

\* Shear Key is required only when specified by the Engineer

### TYPICAL SECTION

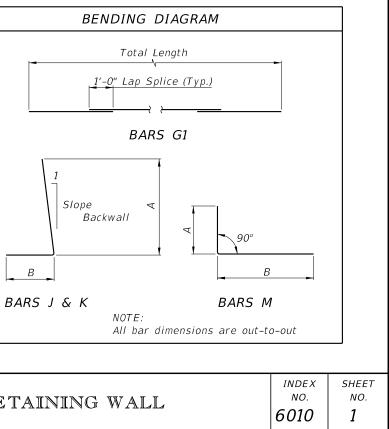
NOTES DESIGN SPECIFICATIONS: Design according to FDOT Structures Manual (current edition). MATERIALS: All reinforcing steel shall conform to ASTM A615 Grade 60. SURFACE FINISH: A Class 5 Applied Finish Coating shall be applied to the top of the wall and the exposed face above ground line. ARCHITECTURAL SURFACE TEXTURES: Alternate Architectural Surface Textures may be substituted for the Striated Pattern shown when approved by the Engineer. Concrete required for Architectural Surface Textures is not included in the quantities. TRAFFIC RAILING BARRIER: If there is a Traffic Railing Barrier on the wall, Wall Joints and Barrier V-Grooves shall align and Wall Expansion Joints and Barrier Open Joints shall align. FOUNDATION: Prepare the soil below the footing in accordance with the requirements for spread footings in Specification Section 455. PAYMENT: В All Retaining Wall costs, including all miscellaneous costs, shall be paid for at the unit contract price for either Class II, III or IV Concrete (Retaining Walls) (CY) and Reinforcing Steel (Retaining Walls) (lbs.). Retaining Wall quantities shall not include concrete nor reinforcing steel for Traffic Railings/Junction Slab. Traffic Railing/Junction Slab shall be paid for under Concrete Traffic Railing Barrier with Junction Slab.

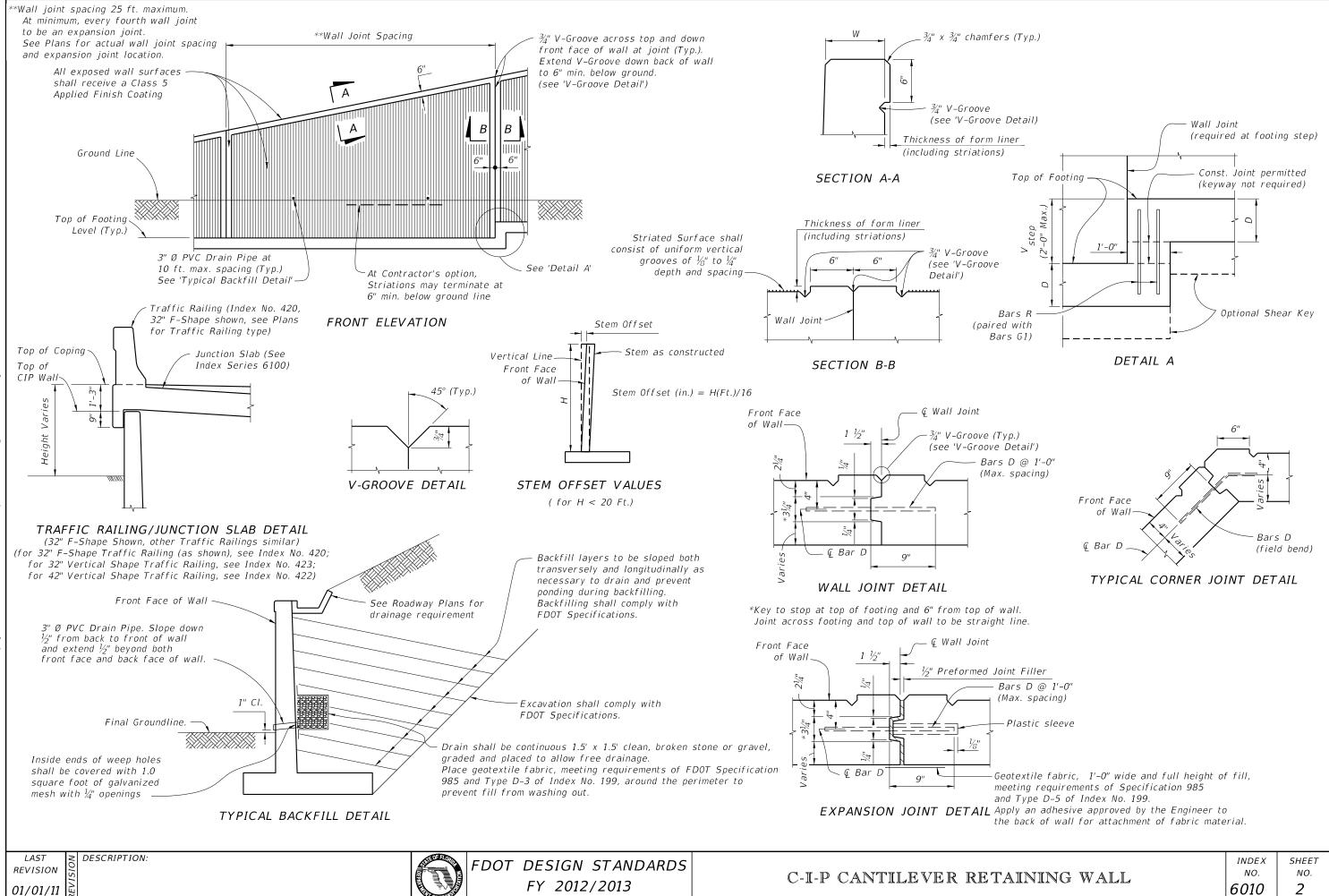
LAST	N	DESCRIPTION:
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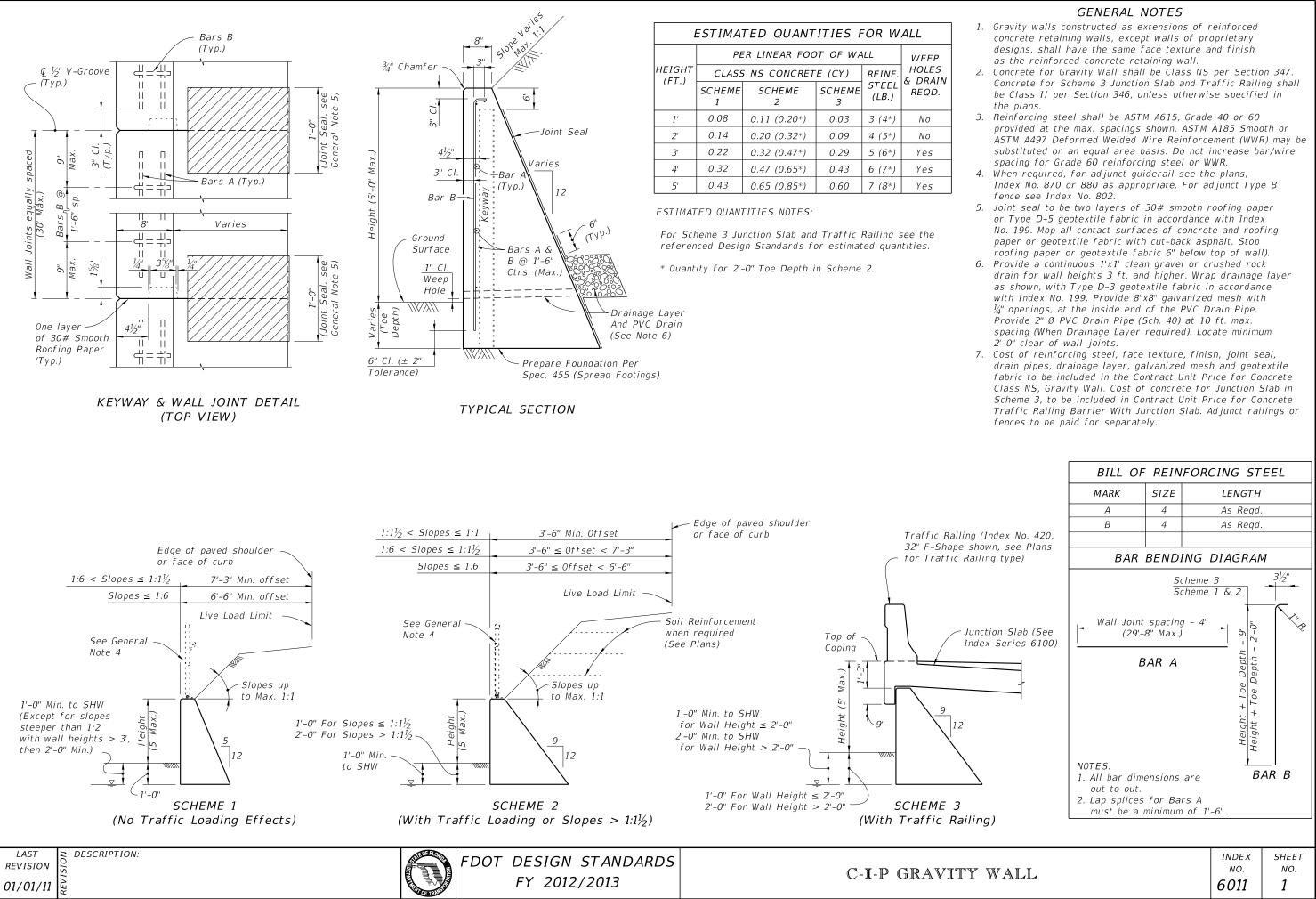


C-I-P CANTILEVER RETAINING WALL





INING WALL	INDEX NO.	SHEET NO.
	6010	2



# NOTES

### SPECIFICATIONS:

- 1. General Specifications:
- The Florida Department of Transportation "Standard Specifications for Road and Bridge Construction", Current Edition and Supplements as Amended. 2. Design Specifications:
- a. Florida Department of Transportation (FDOT) "Structures Design Guidelines", Current Edition
- b. American Association of State Highway and Transportation Officials (AASHTO) "LRFD Bridge Design Specifications", Current Edition.
- c. AASHTO-AGC-ARTBA Task Force 27 (Ground Modification Techniques), "Insitu Soil Improvement Techniques", January 1990.

## DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and Chapter 3 of the FDOT Structures Design Guidelines.
- 2. It is the responsibility of the Engineer of Record to determine that the maximum factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer of Record.
- 4. If there are manholes and/ or drop inlets present, design and analysis for both 14. The top of the leveling pad or footing will be 2'-0" minimum below final internal and external stability shall be considered.

### SOIL PARAMETERS:

- 1. See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- 2. The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

### MATERIALS:

- 1. Concrete Class: See Wall Control Drawings.
- 2. See Specification Section 548 for material requirements.
- 3. For additional material requirements see the Wall Company's General Notes.

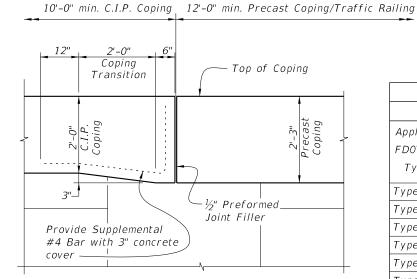
### CONSTRUCTION:

- 1. Walls will be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.
- 4. Refer to Wall Control Drawings of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 5. The Contractor is responsible for controlling water during storm events as needed during construction.
- 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top laver of soil reinforcement, individual reinforcing strips/mesh may be skewed (15° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the guardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be approved by the Engineer.
- 7. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- 8. The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.

- 9. All exposed concrete surfaces will receive a Class 5 Applied Finish Coating in accordance with Specification Section 400. Refer to Typical Section on this sheet and the following notes for limits of applied finish: a. The inside, backside and top of Traffic Railings and Pedestrian/Bicycle
  - Railings. b. Exposed surfaces of coping on top of retaining wall. Other coatings, colors or textures will be applied as required in the Wall Control Drawings.
- 10. For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6" below final ground line.
- 11. Drive piles located within the soil volume prior to construction of the retaining wall, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shafts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 459.
- 12. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- 13. Steps in leveling pads will occur at MSE Wall panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- ground line.
- 15. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 16. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 17. Work this Index with Index 6100 & 6200 Series.

### SHOP DRAWING REQUIREMENTS:

See Specification Section 548 for shop drawing requirements.

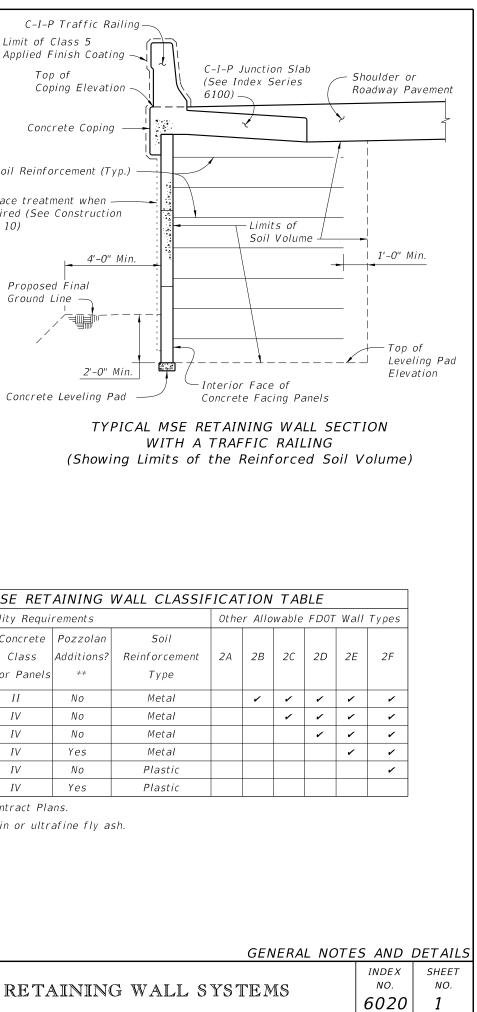


ELEVATION VIEW OF COPING HEIGHT TRANSITION (Railing Not Shown For Clarity) Limit of Class 5

Top of

Soil Reinforcement (Typ.)

Surface treatment when required (See Construction Note 10)



	FDOT	MSE RET	AINING	W
	Dura	bility Requi	rements	
Applicable	Concrete	Concrete	Pozzolan	
FDOT Wall	Cover	Class	Additions?	
Туре *	(in.)	for Panels	**	
Type 2A	2	II	No	
Type 2B	2	IV	No	
Type 2C	3	IV	No	
Type 2D	3	IV	Yes	
Type 2E	3	IV	No	
Type 2F	3	IV	Yes	

\* See Data Table in Contract Plans.

\*\* Silica fume, metakaolin or ultrafine fly ash.

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# NOTES

### SPECIFICATIONS:

- 1. General Specifications:
- The Florida Department of Transportation "Standard Specifications for Road and Bridge Construction", Current Edition and Supplements as Amended.
- 2. Design Specifications: a. Florida Department of Transportation (FDOT) "Structures Design Guidelines", Current Edition.

b. American Association of State Highway and Transportation Officials (AASHTO) "LRFD Bridge Design Specifications", Current Edition. c. AASHTO-AGC-ARTBA Task Force 27 (Ground Modification Techniques), "Insitu Soil Improvement Techniques", January 1990.

### DESIGN CRITERIA:

- 1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and FDOT Structures Design Guidelines Section 3.13.2.
- 2. It is the responsibility of the Engineer to determine that the factored bearing pressure shown for the wall does not exceed the factored bearing resistance of the foundation for that specific wall location.
- 3. The Wall Company is responsible for internal stability of the wall. External stability design, including foundation and slope stability, is the responsibility of the Engineer.
- 4. If present, consider in design and analysis and locate manholes and drop inlets as shown on wall elevations.

### SOIL PARAMETERS:

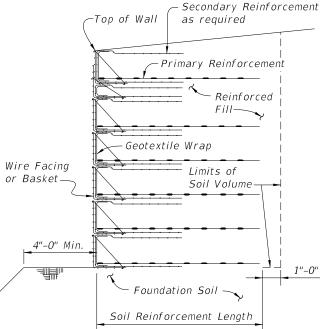
1. See wall control drawings for soil characteristics of foundation material to be used in the design of the wall system. The Contractor must provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site. Provide the values of unit weight, cohesion and internal friction angle in the Shop Drawings.

### MATERIALS:

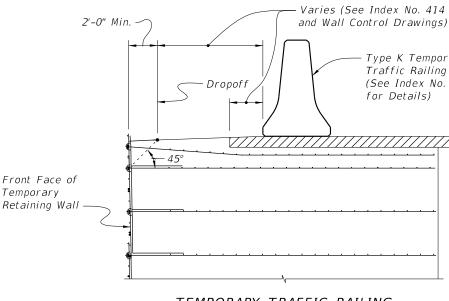
- 1. Provide soil reinforcement in accordance with Specification Section 548.
- 2. For additional material notes, see Wall Company General Notes.

### CONSTRUCTION:

- 1. Walls must be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. Refer to Plan and Elevation sheets of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 4. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor must notify the Engineer to determine what course of action should be taken.
- 5. The Contractor is responsible for gradually deflecting upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.



# TYPICAL RETAINING WALL SECTION (Showing Limits of the Reinforced Soil Volume)





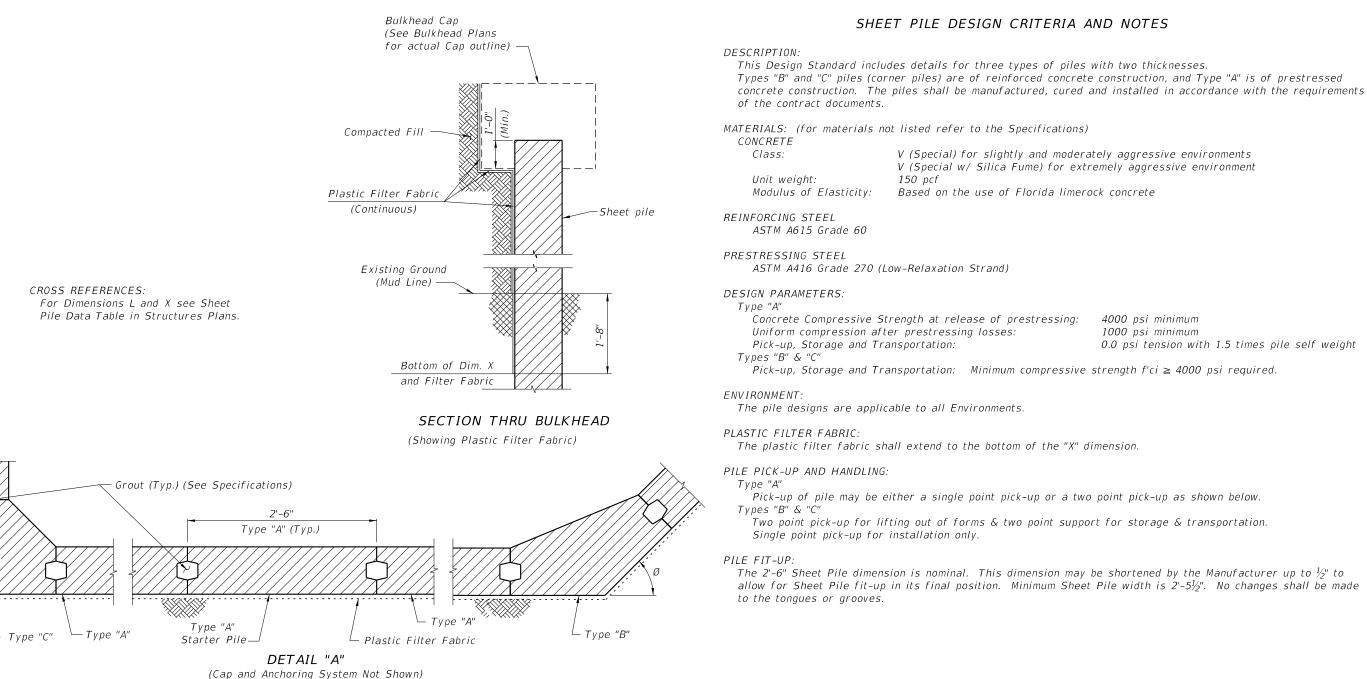
# TEMPORARY MSE RETAINING

1"-0" Min.

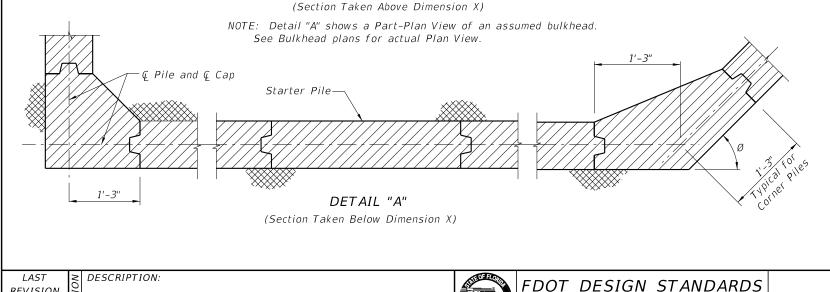
Type K Temporary Traffic Railing (See Index No. 414 for Details)

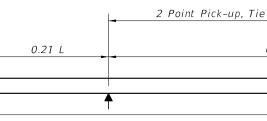
GENERAL NOTES AND DETAILS

	INDEX	SHEET
F WALL SYSTEMS	NO.	NO.
	6030	1



FY 2012/2013





PILE STORAGE AND TRAN

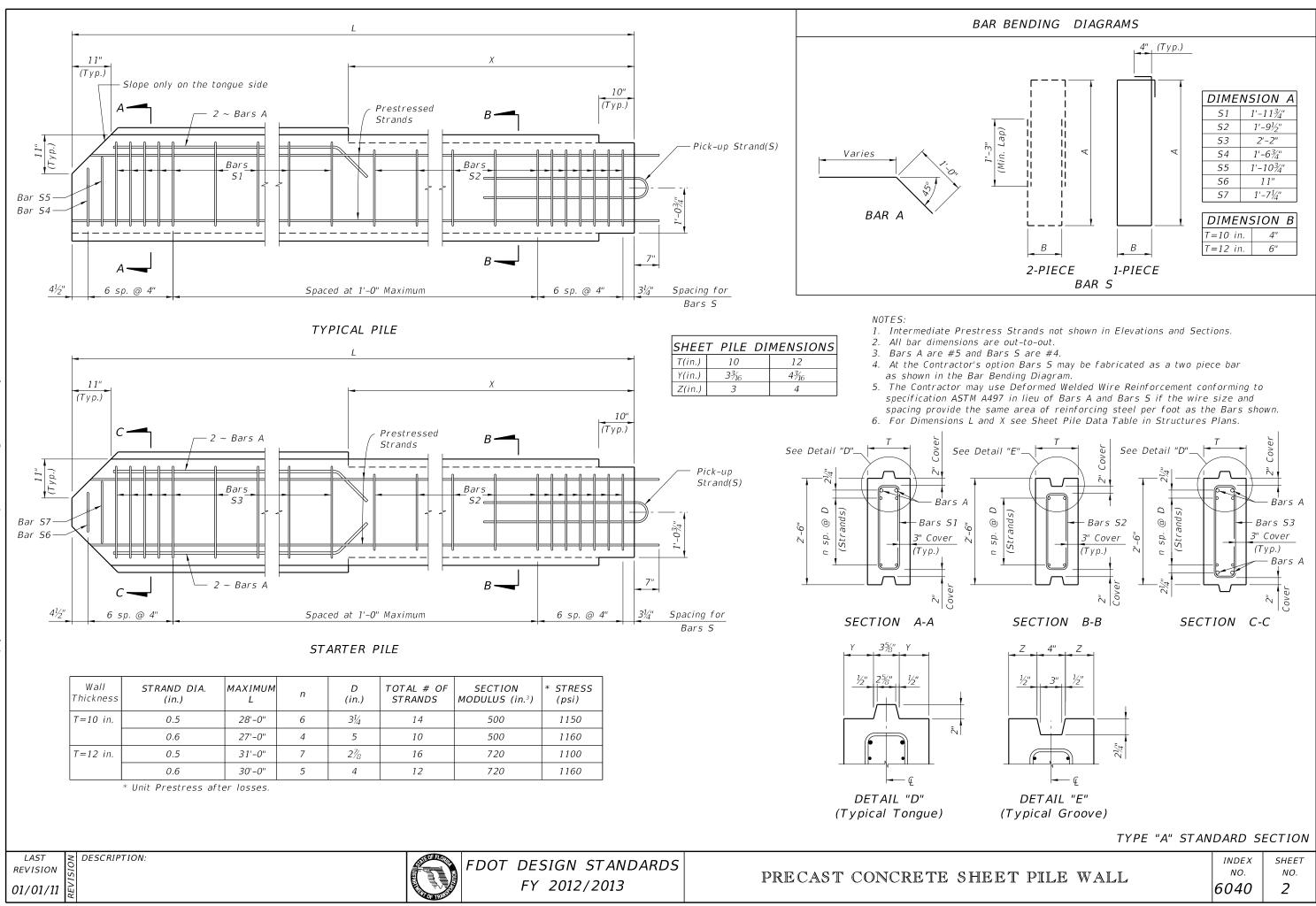
PRECAST CONCRETE SHEE

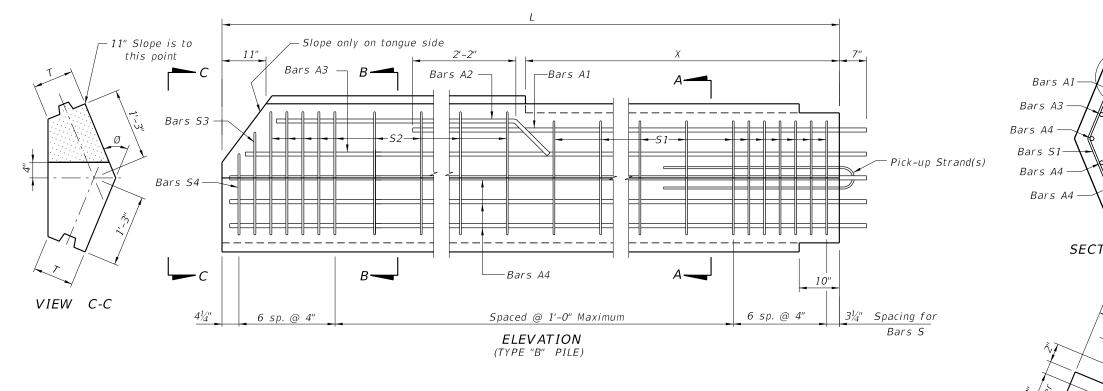
REVISION

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4000 psi minimum 1000 psi minimum 0.0 psi tension with 1.5 times pile self weight

e Down and Support Points	Sing Pick	gle Point up
0.58 L	0.21 L	
L		
	AND DI	ETAILS
ET PILE WALL	index NO. <b>6040</b>	sheet NO. <b>1</b>





BAR BENDING DIAGRAMS																				
		STIR	RRUP I	DIMENS	SIONS	(T =	10")						STIR	RUP I	DIMENS	SIONS	(T =	12")		
Ø	BAR MARK	R1	R:2	R:3	R4	R5	R:6	R7	R·8		Ø	BAR MARK	R1	R2	R3	R4	R·5	R6	R <sub>7</sub>	R8
	51	11¼″	9¾"	1'-6½"	2½"	5"	4¾"	5½"	$4\frac{1}{4}''$	1		<i>S1</i>	11½"	10"	1'-6"	3½"	7"	4¾"	5¾"	6"
30°	S-2	1'-1½"	9¾"	1'-8¾"	2½"	4½"	5½"	5¾"	4¼″		30°	<u>52</u>	1'-1¾"	10"	1'-8¼"	3½"	6½"	5¼"	5¾"	6"
50	53	11¼″	8"	1'-6"	1¼"	5"	4½"	4½"	5"		50	53	11½"	8¼″	1'-5¾"	2"	7"	4¾"	4½"	7¼″
	54	11¼"	4¼″	1'-1¾"	1¾"	5"	3¾"	2½"	$6\frac{1}{4}''$			54	11½"	4"	1'-1¼"	2¼"	7"	3¾"	2½"	8¼″
	S1	11½"	8"	1'-4''	4"	5½"	6½"	8"	4"			S1	1'-0''	8½"	1'-3¼"	5¼"	7½"	6¼"	8½"	5¼″
45°	52	1'-1¾"	8"	1'-5¾"	4"	4½"	7½"	8"	4"		45°	<u>52</u>	1'-2¼"	8½"	1'-5½"	5¼"	6½"	7¼"	8½"	5¼″
	53	11½"	6¾"	1'-4"	2¼"	5½"	6¾"	6¾"	5½"			53	1'-0''	7"	1'-4"	3"	7½"	6¾"	7"	7¼″
	54	11½"	3½"	1'-0''	3"	5½"	5"	3½"	7"			54	1'-0''	3½"	11¾"	3¾"	7½"	5"	3½"	9"
	<i>S1</i>	1'-0''	6"	1'-0¾"	5¼″	6"	7¼″	10¼"	3"			S1	1'-0½"	6¼"	11¾"	7"	8"	6¾"	10¾"	4"
60°	S-2	1'-2"	6"	1'-2¾"	5¼″	4¾"	8¾"	10½"	3"		60°	S-2	1'-2¾"	6¼″	1'-2"	7"	6¾"	8"	10¾"	4"
00	53	1'-0"	4¾"	$1' - 1\frac{1}{2}''$	3¼"	6"	8"	8¾"	$5\frac{1}{4}$ "		00	53	1'-0½"	5"	1'-1½"	4''	8"	8"	9"	7"
	54	1'-0''	2½"	10"	4½"	6"	5¾"	4"	7½"			54	1'-0½"	2½"	9½"	5½"	8"	5½"	4¼″	$9^{1}/4''$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$																				
R2	ø		<u></u>	R2	0	<u>, ч</u>	R2		$\times$	<u>}</u> –	_		-					$\overline{\langle}$	$\sqrt{4}$	5°

DET

Bars A2,-

- This drawing includes detail. and 12" thick sheet pile syst
- The bar configurations show Ø angles between 15° and 75 dimensions may be interpola

Bars Al or A4

S/r

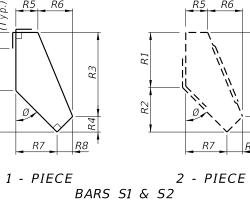
- All bar dimensions are out-t
- Bars A are #8 and Bars S Values for Stirrup Dimensio
- At the Contractor's option Ba
- a minimum lap length of 1'-6 7. If Type "B" pile is used as
- from Dim. X down. Show di 8. If tongue must be on the op,
- S2, S3 and S4 will be the sa 9. For Dimensions L, X and Ang

PRECAST CONCRETE SHEE

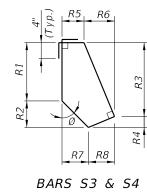
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≥ DESCRIPTION:



 $R_4$ 

R8

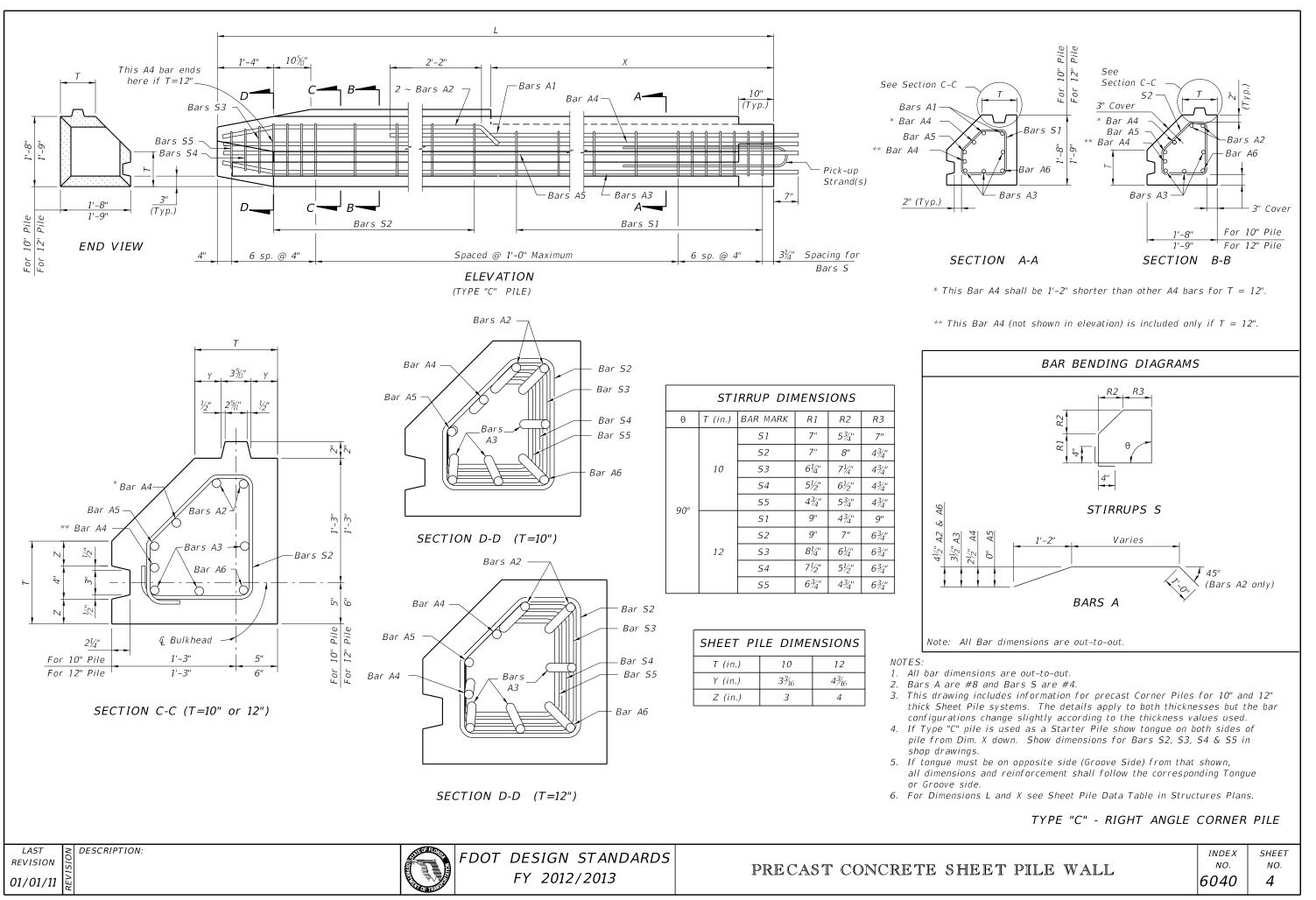
R7

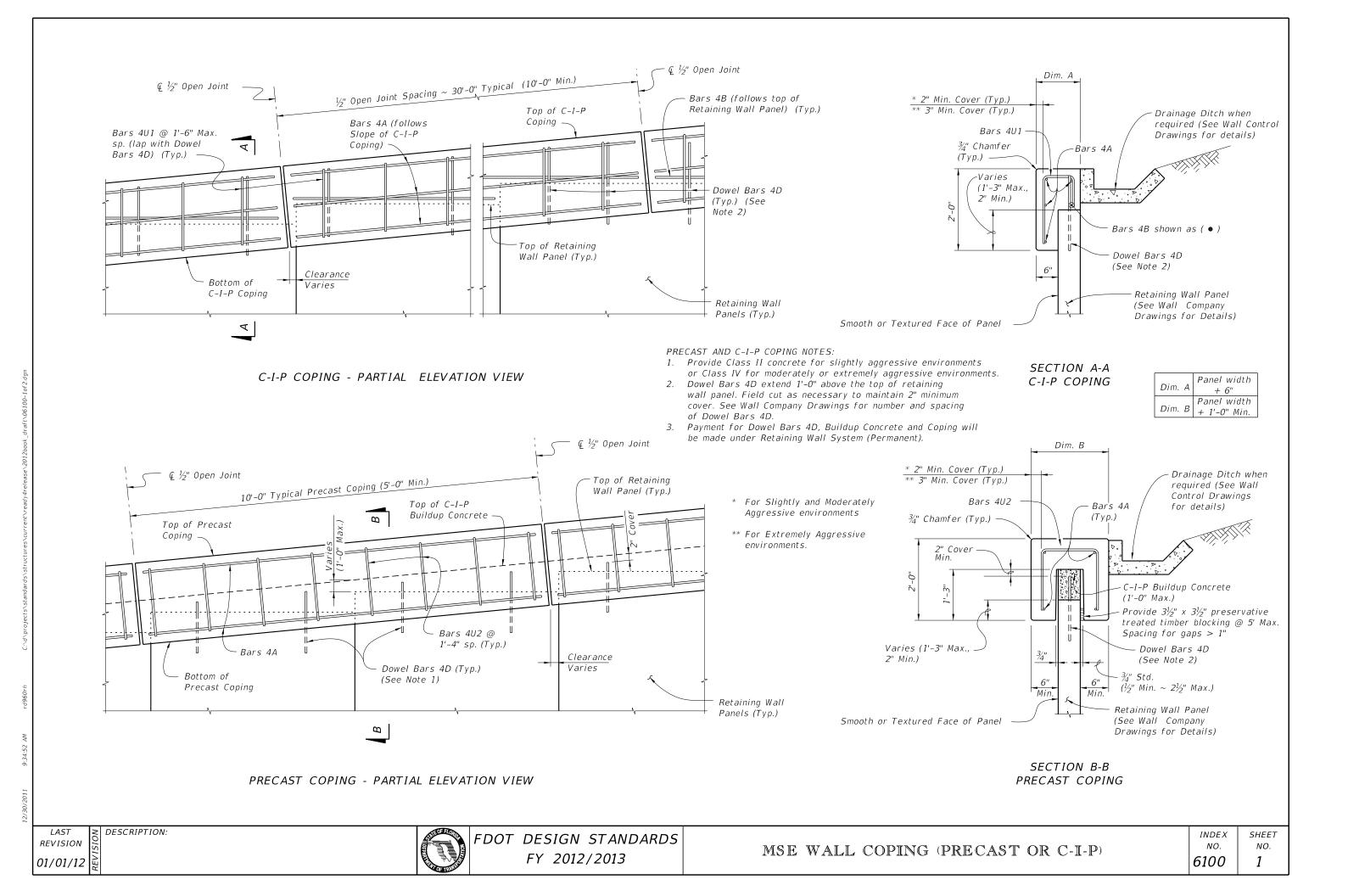


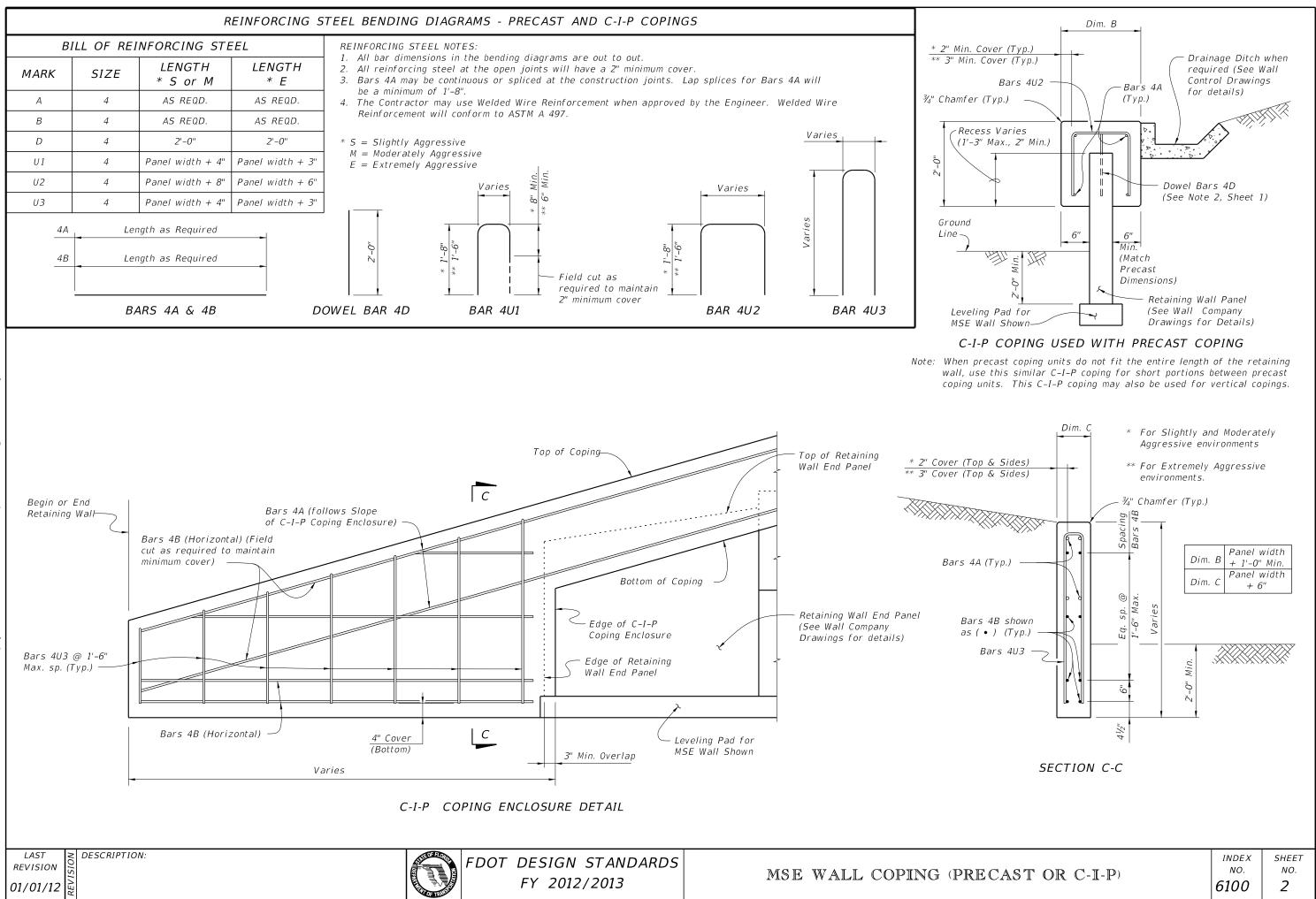


FDOT DESIGN STANDARDS FY 2012/2013

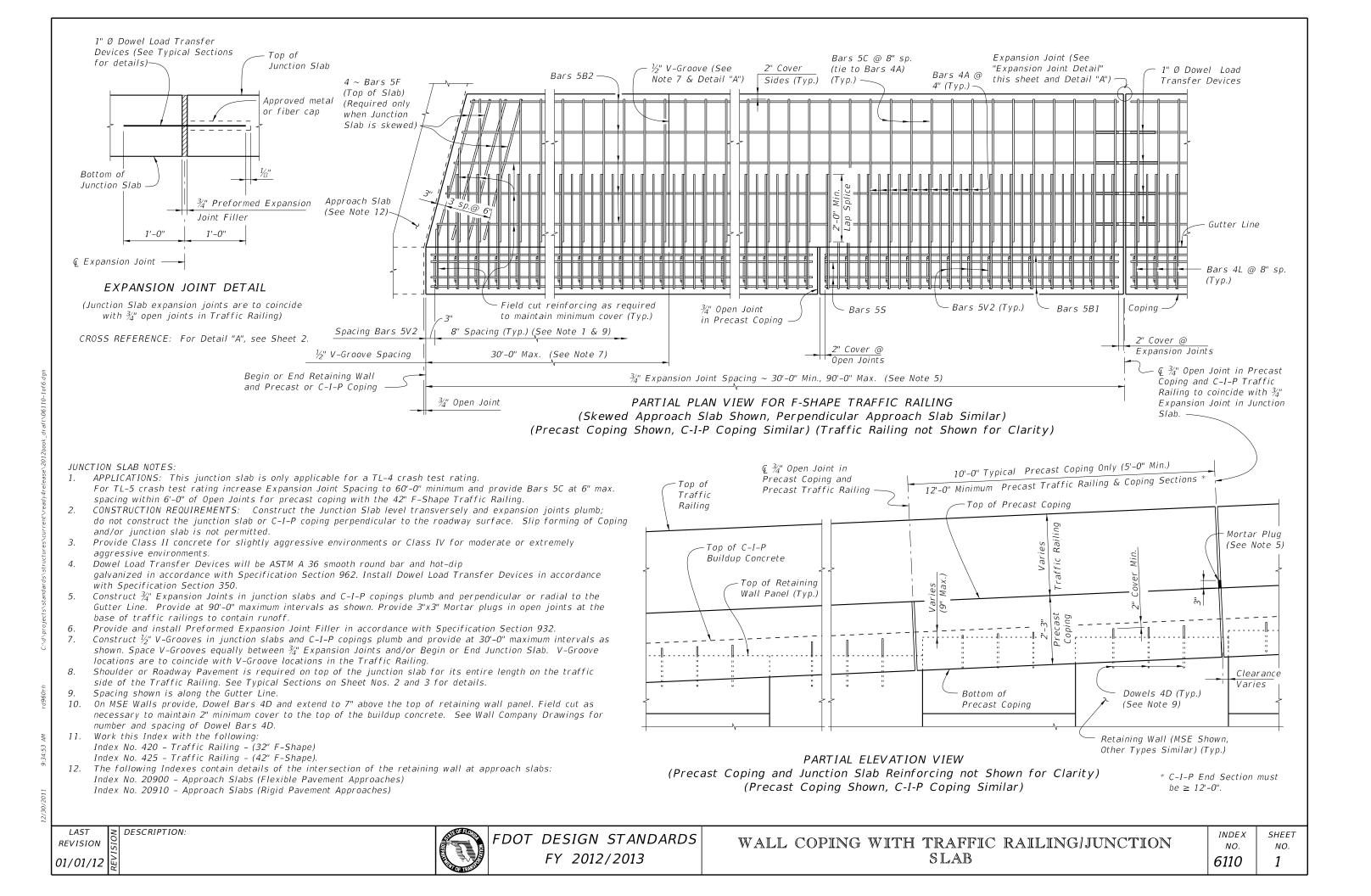
See De	etail "D" —						
Bars A4 Bars A4 Bars A4 Bars A4	Bars A2 Bars A3 — Bars A4 — Bars S2 — Bars A4 — Bars A4 — Bars A4		o Bar o Bar o Bar o Bar o Bar	-s A2 -s A3 -s A4 -s A4 -s A4			
TION A-A	:	SECTI	Typ				
7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7							
	SHEET H	PILE E	DIMENSI	ONS			
	T (in.)	10	1	2			
22.4	Y (in.)	3¾	42	B/16			
$\frac{3''}{4''} + \frac{1}{2'}$ $\frac{2}{2} + \frac{1}{4''} + \frac{2}{1/2'}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$							
s for precast concrete corner piles for 10" tems. The details apply equally to both thicknesses. In in Sections A-A and B-B shall be used for 5°. For Ø angles not shown, the reinforcing bar ted or extrapolated from the stirrup dimensions shown. o-out. are #4. Ins are shown for Ø equal to 30°, 45° & 60° only. ars S may be fabricated as a 2 piece bar with 5°, as shown in Bar Bending Diagrams. a Starter Pile show tongue on both sides of pile mensions for Bars 52, S3 & S4 in shop drawings. posite side from that shown all dimensions and Bars A, ame but opposite hand. gle Ø, see Sheet Pile Data Table in Structures Plans. TYPE "B" - VARIABLE ANGLE CORNER PILE INDEX SHEET							
ET PILE WALI		NO. 6040	NО. <b>З</b>				

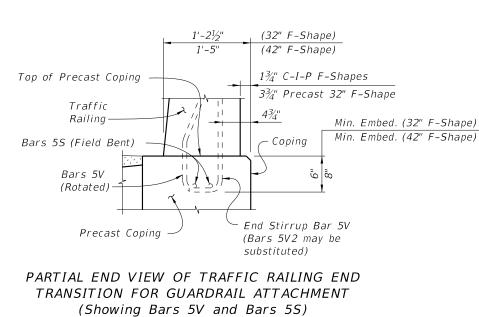






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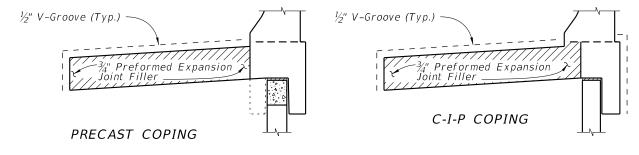


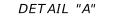
(Precast Coping Shown, C-I-P Coping Similar)

NOTE: See Index No. 420 and Index No. 425, Detail "A" for details.

ESTIMATED QUANTITIES FOR PRECAST COPING							
ITEM	UNIT	QUANTITY					
Concrete (Precast Coping Only)	CY/LF	0.079					
Concrete (Precast Barrier & Coping)	CY/LF	0.165					
Concrete (C-I-P Junction Slab)	CY/LF	0.185					
Reinforcing Steel (Precast Coping & Traffic Railing)	LB/LF	52.67					
Reinforcing Steel (C-I-P Junction Slab) (Typ.)	LB/LF	12.52					
Additional Reinf. @ Expansion Joints	LB	21.36					

(The above concrete quantities are based on a max. superelevation of 6.25% and a 32" F-Shape Traffic Railing.





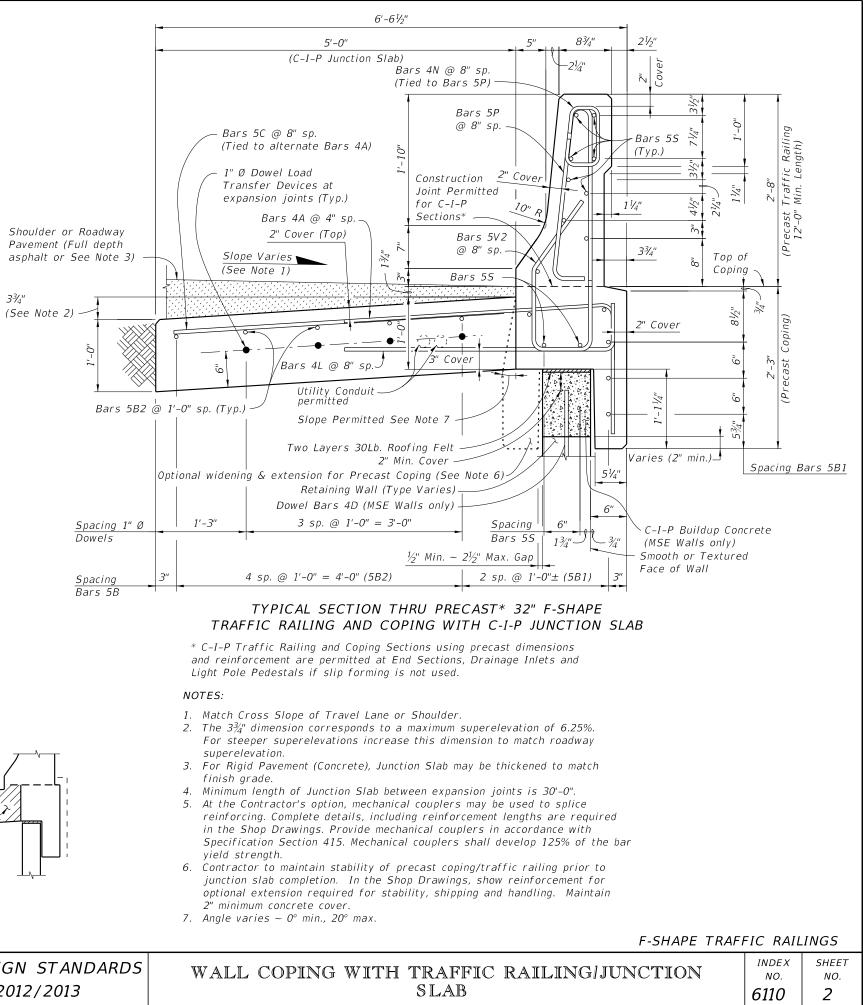
(Showing Locations of  $\frac{1}{2}$ " V-Grooves and  $\frac{3}{4}$ " Preformed Expansion Joint Filler)

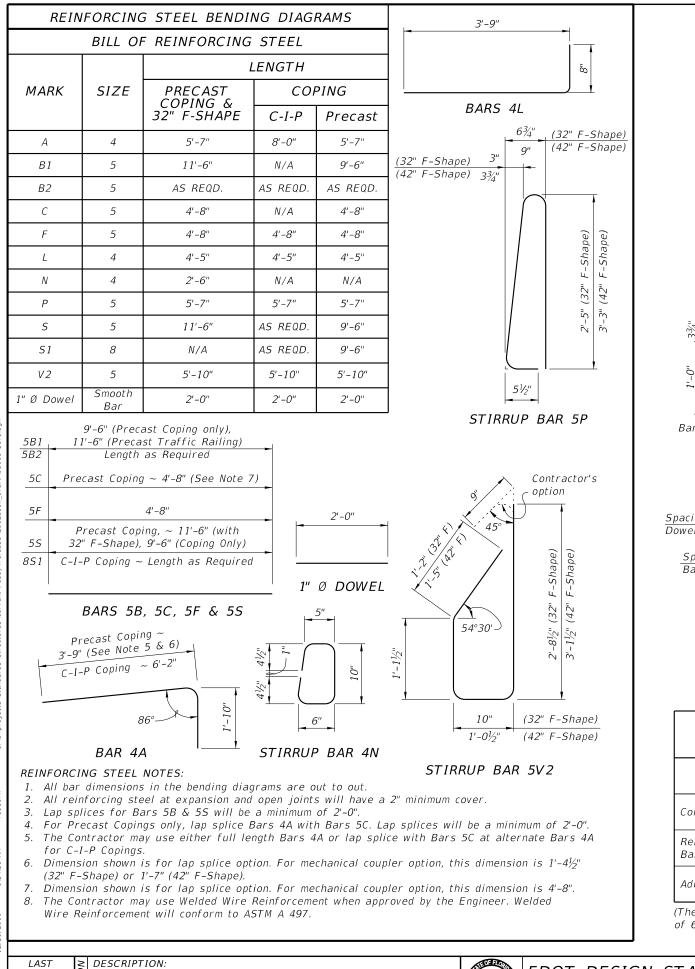
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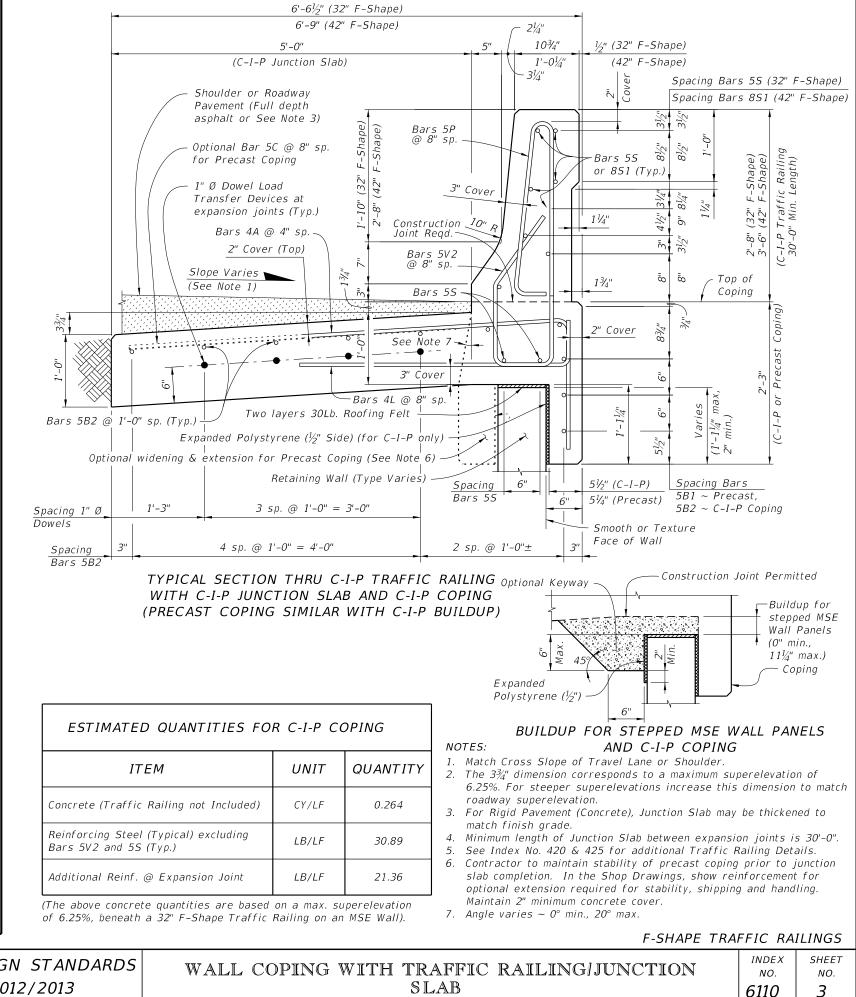


FDOT DESIGN STANDARDS FY 2012/2013

33/4"







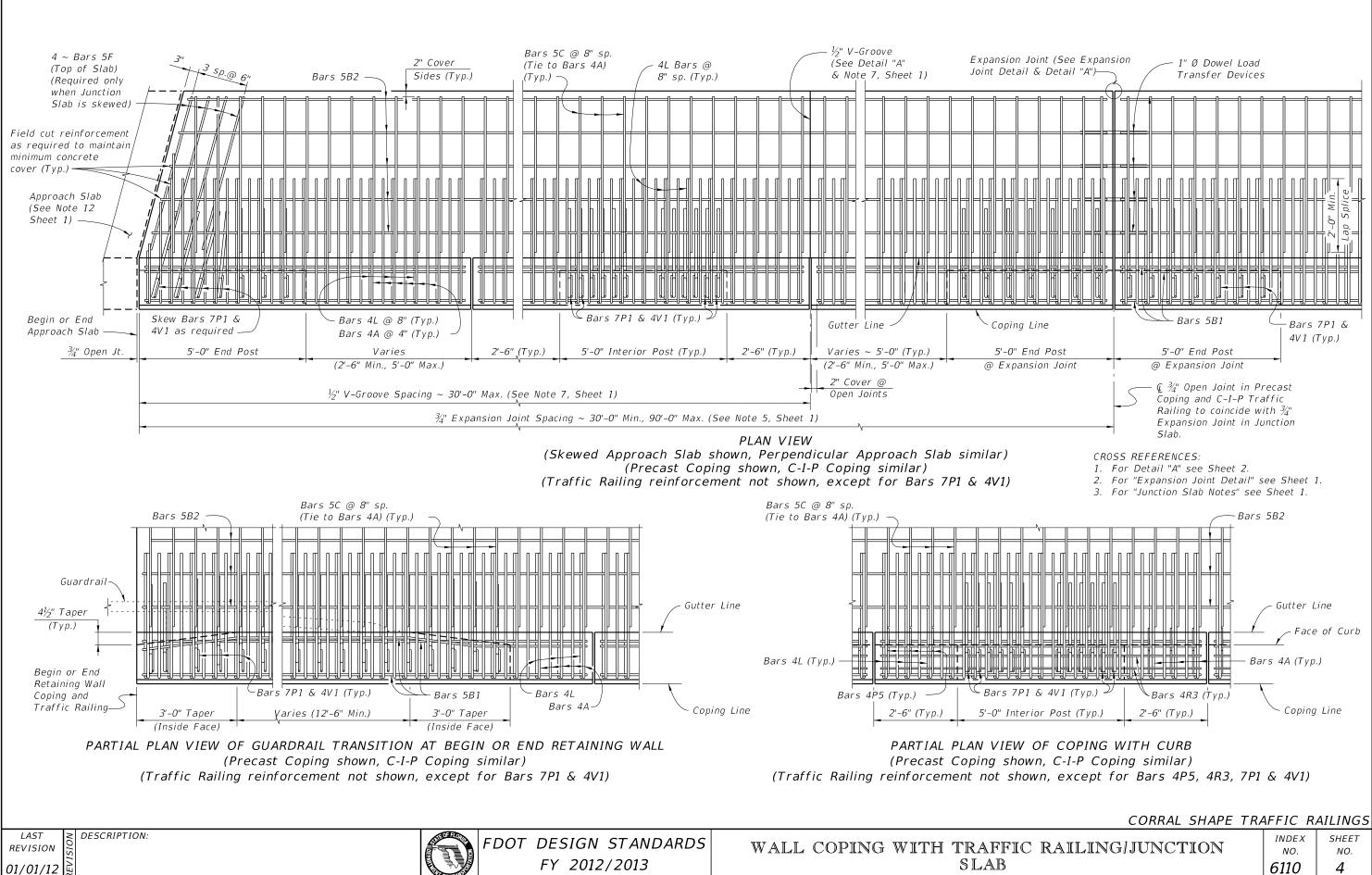
ITEM	UNIT	QUANTIT
Concrete (Traffic Railing not Included)	CY/LF	0.264
Reinforcing Steel (Typical) excluding Bars 5V2 and 5S (Typ.)	LB/LF	30.89
Additional Reinf. @ Expansion Joint	LB/LF	21.36

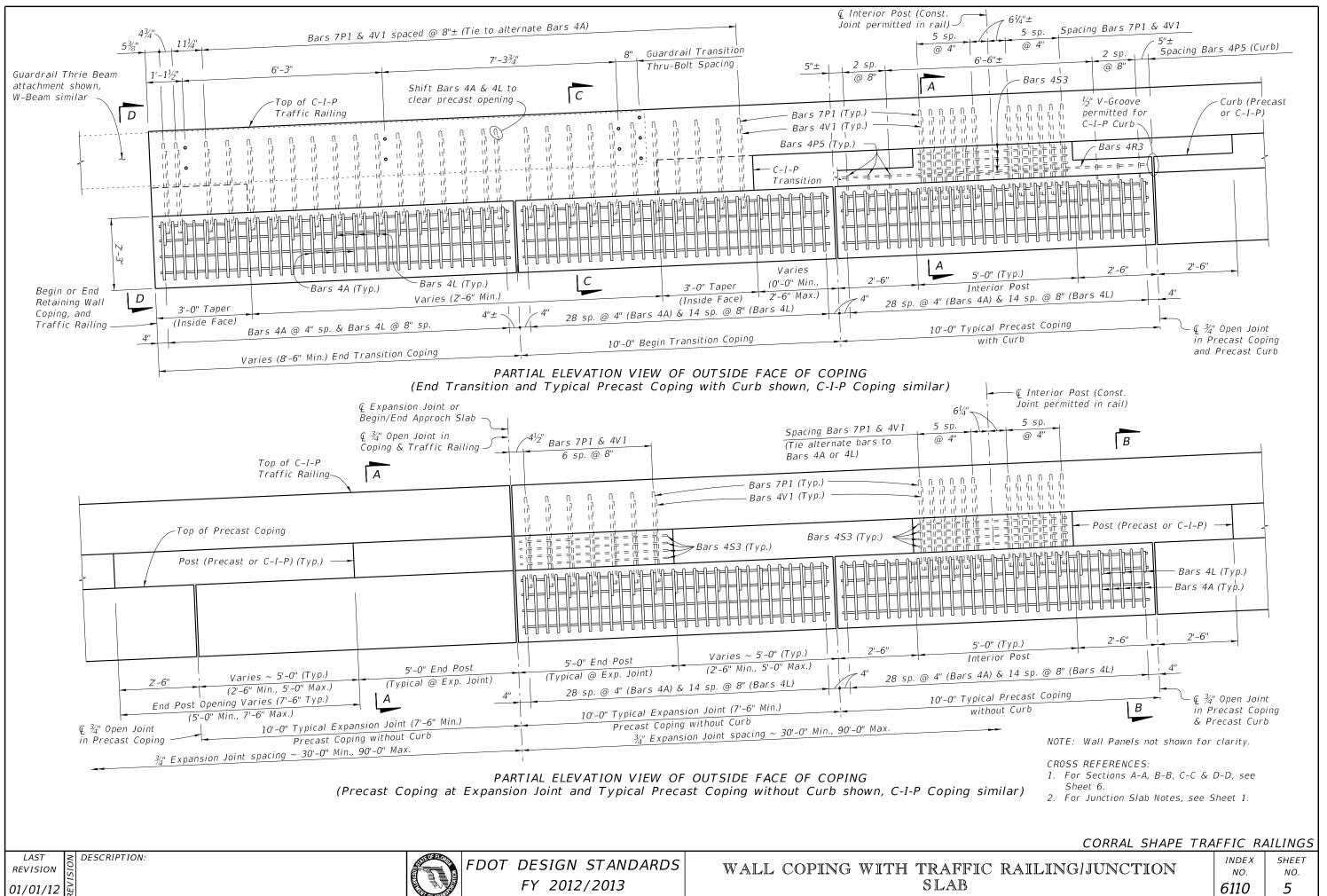
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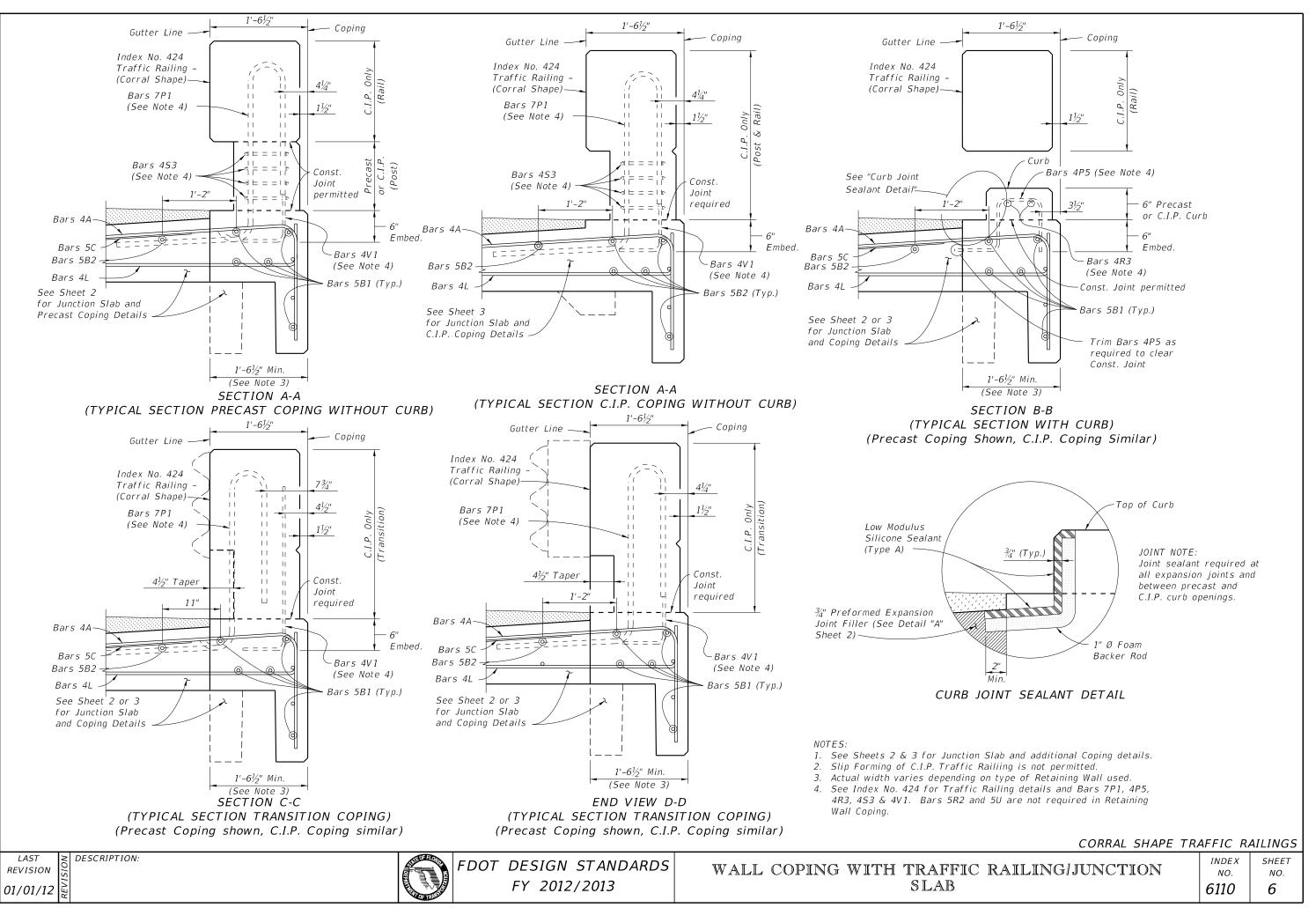
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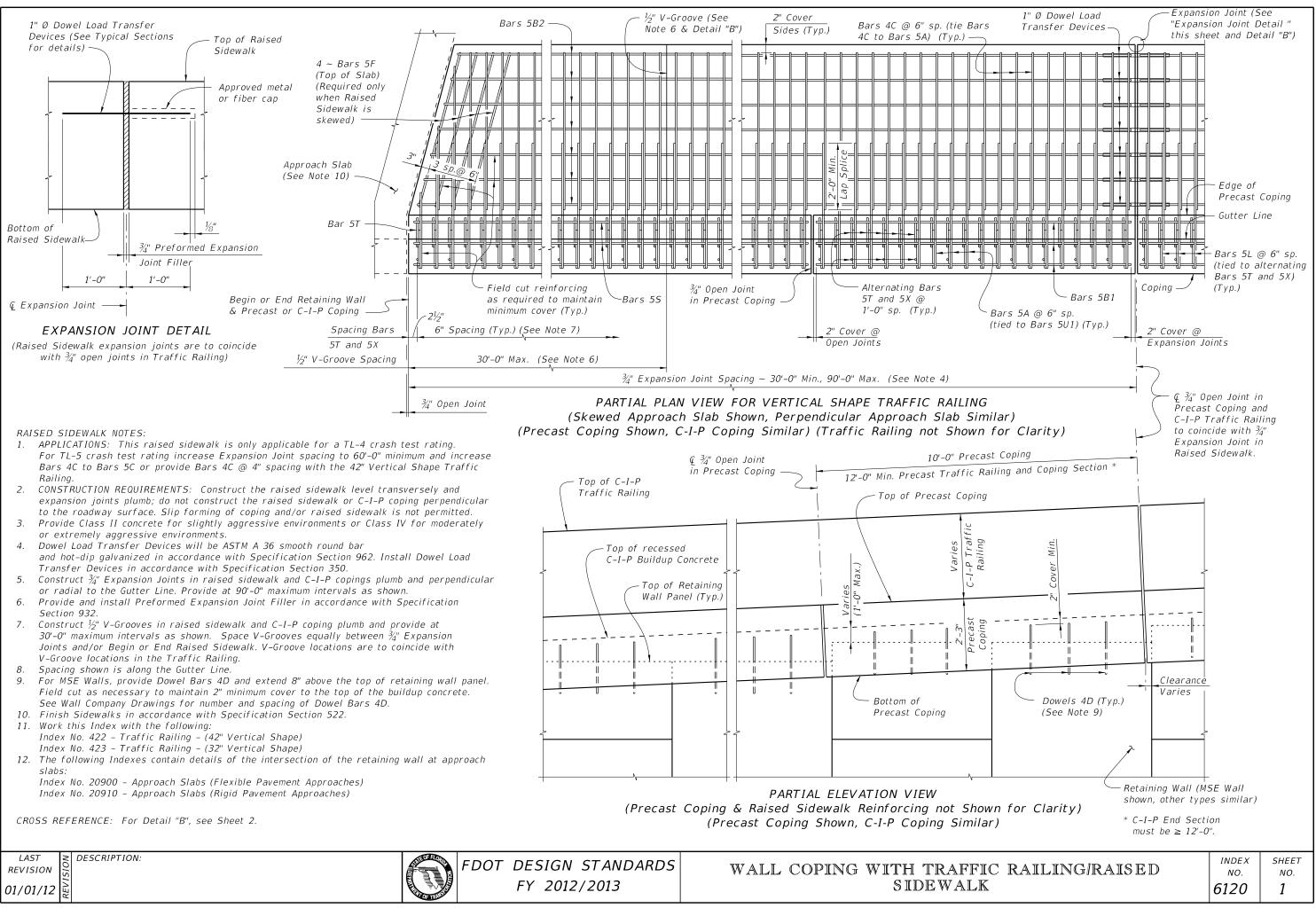


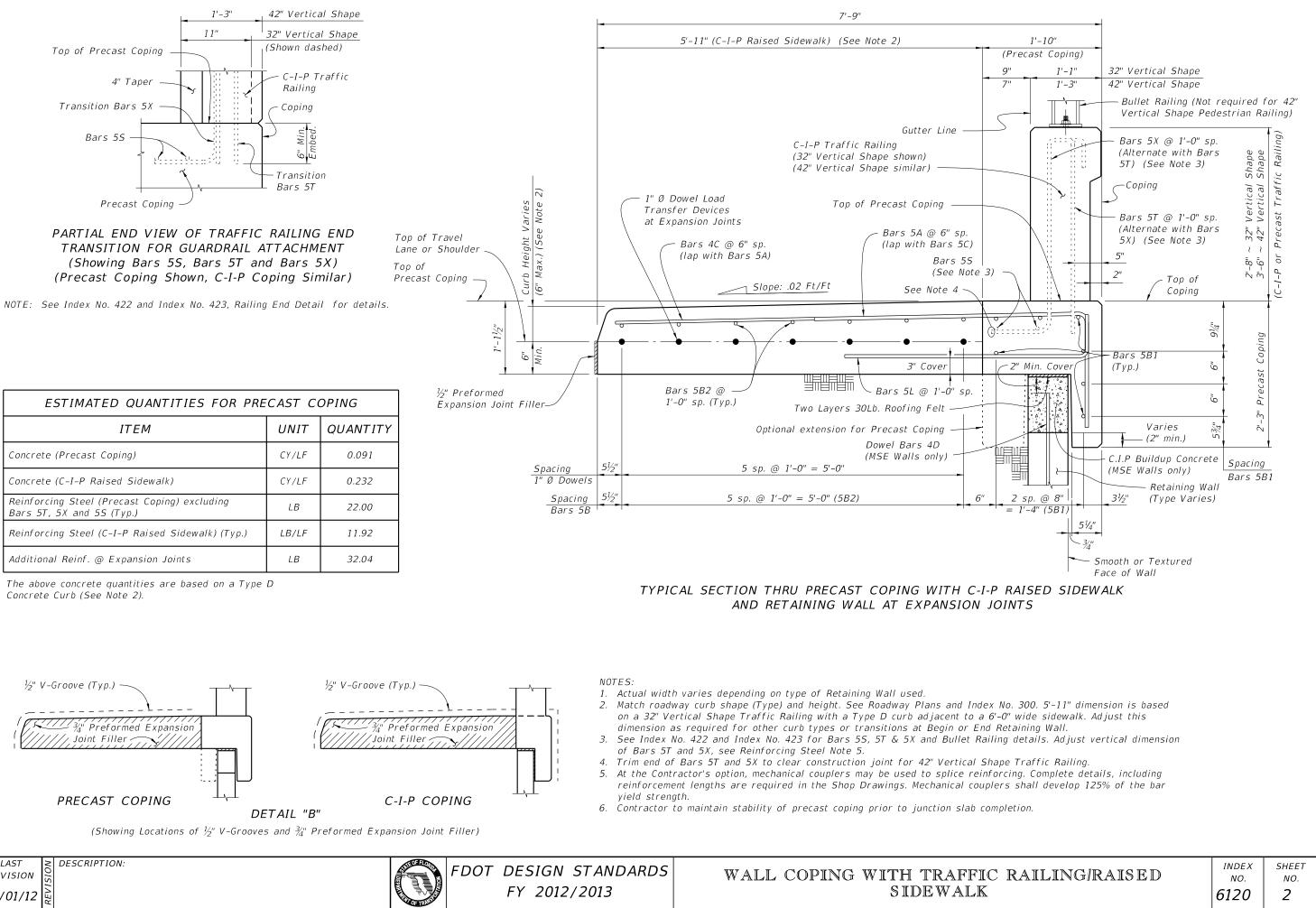
FY 2012/2013

SLAB

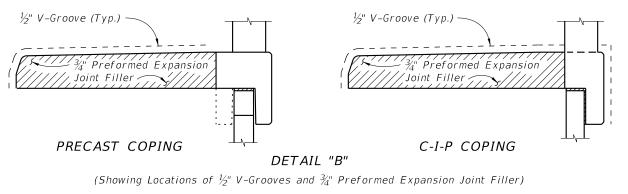


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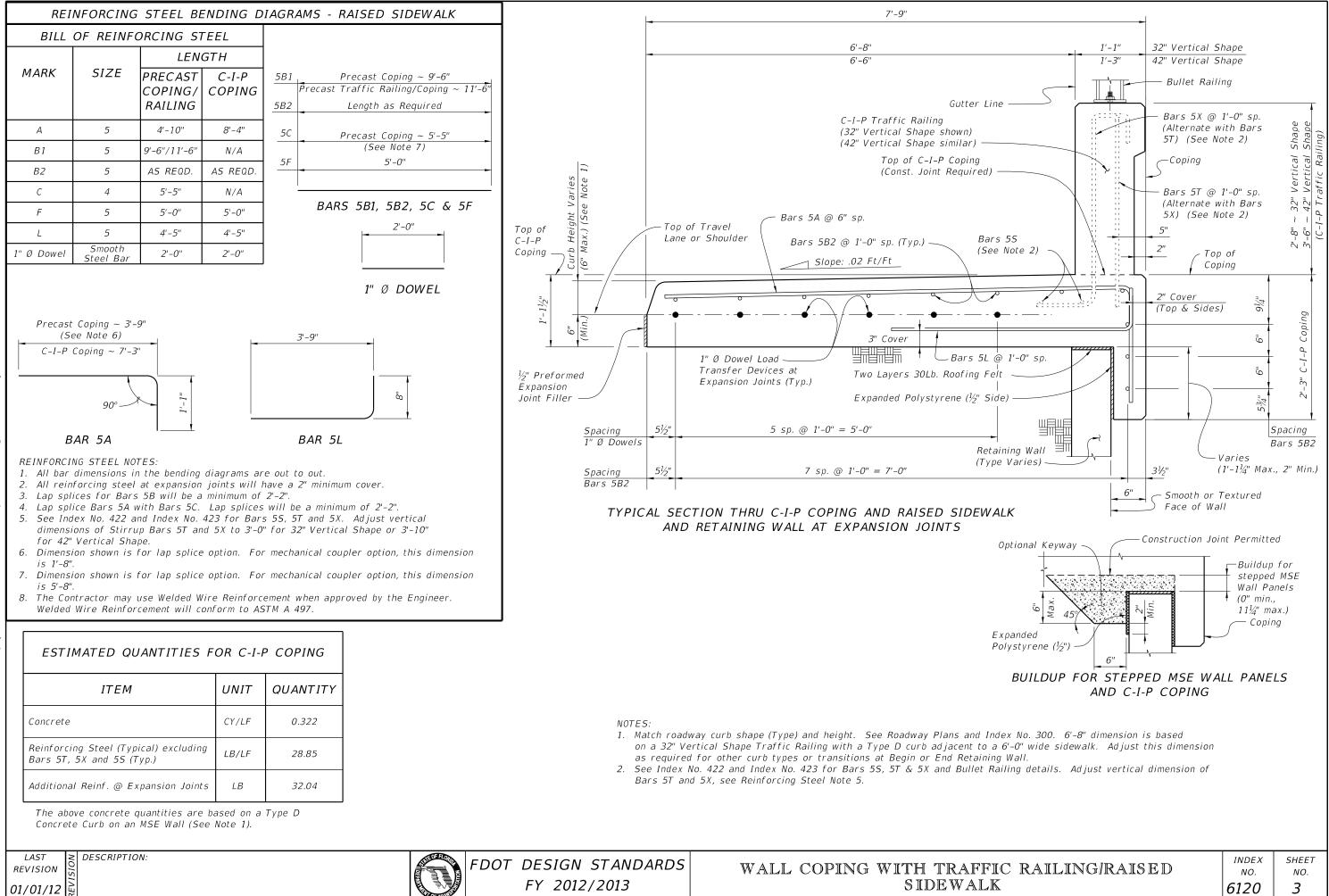


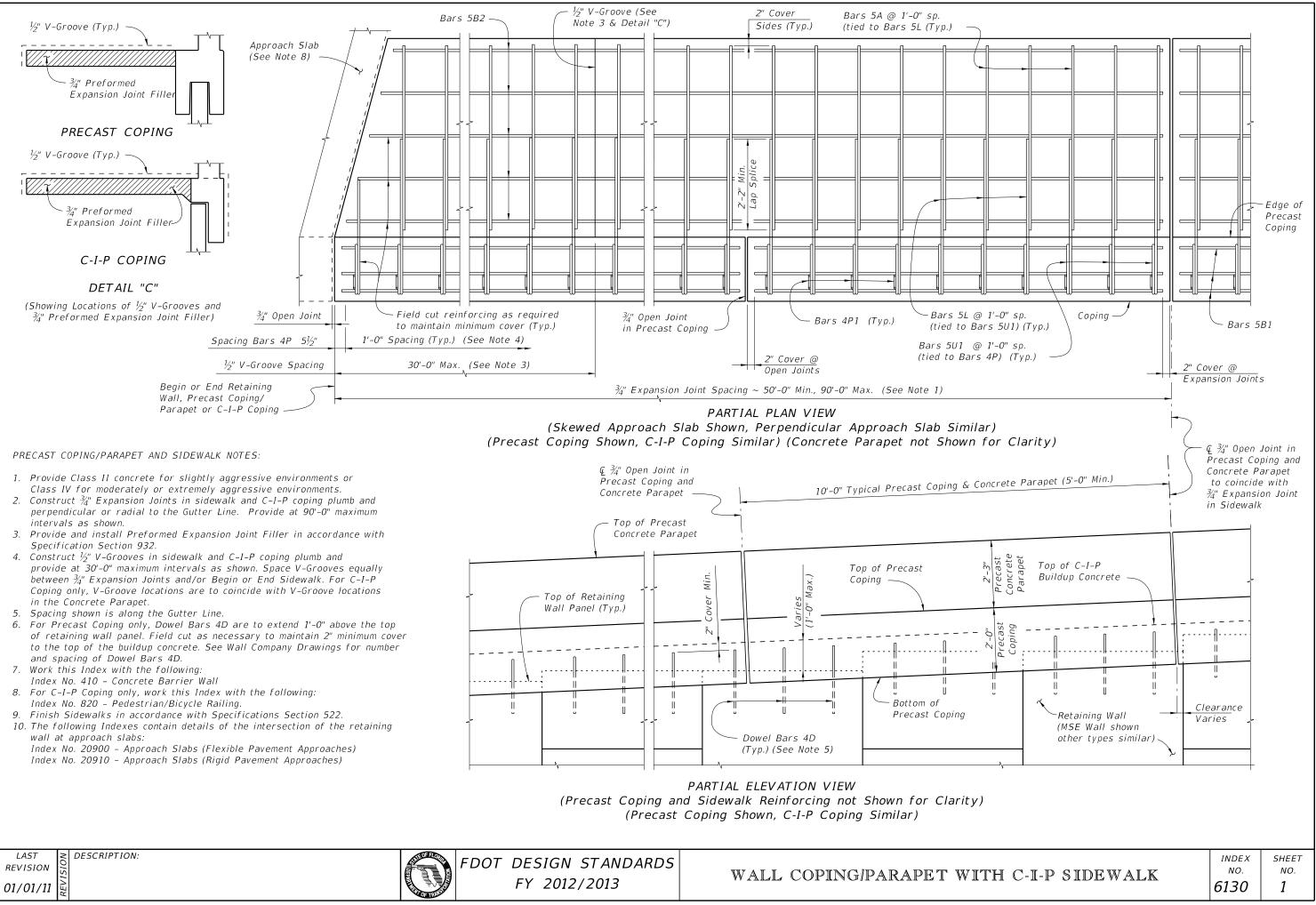
The above concrete quantities are based on a Type D Concrete Curb (See Note 2).



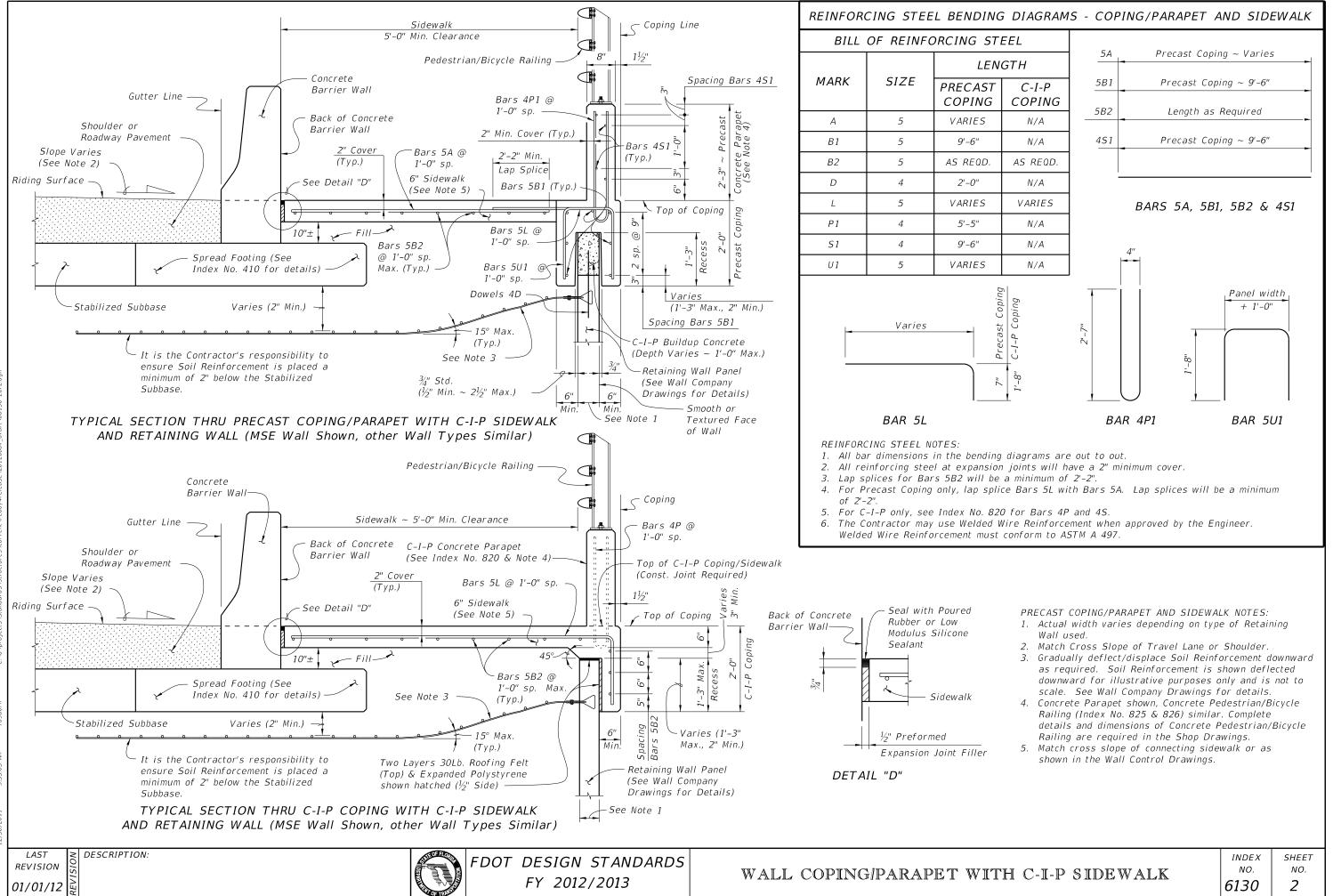
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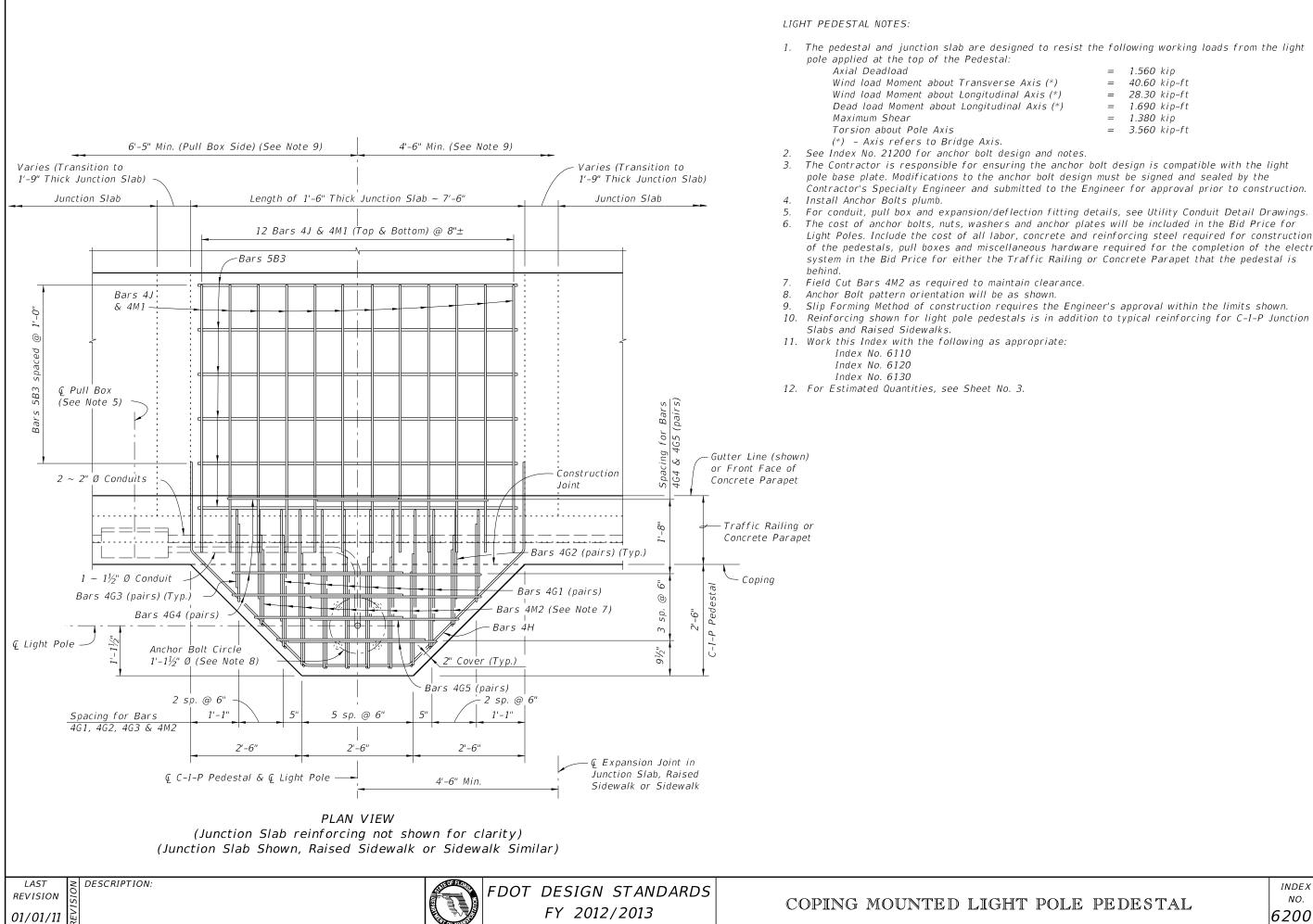




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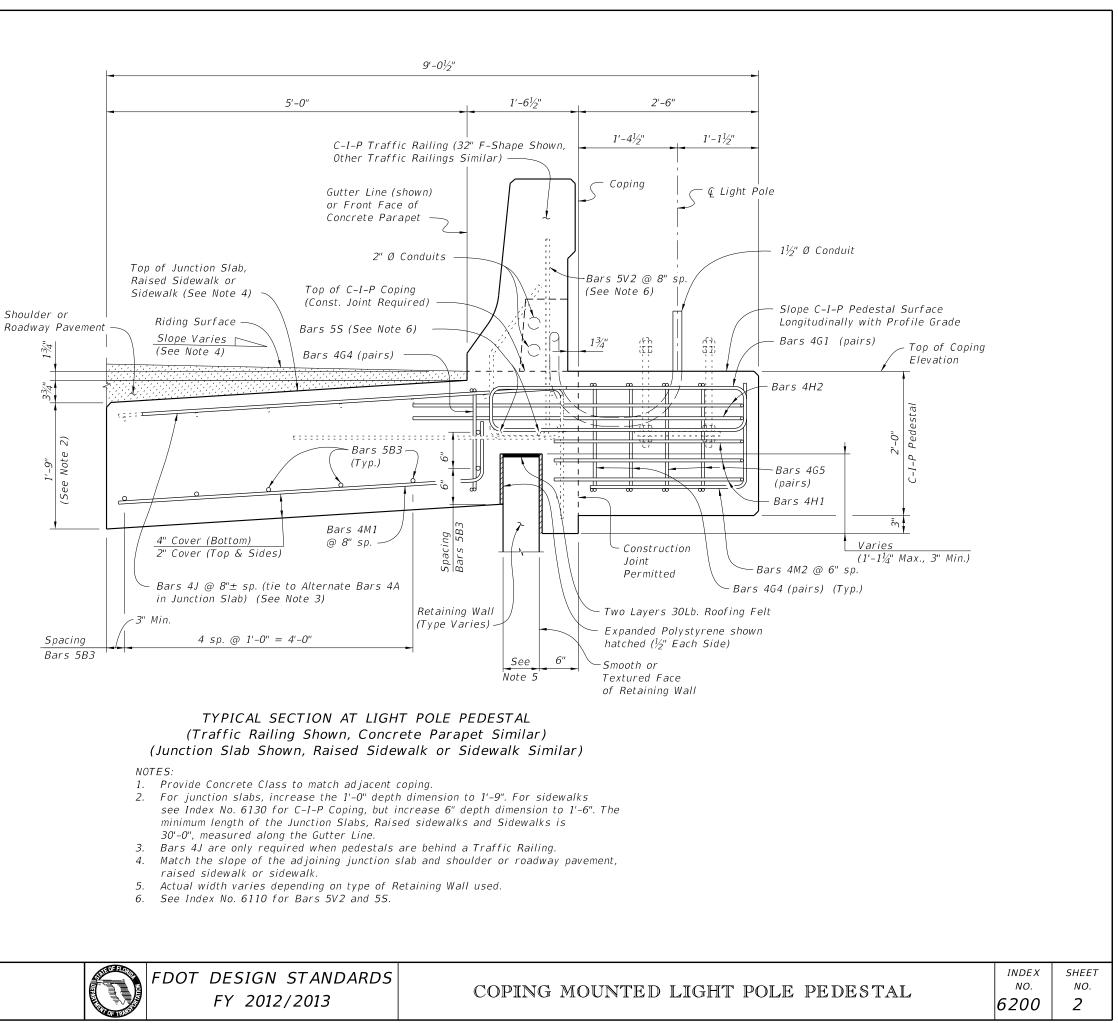


	=	1.560	kip
(*)	=	40.60	kip-ft
(*)	=	28.30	kip-ft
(*)	=	1.690	kip-ft
	=	1.380	kip
	=	3.560	kip-ft

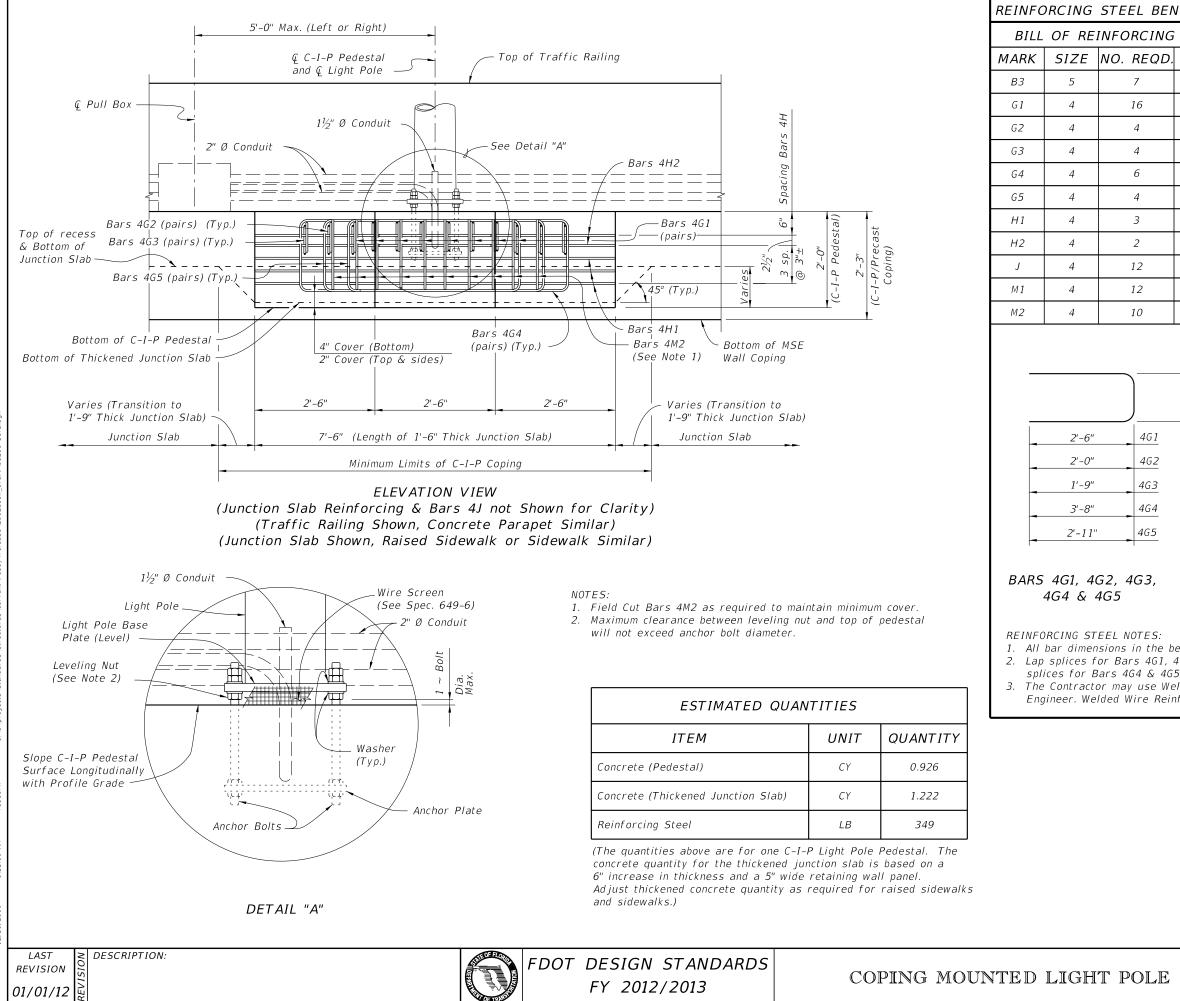
Contractor's Specialty Engineer and submitted to the Engineer for approval prior to construction.

Light Poles. Include the cost of all labor, concrete and reinforcing steel required for construction of the pedestals, pull boxes and miscellaneous hardware required for the completion of the electrical system in the Bid Price for either the Traffic Railing or Concrete Parapet that the pedestal is

		INDEX	SHEET
OLE PEI	PEDESTAL	NO.	NO.
		6200	1







COPING MOUNTED LIGHT POLE PEDESTAL

