Index 20005 Prestressed I-Beam Temporary Bracing

Design Criteria

I

AASHTO LRFD Bridge Design Specifications, 6th Edition; Structures Detailing Manual (SDM); Structures Design Guidelines (SDG)

Design Assumptions and Limitations

Index 20005 depicts notes and details that are schematic for use in the development of shop drawings for temporary I-beam bracing. Use this standard for all superstructures having simply supported Florida-I and non-FDOT standard pretensioned concrete I-Beams. Use this standard with Indexes 20010, 20036, 20045, 20063, 20072, 20078, 20084, 20096, 20199, 20510, 20511 and 20512.

This Index is generally not applicable in its entirety to segmented beams that are erected utilizing temporary shoring and then spliced together using post-tensioning

Companion MicroStation CADD cell 20005 includes the "TABLE OF TEMPORARY BRACING VARIABLES", the "TABLE OF WIND LOAD VARIABLES", the "TABLE OF ASSUMED CONSTRUCTION LOADS (UNFACTORED)", and the "BEAM TEMPORARY BRACING NOTES". These tables are to be completed and included in the plans along with the provided bracing notes. The FDOT Beam Stability MathCAD program may be used to determine the variables to be input into these tables. See **SDG** Chapters 2 and 4 for more information and requirements.

The assumed weight for the finishing machine is left to the discretion of the designer, but suggested total weights for the finishing machine are 10 kips for bridge widths less than 45 feet and 20 kips otherwise.

Plan Content Requirements

In the Structures Plans:

Complete the following "TABLE OF WIND LOAD VARIABLES", "TABLE OF ASSUMED CONSTRUCTION LOADS (UNFACTORED)" and "TABLE OF TEMPORARY BRACING VARIABLES" and include them in the plans for all bridges with prestressed concrete I-beam superstructures. Use additional sheets when the actual number of spans exceeds the capacity of a single plan sheet using the standard tables. Supplemental details and modifications are permitted if special conditions require dimensions, details or notes. However, the tables themselves should not be modified. See Introduction I.3 for more information regarding use of Data Tables.

The forces that are entered into the columns for beam end and intermediate horizontal bracing forces in the 'TABLE OF TEMPORARY BRACING VARIABLES' shall be the horizontal reaction forces at each brace point. Forces should not be resolved into a diagonal component, regardless of any inclination of the actual bracing. These forces are to be used by the Contractor to design bracing members and connections.

If intermediate span braces are not required, enter "N/A" in the "Horizontal Force At Each Intermediate Span Brace" and "Overturning Force At Each Intermediate Span Brace" columns for each span in which intermediate span braces are not required.

Include the 'BEAM TEMPORARY BRACING NOTES' in the plans.

	TABLE	OF WIND LOAD V	ARIABLES	Table Date 1-01-10			
	WIND SPE	WIND SPEED, BASIC (MPH)					
	WIND SPE	WIND SPEED, CONSTRUCTION INACTIVE (MPH)					
	WIND SPE	EED, CONSTRUCTION AC					
	VELOCITY	VELOCITY PRESSURE EXPOSURE COEFFICIENT GUST EFFECT FACTOR					
	GUST EFF						
		OF ASSUMED CON (UNFACTORED)	Table Date 1-01-10]			
	BUILD-UP	· (PLF)		-			
		IGHT (PSF)					
		G MACHINE TOTAL WEI					
	FINISHIN	FINISHING MACHINE FOLAL WEIGHT (KIF) FINISHING MACHINE WHEEL LOCATION BEYOND EDGE OF DECK OVERHANG (IN.)					
	DECK WE	IGHT (PSF)	. ,		-		
		LIVE LOAD (PSF)			-		
	LIVE LOAI	D AT EXTREME DECK E	DGE (PLF)				
				•			
L _B , SPAN MAXIMUM NO. UNBRACED LENGTH (FT)	HORIZONTAL FORCE AT EACH BEAM END AND ANCHOR BRACE (KIP)	FORCE AT EACH	OVERTURNING FORCE AT EACH B END AND ANCHO BRACE (KIP×FT	R INTERMEDIATE SPAN		BRACE ENDS PRIOR TO CRANE RELEASE?	TOTAL LINES OF BRACINO
						YES/NO	
		1				YES/NO	
		1				YES/NO	
						YES/NO	
Design Standard In beam temporary br. 1. Design the bracing in the 'T ABLE OF T overturning forces perpendicular to th centerline of the bu 2. The horizontal brac OF WIND LOAD VAR Load as listed in tl	ion of the beam sta dex No. 20005 is re- acing: members and connec EMPORARY BRACING given in the Table, r e beam web at mid-1 eam at the top of th re forces have been IABLES. The overtu.	determined by applicati rning brace forces have LOAD VARIABLES' plus	following informati compressive and ter ign bracing members horizontal forces. I assume that overtu ion of the Construct e been determined b the assumed constr	on is provided to a nsile forces equal s and connections Assume that hori urning bracing force tion Inactive Wind by application of th	aid the Co to the ho to be cap zontal br ces are a Load as I ne Constru- in the 'T	ontractor in de orizontal force able of resist acing forces a pplied at the isted in the 'T uction Active V TABLE OF ASS	esign of s given ing the re applie ABLE Vind UMED

Payment

The cost of temporary bracing is incidental to the cost of the prestressed beams it is used with. No separate payment is made.

Example Problem

The following example shows the data required for completion of the Data Table for the Prestressed Beam Temporary Bracing Index No. 20005. This case shows a Florida-I 78 Beam (Index No. 20078). The example assumes a three equal span bridge designed for the following conditions:

Girder Span: 182'-0"

Girder Spacing: 6'-0"

Number of Girder Lines: 7

Deck Thickness: 81/2"

Deck Overhang: 3'-0"

Skew Angle: 45°

Bridge Height: 60'-0"

Construction Inactive Wind Load: 44.0 psf (150 mph reduced by 0.6 to 90 mph)

Construction Active Wind Load (20 MPH): 2.2 psf (girder only), 1.1 psf (bridge with forms in place)

Based on beam stability calculations, (1) intermediate brace point would be sufficient, but the bracing force would be very large. Therefore, the bracing requirements will be calculated based on (2) intermediate brace points.

The maximum unbraced length is: 182'-0'' / 3 = 60'-8''

		TABLE	OF WIND LOAD	VARIABLES	able Date 1-01-10		
		WIND SI	PEED, BASIC (MPH)		150		
			PEED, CONSTRUCTION I	NACTIVE (MPH)	90		
		WIND SI	PEED, CONSTRUCTION A	CTIVE (MPH)	20		
		VELOCIT	Y PRESSURE EXPOSUR	E COEFFICIENT	1.137		
		GUST EF	FECT FACTOR		0.85		
			OF ASSUMED CC 5 (UNFACTORED)	DNSTRUCTION	able Date 1-01-10		
		BUILD-U	IP (PLF)		50		
		FORM W	EIGHT (PSF)		20		
		FINISHI	NG MACHINE TOTAL WE	IGHT (KIP)	20		
			NG MACHINE WHEEL LO EDGE OF DECK OVERH		2 ¹ / ₂		
		DECK W	DECK WEIGHT (PSF)		113.3		
		LIVE LO	AD (PSF)		20		
		LIVE LO	AD AT EXTREME DECK	EDGE (PLF)	75		
SPAN NO.	L _B , MAXIMUM UNBRACED LENGTH (FT)	HORIZONTAL FORCE AT EACH BEAM END AND ANCHOR BRACE (KIP)	HORIZONTAL FORCE AT EACH INTERMEDIATE SPAN BRACE (KIP)	OVERTURNING FORCE AT EACH BEAM END AND ANCHOR BRACE (KIPXFT)	OVERTURNING FORCE AT EACH INTERMEDIATE SPAN BRACE (KIP×FT)	BRACE ENDS PRIOR TO CRANE RELEASE?	TOTAL LINES OF BRACING
1	60.67	8.69	23.90	35.31	71.95	NO	4
2	60.67	8.69	23.90 35.31		71.95	NO	4
3	60.67	8.69	23.90	35.31	71.95	NO	4
Ba De be 1. De in ove cer 2. Th OF Lo. CO loa	sed on investig sign Standard am temporary . sign the bracin the 'TABLE OF erturning force rpendicular to therline of the e horizontal br WIND LOAD V. ad as listed in NSTRUCTION LI	Index No. 20005 is r bracing: ng members and conn TEMPORARY BRACINC s given in the Table, the beam web at mid beam at the top of l race forces have bee ARIABLES'. The overt. the 'TABLE OF WIND OADS'. It is the Contr	equired. The Table an ections to transfer boti 5 VARIABLES'. Also des non-simultaneously wi -height of the beam, ai the top flange. In determined by applica urning brace forces hai LOAD VARIABLES' plus actor's responsibility t	ing as shown in the 'TA, d following information h compressive and tensi sign bracing members ar th horizontal forces. A nd assume that overturn ation of the Construction ve been determined by a the assumed construct o re-calculate the braci hachine wheel location f	is provided to aid the le forces equal to the d connections to be ca ssume that horizontal ing bracing forces are n Inactive Wind Load as pplication of the Consi on loads shown in the g requirements if the	Contractor in c horizontal forc pable of resist pracing forces applied at the : listed in the ' ruction Active TABLE OF ASS actual constru	lesign of ing the are appli TABLE Wind UMED ction

the exposure period is more than one year; for this case the Contractor shall be re-calculate bracing requirements. 5. Horizontal and overturning forces are factored per the Strength III limit state for construction.