings for LRFD Deck Designs

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Acknowledgements

Charles Boyd, P.E.
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Special Projects Engineer
Texas Department of Transportation



Dinosaur Bite?



Looks Like a TL-4 Truck



Current Typical Design

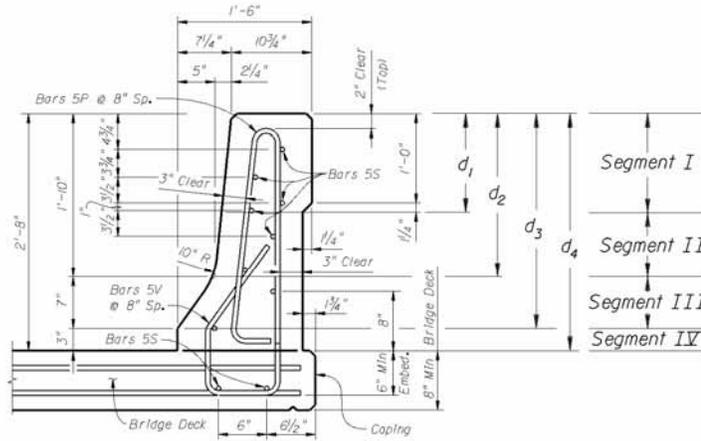


FIGURE 1
 TYPICAL SECTION THRU TRAFFIC RAILING BARRIER



AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

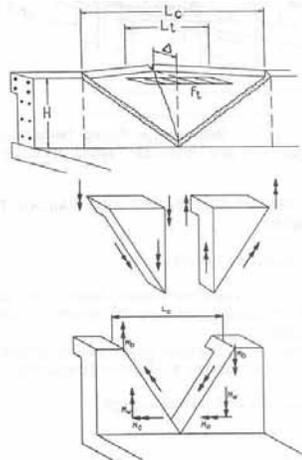


Figure CA13.3.1-1 - Yield Line Analysis of Concrete Parapet Walls for Impact within Wall Segment



AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

Table A13.2-1 - Design Forces for Traffic Railings

Design Forces and Designations	Railing Test Levels						
	TL-1	TL-2	TL-3	TL-4	TL-5A	TL-5	TL-6
F_t Transverse (KIP)	13.5	27	54	54	116	124	175
F_l Longitudinal (KIP)	4.5	9.0	18.0	18	39	41	58
F_v Vertical (KIP) Down	4.5	4.5	4.5	18	50	80	80
L_t and L_l (FT)	4.0	4.0	4.0	3.5	8.0	8.0	8.0
L_v (FT)	18.0	18.0	18.0	18.0	40.0	40.0	40.0
H_b (min) (IN)	18	20	24	32	40	42	56
Minimum H Height of Rail (IN)	27	27	27	32	40	54	90



AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

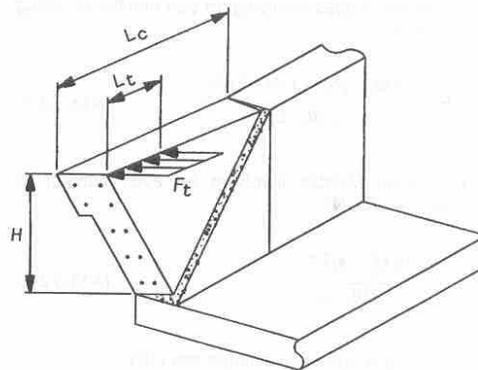


Figure CA13.3.1-2 - Yield Line Analysis of Concrete Parapet Walls for Impact Near End of Wall Segment



Computed Parapet Strengths

- Transverse Load @ Joint (Single Yield Line)
 - 41.5 kips
- Transverse Load @ Mid-span (Dual Yield)
 - 62.1 kips

*Failure is assumed to occur in upper slender portion of parapet.



TL-4 Impact @ Joint





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TL-4 Impact



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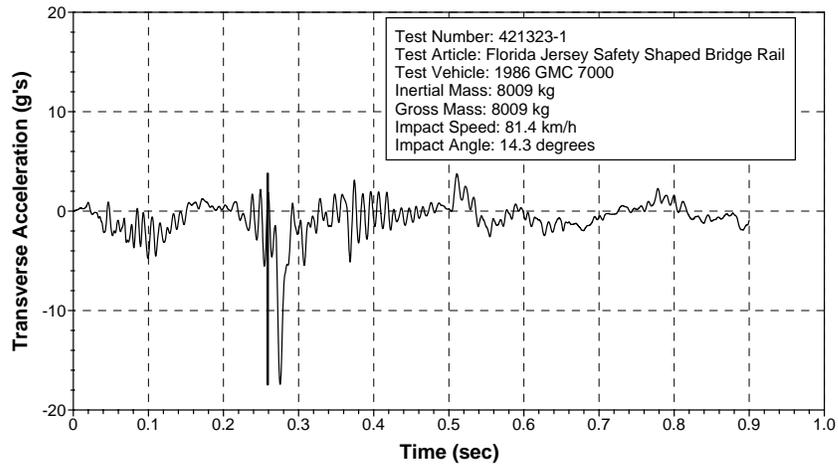
TL-4 Impact



Installation After TL-4 Impact



Y Acceleration at CG



— Time of OIV (0.2589 sec) — SAE Class 60 Filter

50 ms 4.9 g's

Accelerometer on
 Frame

TL-3 Impact @ Joint



TL-3 Impact



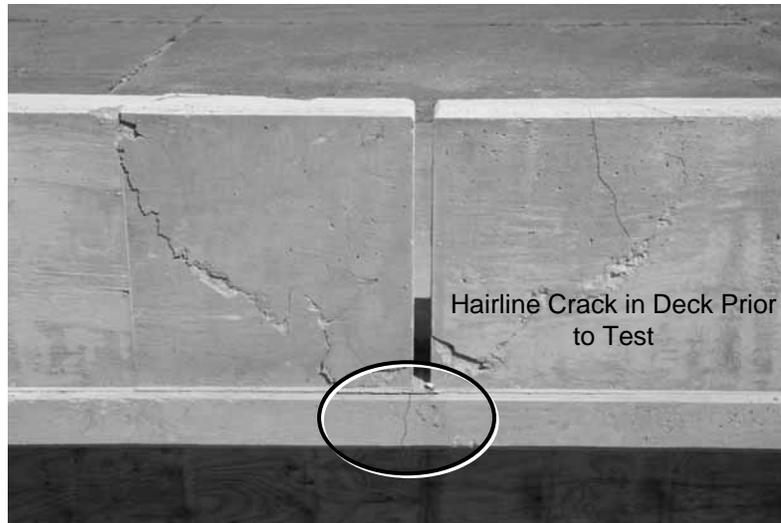
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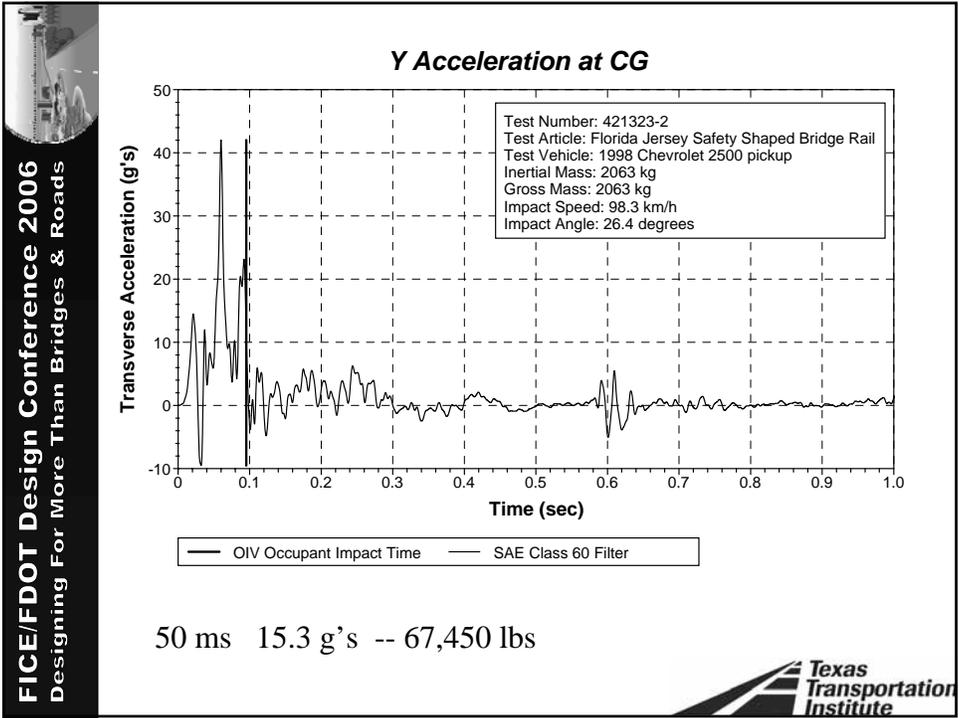
Installation After TL-3 Impact



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Installation After TL-3 Impact



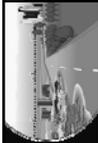


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Static Load Testing

- Simulated AASHTO LRFD Chapter 13 Loading
- Tested at the Joint
- Tested at Mid-Span
- Tested at TL-4 Impact Point
 - After Test (Any damage to parapet not visible?)

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Test Apparatus



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Test Apparatus @ Joint



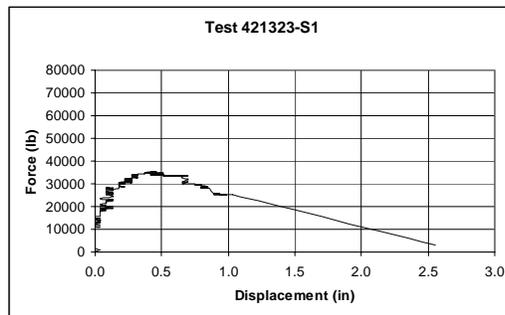
Testing @ Joint



Post Test Damage @ Joint



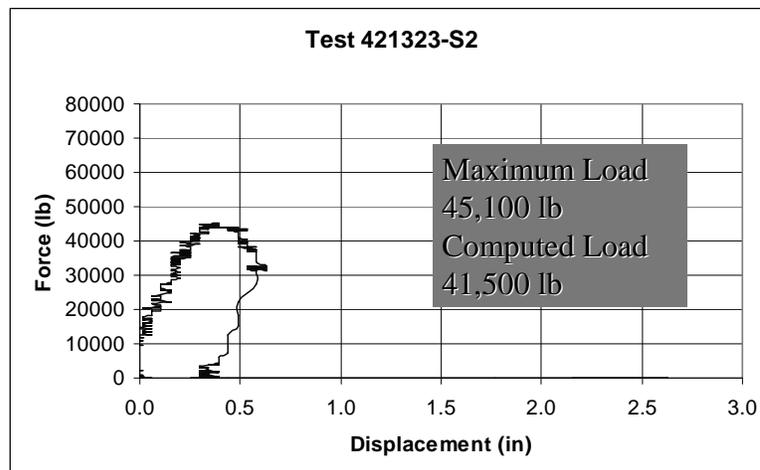
Maximum Load 35,100 lb*
Computed Load 41,500 lb



Post Test Damage @ TL-4 Joint



Post Test Loading @ TL-4 Joint



Testing @ Mid-Span



Break @ Mid-Span





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Damage @ Mid-Span

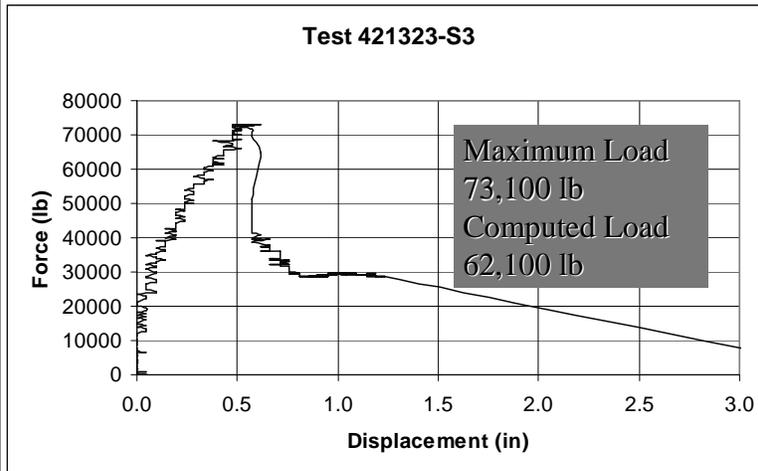


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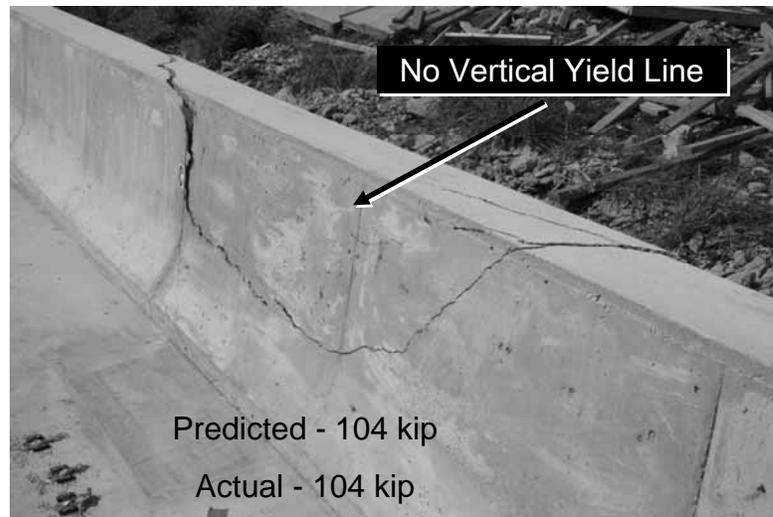
Isolated in Parapet



Loading @ Mid-Span



Mid-Span Current Design



Mid-Span Current Design



Mid-Span Current Design



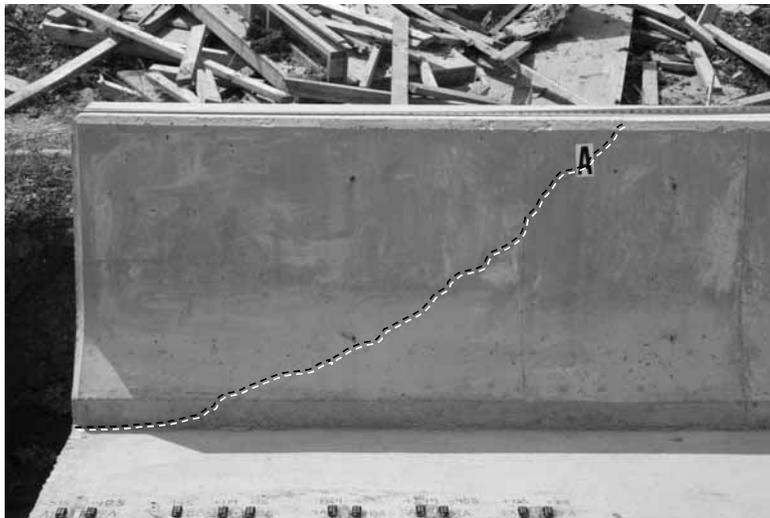
Crack
Propagation into
Deck

104 kips !

End-Span Current Design



End-Span Current Design



73 kip Predicted -- 64 kip Failure

End-Span Current Design



Deck Damage
at 64 kip
at End Segment

Summary

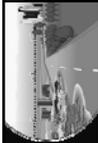
- The minimally reinforced FDOT 32 inch New Jersey Shape Bridge Parapet successfully contained vehicles to TL-4.
- Damage from the TL-3 test was isolated in the parapet.
- The predicted static capacity of the parapet closely matched the actual values.
- Parapet damage in the TL-4 test was cosmetic only and was verified by the “post” test static loading.
- The TL-3 pickup test caused more damage to the parapet than the TL-4 single unit truck.

Summary

- At center span load applications to failure in the parapet, the center vertical yield line was never produced.
- Further review of the damaged zones showed 45 degree shear planes from the load application region.
- Punching shear may be a more appropriate method of analysis for center span failure of strong concrete parapets.

Conclusions

1. Previous Jersey Shaped Parapet Design is Still Adequate to TL-4.
2. Minimal Chance for Deck Damage with Previous Design.
3. Current F-Shape Significant Strength Increase.
4. Chance for Deck Damage Only When Severely Overloaded.
5. Punching Shear may be better Design Method for Mid-Span Design.



Questions?

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