



*Florida Department of Transportation*

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ANANTH PRASAD, P.E.  
SECRETARY

December 31, 2014

Khoa Nguyen  
Director, Office of Technical Services  
Federal Highway Administration  
545 John Knox Road, Suite 200  
Tallahassee, Florida 32303

Re: State Specifications and Estimates Office  
Section **334**  
Proposed Specification: **3340101 Superpave Asphalt Concrete.**

Dear Mr. Nguyen:

We are submitting, for your approval, two copies of the above referenced Supplemental Specification.

The changes are proposed by Timothy Ruelke of the State Materials Office (SMO) to revise the language for consistency with changes proposed for Section 105. The Department is revising the Contractor Quality Control (QC) Plan requirements and deleting the narrative portion of the Contractor QC Plan. This change is being made in conjunction with the implementation of the Materials Acceptance and Certification system (MAC), which is replacing the current Laboratory Information Management (LIMS) application.

Please review and transmit your comments, if any, within two weeks. Comments should be sent via email to SP965DS or daniel.scheer@dot.state.fl.us.

If you have any questions relating to this specification change, please call me at 414-4130.

Sincerely,

Signature on file

Daniel Scheer, P.E.  
State Specifications Engineer

DS/dt

Attachment

cc: Florida Transportation Builders' Assoc.  
State Construction Engineer

**SUPERPAVE ASPHALT CONCRETE.****(REV ~~11-512-22-14~~)**

SUBARTICLE 334-1.1 is deleted and the following substituted:

**334-1.1 General:** Construct a Superpave Asphalt Concrete pavement with the type of mixture specified in the Contract Documents, or when offered as alternates, as selected. Superpave mixes are identified as Type SP-9.5, Type SP-12.5 or Type SP-19.0.

*Obtain Superpave Asphalt Concrete from a plant that is currently on the Department's Production Facility Listing. Producers seeking inclusion on the list shall meet the requirements of Section 105. Producers must meet the requirements of Section 320 for plant and equipment. Meet and the general construction requirements of Section 330, except as modified herein, including the provision for Quality Control (QC) Plans and QC Systems as specified in Section 105.*

SUBARTICLE 334-2.3.3 is deleted and the following substituted:

**334-2.3.3 RAP Stockpile Approval:** Prior to the incorporation of RAP into the asphalt mixture, stockpile the RAP material and obtain approval for the stockpile by one of the following methods:

1. Continuous stockpile: When RAP is obtained from one or multiple sources and is either processed, blended, or fractionated, and stockpiled in a continuous manner, assure an adequate number of test results are obtained for stockpile approval. Test the RAP material for gradation and asphalt content at a minimum frequency of one sample per 1000 tons with a minimum of six test results. Test the RAP material for  $G_{mm}$  (for  $G_{sb}$  determination) at a minimum frequency of one sample per 5000 tons with a minimum of two test results. Based on visual inspection and a review of the test data, the Engineer will determine the suitability of the stockpiled material. In addition, address *the details and specifics of the processing, sampling, testing and actions to be taken* in the *Producer Quality Control (QC) Plan* ~~the details and specifics of the processing, sampling, testing and actions to be taken.~~

2. Non-continuous single stockpile: When an individual stockpile is being constructed, obtain representative samples at random locations and test the RAP material for gradation and asphalt content at a minimum frequency of one sample per 1000 tons with a minimum of six test results. Test the RAP material for  $G_{mm}$  (for  $G_{sb}$  determination) at a minimum frequency of one sample per 5000 tons with a minimum of two test results. Based on visual inspection and a review of the test data, the Engineer will determine the suitability of the stockpiled material. Once the RAP stockpile has been approved, do not add additional material without prior approval of the Engineer.

Determine the asphalt binder content and gradation of the RAP material in accordance with FM 5-563 and FM 1-T 030, respectively. Establish the  $G_{sb}$  of the RAP material by using one of the following methods:

a. Calculate the  $G_{sb}$  value based upon the effective specific gravity ( $G_{se}$ ) of the RAP material, determined on the basis of the asphalt binder content and maximum specific gravity ( $G_{mm}$ ) of the RAP material. The Engineer will approve the estimated asphalt binder absorption value used in the calculation.

b. Measure the  $G_{sb}$  of the RAP aggregate, in accordance with FM 1-T 084 and FM 1-T 085. Obtain the aggregate by using a solvent extraction method.

ARTICLE 334-4 is deleted and the following substituted:

**334-4 ~~Contractor~~ *Producer* Process Control (PC).**

Assume full responsibility for controlling all operations and processes such that the requirements of these Specifications are met at all times. Perform any tests necessary at the plant and roadway for process control purposes. Enter all PC test data into the Department's database. The Engineer will not use these test results in the acceptance payment decision.

Address in the *Producer* QC Plan how PC failures will be handled. When a PC failure occurs, investigate, at a minimum, the production process, testing equipment and/or sampling methods to determine the cause of the failure, and make any necessary changes to assure compliance with these Specifications. Obtain a follow up sample immediately after corrective actions are taken to assess the adequacy of the corrections. In the event the follow-up PC sample also fails to meet Specification requirements, cease production of the asphalt mixture until the problem is adequately resolved to the satisfaction of the QC Manager.

SUBARTICLE 334-5.4 is deleted and the following substituted:

**334-5.4 QC Sampling and Testing:** Obtain all samples randomly as directed by the Engineer.

Should the Engineer determine that the QC requirements are not being met or that unsatisfactory results are being obtained, or should any instances of falsification of test data occur, ~~approval~~ *acceptance* of the ~~Contractor's~~ *Producer's* QC Plan will be suspended and production will be stopped.

**334-5.4.1 Lost or Missing Verification/Resolution Samples:** In the event that any of the Verification and/or Resolution samples that are in the custody of the Contractor are lost, damaged, destroyed, or are otherwise unavailable for testing, the minimum possible pay factor for each quality characteristic as described in 334-8.2 will be applied to the entire LOT in question, unless called for otherwise by the Engineer. Specifically, if the LOT in question has more than two sublots, the pay factor for each quality characteristic will be 0.55. If the LOT has two or less sublots, the pay factor for each quality characteristic will be 0.80. In either event, the material in question will also be evaluated in accordance with 334-5.9.5.

If any of the Verification and/or Resolution samples that are in the custody of the Department are lost, damaged, destroyed or are otherwise unavailable for testing, the corresponding QC test result will be considered verified, and payment will be based upon the Contractor's data.

**334-5.4.2 Plant Sampling and Testing Requirements:** Obtain one random sample of mix per subplot in accordance with 334-5.1.1 as directed by the Engineer. Test the QC split sample for gradation, asphalt binder content and volumetrics in accordance with 334-5.1.1. Complete all QC testing within one working day from the time the samples were obtained.

**334-5.4.3 Roadway Sampling and Testing Requirements:** Obtain five 6 inch diameter roadway cores within 24 hours of placement at random locations as directed by the Engineer within each subplot. Test these QC samples for density ( $G_{mb}$ ) in accordance with

334-5.1.1. Obtain a minimum of three cores per subplot at random locations as identified by the Engineer in situations where the subplot/LOT was closed or terminated before the random numbers were reached or where it is impractical to cut five cores per subplot. Do not obtain cores any closer than 12 inches from an unsupported edge. The Engineer may adjust randomly generated core locations for safety purposes or as the Engineer deems necessary. Maintain traffic during the coring operation; core the roadway, patch the core holes (within three days of coring); and trim the cores to the proper thickness prior to density testing.

Density for the subplot shall be based on the average value for the cores cut from the subplot with the target density being the maximum specific gravity ( $G_{mm}$ ) of the subplot. Once the average density of a subplot has been determined, do not retest the samples unless approved by the Engineer. Ensure proper handling and storage of all cores until the LOT in question has been accepted.

**334-5.4.4 Individual Test Tolerances for QC Testing:** Terminate the LOT if any of the following QC failures occur:

- 1) An individual test result of a subplot for air voids does not meet the requirements of Table 334-5,
- 2) The average subplot density does not meet the requirements of Table 334-5,
- 3) Two consecutive test results within the same LOT for gradation or asphalt binder content do not meet the requirements of Table 334-5,

When a LOT is terminated due to a QC failure, stop production of the mixture until the problem is resolved to the satisfaction of the QC Manager and/or Asphalt Plant Level II technician responsible for the decision to resume production after a QC failure, as identified in [Section 105-8.6.4](#). In the event that it can be demonstrated that the problem can immediately be or already has been resolved, it will not be necessary to stop production. When a LOT is terminated, make all necessary changes to correct the problem. Do not resume production until appropriate corrections have been made. Inform the Engineer of the problem and corrections made to correct the problem. After resuming production, sample and test the material to verify that the changes have corrected the problem. Summarize this information and provide it to the Engineer prior to the end of the work shift when production resumes.

In the event that a QC failure is not addressed as defined above, the Engineer's approval will be required prior to resuming production after any future QC failures.

Address any material represented by a failing test result in accordance with 334-5.9.5. Any LOT terminated under this subarticle will be limited to a maximum Pay Factor of 1.00 (as defined in 334-8.2) for each quality characteristic.

In the event that a  $G_{mm}$  test result differs by more than 0.040 from the mix design  $G_{mm}$ , investigate the causes of the discrepancy and report the findings and proposed actions to the Engineer.

Characteristic	Tolerance <sup>(1)</sup>
Asphalt Binder Content (%)	Target $\pm 0.55$
Passing No. 200 Sieve (%)	Target $\pm 1.50$
Air Voids (%) <a href="#">Fine Graded</a>	2.30 – 6.00
Density (minimum % $G_{mm}$ )	90.00

Table 334-5 Master Production Range	
Characteristic	Tolerance <sup>(1)</sup>
(1). Tolerances for sample size of n = 1 from the verified mix design	
(2). Based on an average of 5 randomly located cores	

SUBARTICLE 334-5.6.3 is deleted and the following substituted:

**334-5.6.3 Resolution Determination:** The Resolution test results (for the property or properties in question) will be compared with the QC test results based on the between-laboratory precision values shown in Table 334-6.

If the Resolution *laboratory test* results compare favorably with all of the QC results, then acceptance and payment for the LOT will be based on the QC results, and the Department will bear the costs associated with Resolution testing. No additional compensation, either monetary or time, will be made for the impacts of any such testing.

If the Resolution *laboratory test* results do not compare favorably with all of the QC results, then acceptance and payment for the LOT will be based on the Resolution test data for the LOT, and the costs of the Resolution testing will be deducted from monthly estimates. No additional time will be granted for the impacts of any such testing. In addition, in the event that the application of the Resolution test *data* results in a failure to meet the requirements of Table 334-5, address any material represented by the failing test result in accordance with 334-5.9.5.

In the event of an unfavorable comparison between the Resolution test results and QC test results, make the necessary adjustments to assure that future comparisons are favorable.

SUBARTICLE 334-5.9.5 is deleted and the following substituted:

**334-5.9.5 Defective Material:** Assume responsibility for removing and replacing all defective material placed on the project, at no cost to the Department.

As an exception to the above and upon approval of the Engineer, obtain an engineering analysis *in accordance with Section 6* by an independent laboratory (as approved by the Engineer) to determine the disposition of the material. The engineering analysis must be signed and sealed by a Professional Engineer licensed in the State of Florida.

The Engineer may determine that an engineering analysis is not necessary or may perform an engineering analysis to determine the disposition of the material.

Any material that remains in place will be accepted with a CPF as determined by 334-8, or as determined by the Engineer.

If the defective material is due to a gradation, asphalt binder content or density failure, upon the approval of the Engineer the Contractor may perform delineation tests on roadway cores in lieu of an engineering analysis to determine the limits of the defective material that may require removal and replacement. Prior to any delineation testing, all sampling locations shall be approved by the Engineer. All delineation sampling and testing shall be monitored and verified by the Engineer. For materials that are defective due to air voids, an engineering analysis is required.

When evaluating defective material by engineering analysis or delineation testing, at a minimum, evaluate all material located between passing QC, PC or IV test results. Exceptions to this requirement shall be approved by the Engineer.

ARTICLE 334-8 is deleted and the following substituted:

### 334-8 Basis of Payment.

**334-8.1 General:** Price and payment will be full compensation for all the work specified under this Section (including the applicable requirements of Sections 320 and 330).

For materials accepted in accordance with 334-5, based upon the quality of the material, a pay adjustment will be applied to the bid price of the material as determined on a LOT by LOT basis. The pay adjustment will be assessed by calculating a Pay Factor for the following individual quality characteristics: pavement density, air voids, asphalt binder content, and the percentage passing the No. 200 and No. 8 sieves. The pay adjustment will be computed by multiplying a *Composite Pay Factor* (CPF) for the LOT by the bid price per ton. ~~Perform all calculations using the latest version of the Department's Asphalt Plant Worksheet.~~

### 334-8.2 Pay Factors:

**334-8.2.1 Partial LOTs:** For Partial LOTs where no random sample is obtained due to insufficient tonnage, a CPF of 1.00 shall be applied.

**334-8.2.2 Two or Less Sublot Test Results:** In the event that two or less sublot test results are available for a LOT, Pay Factors will be determined based on ~~the Small Quantity Pay Table~~. ~~The Small Quantity Pay Table and Pay Factor calculations are determined in accordance with the instructions contained within the Department's Asphalt Plant Worksheet 334-7, using the average of the accumulated deviations from the target value. (Deviations are absolute values with no plus or minus signs.) Use the 1-Test column when there is only one sublot test result and use the 2-Tests column when there are two sublots.~~

<i>Table 334-7</i>		
<i>Small Quantity Pay Table</i>		
<i>Pay Factor</i>	<i>1 Sublot Test Deviation</i>	<i>2 Sublot Test Average Deviation</i>
<i>Asphalt Binder Content</i>		
<i>1.05</i>	<i>0.00-0.23</i>	<i>0.00-0.16</i>
<i>1.00</i>	<i>0.24-0.45</i>	<i>0.17-0.32</i>
<i>0.90</i>	<i>0.46-0.55</i>	<i>0.33-0.39</i>
<i>0.80</i>	<i>&gt;0.55</i>	<i>&gt;0.39</i>
<i>No. 8 Sieve</i>		
<i>1.05</i>	<i>0.00-2.25</i>	<i>0.00-1.59</i>
<i>1.00</i>	<i>2.26-4.50</i>	<i>1.60-3.18</i>
<i>0.90</i>	<i>4.51-5.50</i>	<i>3.19-3.89</i>
<i>0.80</i>	<i>&gt;5.50</i>	<i>&gt;3.89</i>
<i>No. 200 Sieve</i>		
<i>1.05</i>	<i>0.00-0.55</i>	<i>0.00-0.39</i>
<i>1.00</i>	<i>0.56-1.10</i>	<i>0.40-0.78</i>
<i>0.90</i>	<i>1.11-1.50</i>	<i>0.79-1.06</i>
<i>0.80</i>	<i>&gt;1.50</i>	<i>&gt;1.06</i>

<i>Table 334-7 Small Quantity Pay Table</i>		
<i>Pay Factor</i>	<i>1 Sublot Test Deviation</i>	<i>2 Sublot Test Average Deviation</i>
<i>Air Voids (Coarse Mixes)</i>		
<i>1.05</i>	<i>0.00-0.55</i>	<i>0.00-0.39</i>
<i>1.00</i>	<i>0.56-1.10</i>	<i>0.40-0.78</i>
<i>0.90</i>	<i>1.11-2.00</i>	<i>0.79-1.41</i>
<i>0.80</i>	<i>2.01-2.25</i>	<i>1.42-1.59</i>
<i>0.70</i>	<i>2.26-2.50</i>	<i>1.60-1.77</i>
<i>0.55</i>	<i>&gt;2.50</i>	<i>&gt;1.77</i>
<i>Air Voids (Fine Mixes)</i>		
<i>1.05</i>	<i>0.00-0.50</i>	<i>0.00-0.35</i>
<i>1.00</i>	<i>0.51-1.00</i>	<i>0.36-0.71</i>
<i>0.90</i>	<i>1.01-1.70</i>	<i>0.72-1.20</i>
<i>0.80</i>	<i>1.71-2.00</i>	<i>1.21-1.41</i>
<i>0.70</i>	<i>2.01-2.50</i>	<i>1.42-1.77</i>
<i>0.55</i>	<i>&gt;2.50</i>	<i>&gt;1.77</i>
<i>Density (Coarse Mixes) Note<sup>(1)</sup></i>		
<i>1.05</i>	<i>0.00-0.50</i>	<i>0.00-0.35</i>
<i>1.00</i>	<i>0.51-1.00</i>	<i>0.36-0.71</i>
<i>0.95</i>	<i>1.01-1.50</i>	<i>0.72-1.06</i>
<i>0.90</i>	<i>&gt;1.50</i>	<i>&gt;1.06</i>
<i>Density (Fine Graded Mixtures) Note<sup>(1)</sup></i>		
<i>1.05</i>	<i>0.00-0.50</i>	<i>0.00-0.35</i>
<i>1.00</i>	<i>0.51-1.00</i>	<i>0.36-0.71</i>
<i>0.95</i>	<i>1.01-2.00</i>	<i>0.72-1.41</i>
<i>0.90</i>	<i>2.01-3.00</i>	<i>1.42-2.12</i>
<i>0.80</i>	<i>&gt;3.00</i>	<i>&gt;2.12</i>
<i>Notes:</i>		
<i>(1). Each density test result is the average of five cores. The target density for coarse mixes is 94.50 percent of G<sub>mm</sub>. The target density for fine mixes is 93.00 percent of G<sub>mm</sub> (92.00 percent when compaction is limited to the static mode or for layers specified to be one inch thick). When compaction is limited to the static mode, no vibratory mode in the vertical direction will be allowed. Other vibratory modes will be allowed, if approved by the Engineer. In this case, the target density for fine mixes is 92.00 percent of G<sub>mm</sub>.</i>		

**334-8.2.3 Three or More Sublot Test Results:** When three or more subplot test results are available for a LOT, the variability-unknown, standard deviation method will be used to determine the estimated percentage of the LOT that is within the specification limits ~~shown in Table 334-7. The Percent Within Limits (PWL) is determined in accordance with the instructions contained within the Department's Asphalt Plant Worksheet.~~ *The number of significant figures used in the calculations will be in accordance with requirements of AASHTO R11-06, Absolute Method.*

<i>Table 334-7 Specification Limits</i>	
<i>Quality Characteristic</i>	<i>Specification Limits</i>
<i>Passing No. 8 sieve (%)</i>	<i>Target ± 3.1</i>

Table 334-7  
Specification Limits

Quality Characteristic	Specification Limits
Passing No. 200 sieve (%)	Target $\pm 1.0$
Asphalt Content (%)	Target $\pm 0.40$
Air Voids (%)	4.00 $\pm 1.20$
Density (% of $G_{mm}$ ):	93.00 $\pm 2.00, -1.20^{(1)}$

Note (1): If the Engineer (or Contract Documents) limits compaction to the static mode only, or for all one-inch thick lifts, compaction shall be in the static mode. No vibratory mode in the vertical direction will be allowed. Other vibratory modes will be allowed, if approved by the Engineer. In either case, the specification limits will be as follows: 92.00  $\pm 3.00, -1.20\%$  of  $G_{mm}$ . No additional compensation, cost or time, shall be made.

**334-8.2.3.1 Pay Factors (PF):** Pay Factors will be calculated by using the following equation:

$$\text{Pay Factor} = (55 + 0.5 \times \text{PWL}) / 100$$

The PWL is determined in accordance with the instructions contained within the Department's Asphalt Plant Worksheet. **Percent Within Limits:** The percent within limits (PWL) and Pay Factors for the LOT will be calculated as described below. Variables used in the calculations are as follows:

$x$  = individual test value (sublot)

$n$  = number of tests (sublots)

$s$  = sample standard deviation

$\Sigma(x^2)$  = summation of squares of individual test values

$(\Sigma x)^2$  = summation of individual test values squared

$Q_U$  = upper quality index

$USL$  = upper specification limit (target value plus upper specification

limit from Table 334-8)

$Q_L$  = lower quality index

$LSL$  = lower specification limit (target value minus lower specification

limit from Table 334-8)

$P_U$  = estimated percentage below the  $USL$

$P_L$  = estimated percentage above the  $LSL$

(1) Calculate the arithmetic mean ( $\bar{X}$ ) of the test values:

$$\bar{X} = \frac{\sum x}{n}$$

(2) Calculate the sample standard deviation ( $s$ ):

$$s = \sqrt{\frac{n \sum (x^2) - (\sum x)^2}{n(n-1)}}$$

(3) Calculate the upper quality index ( $Q_U$ ):

$$Q_U = \frac{USL - \bar{X}}{s}$$

(4) Calculate the lower quality index ( $Q_L$ ):

$$Q_L = \frac{\bar{X} - LSL}{s}$$

(5) From Table 334-9, determine the percentage of work below the USL ( $P_U$ ).

(6) From Table 334-9, determine percentage of work above the LSL ( $P_L$ ) Note: If USL or LSL is not specified; percentages within (USL or LSL) will be 100.

(7) If  $Q_U$  or  $Q_L$  is a negative number, then calculate the percent within limits for  $Q_U$  or  $Q_L$  as follows: enter Table 334-9 with the positive value of  $Q_U$  or  $Q_L$  and obtain the corresponding percent within limits for the proper sample size. Subtract this number from 100.00. The resulting number is the value to be used in the next step (Step 8) for the calculation of quality level.

(8) Calculate the percent within limits ( $PWL$ ) = ( $P_U + P_L$ ) - 100

(9) Calculate the Pay Factor (PF) for each quality characteristic using the equation given in 334-8.2.3.2.

<i>Table 334-8 Specification Limits</i>	
<i>Quality Characteristic</i>	<i>Specification Limits</i>
<i>Passing No. 8 sieve ( percent)</i>	<i>Target ± 3.1</i>
<i>Passing No. 200 sieve ( percent)</i>	<i>Target ± 1.0</i>
<i>Asphalt Content ( percent)</i>	<i>Target ± 0.40</i>
<i>Air Voids – Coarse Mixes ( percent)</i>	<i>4.00 ± 1.40</i>
<i>Air Voids – Fine Mixes ( percent)</i>	<i>4.00 ± 1.20</i>
<i>Density – Coarse Mixes ( percent of <math>G_{mm}</math>):</i>	<i>94.50 ± 1.30</i>
<i>Density – Fine Mixes ( percent of <math>G_{mm}</math>):</i>	<i>93.00 + 2.00, - 1.20<sup>(1)</sup></i>
<i>Note (1): If the Engineer (or Contract Documents) limits compaction to the static mode only, or for all one-inch thick lifts, compaction shall be in the static mode. No vibratory mode in the vertical direction will be allowed. Other vibratory modes will be allowed, if approved by the Engineer. In either case, the specification limits will be as follows: 92.00 + 3.00, -1.20 percent of <math>G_{mm}</math>. No additional compensation, cost or time, shall be made.</i>	

<i>Table 334-9 Percent Within Limits</i>				
<i>Quality Index</i>	<i>Percent within Limits for Selected Sample Size</i>			
	<i>n = 3</i>	<i>n = 4</i>	<i>n = 5</i>	<i>n = 6</i>
<i>0.00</i>	<i>50.00</i>	<i>50.00</i>	<i>50.00</i>	<i>50.00</i>
<i>0.05</i>	<i>51.38</i>	<i>51.67</i>	<i>51.78</i>	<i>51.84</i>
<i>0.10</i>	<i>52.76</i>	<i>53.33</i>	<i>53.56</i>	<i>53.67</i>
<i>0.15</i>	<i>54.15</i>	<i>55.00</i>	<i>55.33</i>	<i>55.50</i>

<i>Table 334-9</i>				
<i>Percent Within Limits</i>				
<i>Quality Index</i>	<i>Percent within Limits for Selected Sample Size</i>			
	<i>n = 3</i>	<i>n = 4</i>	<i>n = 5</i>	<i>n = 6</i>
<i>0.20</i>	<i>55.54</i>	<i>56.67</i>	<i>57.10</i>	<i>57.32</i>
<i>0.25</i>	<i>56.95</i>	<i>58.33</i>	<i>58.87</i>	<i>59.14</i>
<i>0.30</i>	<i>58.37</i>	<i>60.00</i>	<i>60.63</i>	<i>60.94</i>
<i>0.35</i>	<i>59.80</i>	<i>61.67</i>	<i>62.38</i>	<i>62.73</i>
<i>0.40</i>	<i>61.26</i>	<i>63.33</i>	<i>64.12</i>	<i>64.51</i>
<i>0.45</i>	<i>62.74</i>	<i>65.00</i>	<i>65.84</i>	<i>66.27</i>
<i>0.50</i>	<i>64.25</i>	<i>66.67</i>	<i>67.56</i>	<i>68.00</i>
<i>0.55</i>	<i>65.80</i>	<i>68.33</i>	<i>69.26</i>	<i>69.72</i>
<i>0.60</i>	<i>67.39</i>	<i>70.00</i>	<i>70.95</i>	<i>71.41</i>
<i>0.65</i>	<i>69.03</i>	<i>71.67</i>	<i>72.61</i>	<i>73.08</i>
<i>0.70</i>	<i>70.73</i>	<i>73.33</i>	<i>74.26</i>	<i>74.71</i>
<i>0.75</i>	<i>72.50</i>	<i>75.00</i>	<i>75.89</i>	<i>76.32</i>
<i>0.80</i>	<i>74.36</i>	<i>76.67</i>	<i>77.49</i>	<i>77.89</i>
<i>0.85</i>	<i>76.33</i>	<i>78.33</i>	<i>79.07</i>	<i>79.43</i>
<i>0.90</i>	<i>78.45</i>	<i>80.00</i>	<i>80.62</i>	<i>80.93</i>
<i>0.95</i>	<i>80.75</i>	<i>81.67</i>	<i>82.14</i>	<i>82.39</i>
<i>1.00</i>	<i>83.33</i>	<i>83.33</i>	<i>83.64</i>	<i>83.80</i>
<i>1.05</i>	<i>86.34</i>	<i>85.00</i>	<i>85.09</i>	<i>85.18</i>
<i>1.10</i>	<i>90.16</i>	<i>86.67</i>	<i>86.52</i>	<i>86.50</i>
<i>1.15</i>	<i>97.13</i>	<i>88.33</i>	<i>87.90</i>	<i>87.78</i>
<i>1.20</i>	<i>100.00</i>	<i>90.00</i>	<i>89.24</i>	<i>89.01</i>
<i>1.25</i>	<i>100.00</i>	<i>91.67</i>	<i>90.54</i>	<i>90.19</i>
<i>1.30</i>	<i>100.00</i>	<i>93.33</i>	<i>91.79</i>	<i>91.31</i>
<i>1.35</i>	<i>100.00</i>	<i>95.00</i>	<i>92.98</i>	<i>92.37</i>
<i>1.40</i>	<i>100.00</i>	<i>96.67</i>	<i>94.12</i>	<i>93.37</i>
<i>1.45</i>	<i>100.00</i>	<i>98.33</i>	<i>95.19</i>	<i>94.32</i>
<i>1.50</i>	<i>100.00</i>	<i>100.00</i>	<i>96.20</i>	<i>95.19</i>
<i>1.55</i>	<i>100.00</i>	<i>100.00</i>	<i>97.13</i>	<i>96.00</i>
<i>1.60</i>	<i>100.00</i>	<i>100.00</i>	<i>97.97</i>	<i>96.75</i>
<i>1.65</i>	<i>100.00</i>	<i>100.00</i>	<i>98.72</i>	<i>97.42</i>
<i>1.70</i>	<i>100.00</i>	<i>100.00</i>	<i>99.34</i>	<i>98.02</i>
<i>1.75</i>	<i>100.00</i>	<i>100.00</i>	<i>99.81</i>	<i>98.55</i>
<i>1.80</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>98.99</i>

<i>Table 334-9</i>				
<i>Percent Within Limits</i>				
<i>Quality Index</i>	<i>Percent within Limits for Selected Sample Size</i>			
	<i>n = 3</i>	<i>n = 4</i>	<i>n = 5</i>	<i>n = 6</i>
<i>1.85</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>99.36</i>
<i>1.90</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>99.65</i>
<i>1.95</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>99.85</i>
<i>2.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>99.97</i>
<i>2.05</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.10</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.15</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.20</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.25</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.30</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.35</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.40</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.45</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.50</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.55</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.60</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>
<i>2.65</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>	<i>100.00</i>

**334-8.2.3.2 Pay Factors (PF):** Pay Factors will be calculated by using the following equation:

$$\text{Pay Factor} = (55 + 0.5 \times \text{PWL}) / 100$$

The PWL is determined from Step (8) of 334-8.2.3.1.

**334-8.3 Composite Pay Factor (CPF):** A CPF for the LOT will be calculated based on the individual PFs with the following weighting applied: 35% Density (D), 25% Air Voids ( $V_a$ ), 25% asphalt binder content ( $P_b$ ), 10% Passing No. 200 ( $P_{-200}$ ) and 5% Passing No. 8 ( $P_8$ ). Calculate the CPF by using the following formula:

$$\text{CPF} = [(0.350 \times \text{PF } D) + (0.250 \times \text{PF } V_a) + (0.250 \times \text{PF } P_b) + (0.100 \times \text{PF } P_{-200}) + (0.050 \times \text{PF } P_8)]$$

Where the PF for each quality characteristic is determined in either 334-8.2.2 or 334-8.2.3, depending on the number of subplot tests. Note that the number after each multiplication will be rounded to the nearest 0.01.

The pay adjustment shall be computed by multiplying the CPF for the LOT by the bid price per ton.

**334-8.4 Payment:** Payment will be made under:

Item No. 334- 1- Superpave Asphaltic Concrete - per ton.

**SUPERPAVE ASPHALT CONCRETE.**  
**(REV 12-22-14)**

SUBARTICLE 334-1.1 is deleted and the following substituted:

**334-1.1 General:** Construct a Superpave Asphalt Concrete pavement with the type of mixture specified in the Contract Documents, or when offered as alternates, as selected. Superpave mixes are identified as Type SP-9.5, Type SP-12.5 or Type SP-19.0.

Obtain Superpave Asphalt Concrete from a plant that is currently on the Department's Production Facility Listing. Producers seeking inclusion on the list shall meet the requirements of Section 105. Producers must meet the requirements of Section 320 for plant and equipment and the general construction requirements of Section 330.

SUBARTICLE 334-2.3.3 is deleted and the following substituted:

**334-2.3.3 RAP Stockpile Approval:** Prior to the incorporation of RAP into the asphalt mixture, stockpile the RAP material and obtain approval for the stockpile by one of the following methods:

1. Continuous stockpile: When RAP is obtained from one or multiple sources and is either processed, blended, or fractionated, and stockpiled in a continuous manner, assure an adequate number of test results are obtained for stockpile approval. Test the RAP material for gradation and asphalt content at a minimum frequency of one sample per 1000 tons with a minimum of six test results. Test the RAP material for  $G_{mm}$  (for  $G_{sb}$  determination) at a minimum frequency of one sample per 5000 tons with a minimum of two test results. Based on visual inspection and a review of the test data, the Engineer will determine the suitability of the stockpiled material. In addition, address the details and specifics of the processing, sampling, testing and actions to be taken in the Producer Quality Control (QC) Plan.

2. Non-continuous single stockpile: When an individual stockpile is being constructed, obtain representative samples at random locations and test the RAP material for gradation and asphalt content at a minimum frequency of one sample per 1000 tons with a minimum of six test results. Test the RAP material for  $G_{mm}$  (for  $G_{sb}$  determination) at a minimum frequency of one sample per 5000 tons with a minimum of two test results. Based on visual inspection and a review of the test data, the Engineer will determine the suitability of the stockpiled material. Once the RAP stockpile has been approved, do not add additional material without prior approval of the Engineer.

Determine the asphalt binder content and gradation of the RAP material in accordance with FM 5-563 and FM 1-T 030, respectively. Establish the  $G_{sb}$  of the RAP material by using one of the following methods:

a. Calculate the  $G_{sb}$  value based upon the effective specific gravity ( $G_{se}$ ) of the RAP material, determined on the basis of the asphalt binder content and maximum specific gravity ( $G_{mm}$ ) of the RAP material. The Engineer will approve the estimated asphalt binder absorption value used in the calculation.

b. Measure the  $G_{sb}$  of the RAP aggregate, in accordance with FM 1-T 084 and FM 1-T 085. Obtain the aggregate by using a solvent extraction method.

ARTICLE 334-4 is deleted and the following substituted:

**334-4 Producer Process Control (PC).**

Assume full responsibility for controlling all operations and processes such that the requirements of these Specifications are met at all times. Perform any tests necessary at the plant and roadway for process control purposes. Enter all PC test data into the Department's database. The Engineer will not use these test results in the acceptance payment decision.

Address in the Producer QC Plan how PC failures will be handled. When a PC failure occurs, investigate, at a minimum, the production process, testing equipment and/or sampling methods to determine the cause of the failure, and make any necessary changes to assure compliance with these Specifications. Obtain a follow up sample immediately after corrective actions are taken to assess the adequacy of the corrections. In the event the follow-up PC sample also fails to meet Specification requirements, cease production of the asphalt mixture until the problem is adequately resolved to the satisfaction of the QC Manager.

SUBARTICLE 334-5.4 is deleted and the following substituted:

**334-5.4 QC Sampling and Testing:** Obtain all samples randomly as directed by the Engineer.

Should the Engineer determine that the QC requirements are not being met or that unsatisfactory results are being obtained, or should any instances of falsification of test data occur, acceptance of the Producer's QC Plan will be suspended and production will be stopped.

**334-5.4.1 Lost or Missing Verification/Resolution Samples:** In the event that any of the Verification and/or Resolution samples that are in the custody of the Contractor are lost, damaged, destroyed, or are otherwise unavailable for testing, the minimum possible pay factor for each quality characteristic as described in 334-8.2 will be applied to the entire LOT in question, unless called for otherwise by the Engineer. Specifically, if the LOT in question has more than two sublots, the pay factor for each quality characteristic will be 0.55. If the LOT has two or less sublots, the pay factor for each quality characteristic will be 0.80. In either event, the material in question will also be evaluated in accordance with 334-5.9.5.

If any of the Verification and/or Resolution samples that are in the custody of the Department are lost, damaged, destroyed or are otherwise unavailable for testing, the corresponding QC test result will be considered verified, and payment will be based upon the Contractor's data.

**334-5.4.2 Plant Sampling and Testing Requirements:** Obtain one random sample of mix per subplot in accordance with 334-5.1.1 as directed by the Engineer. Test the QC split sample for gradation, asphalt binder content and volumetrics in accordance with 334-5.1.1. Complete all QC testing within one working day from the time the samples were obtained.

**334-5.4.3 Roadway Sampling and Testing Requirements:** Obtain five 6 inch diameter roadway cores within 24 hours of placement at random locations as directed by the Engineer within each subplot. Test these QC samples for density ( $G_{mb}$ ) in accordance with 334-5.1.1. Obtain a minimum of three cores per subplot at random locations as identified by the Engineer in situations where the subplot/LOT was closed or terminated before the random numbers were reached or where it is impractical to cut five cores per subplot. Do not obtain cores any closer than 12 inches from an unsupported edge. The Engineer may adjust randomly

generated core locations for safety purposes or as the Engineer deems necessary. Maintain traffic during the coring operation; core the roadway, patch the core holes (within three days of coring); and trim the cores to the proper thickness prior to density testing.

Density for the subplot shall be based on the average value for the cores cut from the subplot with the target density being the maximum specific gravity ( $G_{mm}$ ) of the subplot. Once the average density of a subplot has been determined, do not retest the samples unless approved by the Engineer. Ensure proper handling and storage of all cores until the LOT in question has been accepted.

**334-5.4.4 Individual Test Tolerances for QC Testing:** Terminate the LOT if any of the following QC failures occur:

- 1) An individual test result of a subplot for air voids does not meet the requirements of Table 334-5,
- 2) The average subplot density does not meet the requirements of Table 334-5,
- 3) Two consecutive test results within the same LOT for gradation or asphalt binder content do not meet the requirements of Table 334-5,

When a LOT is terminated due to a QC failure, stop production of the mixture until the problem is resolved to the satisfaction of the QC Manager and/or Asphalt Plant Level II technician responsible for the decision to resume production after a QC failure, as identified in Section 105. In the event that it can be demonstrated that the problem can immediately be or already has been resolved, it will not be necessary to stop production. When a LOT is terminated, make all necessary changes to correct the problem. Do not resume production until appropriate corrections have been made. Inform the Engineer of the problem and corrections made to correct the problem. After resuming production, sample and test the material to verify that the changes have corrected the problem. Summarize this information and provide it to the Engineer prior to the end of the work shift when production resumes.

In the event that a QC failure is not addressed as defined above, the Engineer's approval will be required prior to resuming production after any future QC failures.

Address any material represented by a failing test result in accordance with 334-5.9.5. Any LOT terminated under this subarticle will be limited to a maximum Pay Factor of 1.00 (as defined in 334-8.2) for each quality characteristic.

In the event that a  $G_{mm}$  test result differs by more than 0.040 from the mix design  $G_{mm}$ , investigate the causes of the discrepancy and report the findings and proposed actions to the Engineer.

Table 334-5 Master Production Range	
Characteristic	Tolerance <sup>(1)</sup>
Asphalt Binder Content (%)	Target $\pm 0.55$
Passing No. 200 Sieve (%)	Target $\pm 1.50$
Air Voids (%)	2.30 – 6.00
Density (minimum % $G_{mm}$ )	90.00
(1). Tolerances for sample size of $n = 1$ from the verified mix design (2). Based on an average of 5 randomly located cores	

SUBARTICLE 334-5.6.3 is deleted and the following substituted:

**334-5.6.3 Resolution Determination:** The Resolution test results (for the property or properties in question) will be compared with the QC test results based on the between-laboratory precision values shown in Table 334-6.

If the Resolution test results compare favorably with all of the QC results, then acceptance and payment for the LOT will be based on the QC results, and the Department will bear the costs associated with Resolution testing. No additional compensation, either monetary or time, will be made for the impacts of any such testing.

If the Resolution test results do not compare favorably with all of the QC results, then acceptance and payment for the LOT will be based on the Resolution test data for the LOT, and the costs of the Resolution testing will be deducted from monthly estimates. No additional time will be granted for the impacts of any such testing. In addition, in the event that the application of the Resolution test results in a failure to meet the requirements of Table 334-5, address any material represented by the failing test result in accordance with 334-5.9.5.

In the event of an unfavorable comparison between the Resolution test results and QC test results, make the necessary adjustments to assure that future comparisons are favorable.

SUBARTICLE 334-5.9.5 is deleted and the following substituted:

**334-5.9.5 Defective Material:** Assume responsibility for removing and replacing all defective material placed on the project, at no cost to the Department.

As an exception to the above and upon approval of the Engineer, obtain an engineering analysis in accordance with Section 6 by an independent laboratory (as approved by the Engineer) to determine the disposition of the material. The engineering analysis must be signed and sealed by a Professional Engineer licensed in the State of Florida.

The Engineer may determine that an engineering analysis is not necessary or may perform an engineering analysis to determine the disposition of the material.

Any material that remains in place will be accepted with a CPF as determined by 334-8, or as determined by the Engineer.

If the defective material is due to a gradation, asphalt binder content or density failure, upon the approval of the Engineer the Contractor may perform delineation tests on roadway cores in lieu of an engineering analysis to determine the limits of the defective material that may require removal and replacement. Prior to any delineation testing, all sampling locations shall be approved by the Engineer. All delineation sampling and testing shall be monitored and verified by the Engineer. For materials that are defective due to air voids, an engineering analysis is required.

When evaluating defective material by engineering analysis or delineation testing, at a minimum, evaluate all material located between passing QC, PC or IV test results. Exceptions to this requirement shall be approved by the Engineer.

ARTICLE 334-8 is deleted and the following substituted:

**334-8 Basis of Payment.**

**334-8.1 General:** Price and payment will be full compensation for all the work specified under this Section (including the applicable requirements of Sections 320 and 330).

For materials accepted in accordance with 334-5, based upon the quality of the material, a pay adjustment will be applied to the bid price of the material as determined on a LOT by LOT basis. The pay adjustment will be assessed by calculating a Pay Factor for the following individual quality characteristics: pavement density, air voids, asphalt binder content, and the percentage passing the No. 200 and No. 8 sieves. The pay adjustment will be computed by multiplying a Composite Pay Factor (CPF) for the LOT by the bid price per ton..

**334-8.2 Pay Factors:**

**334-8.2.1 Partial LOTs:** For Partial LOTs where no random sample is obtained due to insufficient tonnage, a CPF of 1.00 shall be applied.

**334-8.2.2 Two or Less Sublot Test Results:** In the event that two or less sublot test results are available for a LOT, Pay Factors will be determined based on Table 334-7, using the average of the accumulated deviations from the target value. (Deviations are absolute values with no plus or minus signs.) Use the 1-Test column when there is only one sublot test result and use the 2-Tests column when there are two sublots.

Table 334-7 Small Quantity Pay Table		
Pay Factor	1 Sublot Test Deviation	2 Sublot Test Average Deviation
Asphalt Binder Content		
1.05	0.00-0.23	0.00-0.16
1.00	0.24-0.45	0.17-0.32
0.90	0.46-0.55	0.33-0.39
0.80	>0.55	>0.39
No. 8 Sieve		
1.05	0.00-2.25	0.00-1.59
1.00	2.26-4.50	1.60-3.18
0.90	4.51-5.50	3.19-3.89
0.80	>5.50	>3.89
No. 200 Sieve		
1.05	0.00-0.55	0.00-0.39
1.00	0.56-1.10	0.40-0.78
0.90	1.11-1.50	0.79-1.06
0.80	>1.50	>1.06
Air Voids		
1.05	0.00-0.50	0.00-0.35
1.00	0.51-1.00	0.36-0.71
0.90	1.01-1.70	0.72-1.20
0.80	1.71-2.00	1.21-1.41
0.70	2.01-2.50	1.42-1.77
0.55	>2.50	>1.77
Density <sup>(1)</sup>		

Pay Factor	1 Sublot Test Deviation	2 Sublot Test Average Deviation
1.05	0.00-0.50	0.00-0.35
1.00	0.51-1.00	0.36-0.71
0.95	1.01-2.00	0.72-1.41
0.90	2.01-3.00	1.42-2.12
0.80	>3.00	>2.12

(1). Each density test result is the average of five cores. The target density is 93.00 percent of  $G_{mm}$  (92.00 percent when compaction is limited to the static mode or for layers specified to be one inch thick). When compaction is limited to the static mode, no vibratory mode in the vertical direction will be allowed. Other vibratory modes will be allowed, if approved by the Engineer. In this case, the target density is 92.00 percent of  $G_{mm}$ .

**334-8.2.3 Three or More Sublot Test Results:** When three or more sublot test results are available for a LOT, the variability-unknown, standard deviation method will be used to determine the estimated percentage of the LOT that is within the specification limits. The number of significant figures used in the calculations will be in accordance with requirements of AASHTO R11-06, Absolute Method.

**334-8.2.3.1 Percent Within Limits:** The percent within limits (PWL) and Pay Factors for the LOT will be calculated as described below. Variables used in the calculations are as follows:

- x = individual test value (sublot)
- n = number of tests (sublots)
- s = sample standard deviation
- $\Sigma(x^2)$  = summation of squares of individual test values
- $(\Sigma x)^2$  = summation of individual test values squared
- $Q_U$  = upper quality index
- USL = upper specification limit (target value plus upper specification limit from Table 334-8)
- $Q_L$  = lower quality index
- LSL = lower specification limit (target value minus lower specification limit from Table 334-8)
- $P_U$  = estimated percentage below the USL
- $P_L$  = estimated percentage above the LSL

(1) Calculate the arithmetic mean ( $\bar{X}$ ) of the test values:

$$\bar{X} = \frac{\sum x}{n}$$

(2) Calculate the sample standard deviation (s):

$$s = \sqrt{\frac{n \sum (x^2) - (\sum x)^2}{n(n-1)}}$$

(3) Calculate the upper quality index ( $Q_U$ ):

$$Q_U = \frac{USL - \bar{X}}{s}$$

(4) Calculate the lower quality index ( $Q_L$ ):

$$Q_L = \frac{\bar{X} - LSL}{s}$$

(5) From Table 334-9, determine the percentage of work below the USL ( $P_U$ ).

(6) From Table 334-9, determine percentage of work above the LSL ( $P_L$ ) Note: If USL or LSL is not specified; percentages within (USL or LSL) will be 100.

(7) If  $Q_U$  or  $Q_L$  is a negative number, then calculate the percent within limits for  $Q_U$  or  $Q_L$  as follows: enter Table 334-9 with the positive value of  $Q_U$  or  $Q_L$  and obtain the corresponding percent within limits for the proper sample size. Subtract this number from 100.00. The resulting number is the value to be used in the next step (Step 8) for the calculation of quality level.

(8) Calculate the percent within limits ( $PWL$ ) =  $(P_U + P_L) - 100$

(9) Calculate the Pay Factor (PF) for each quality characteristic using the equation given in 334-8.2.3.2.

Table 334-8 Specification Limits	
Quality Characteristic	Specification Limits
Passing No. 200 sieve ( percent)	Target $\pm$ 1.0
Asphalt Content ( percent)	Target $\pm$ 0.40
Air Voids ( percent)	4.00 $\pm$ 1.20
Density ( percent of $G_{mm}$ ):	93.00 + 2.00, - 1.20 <sup>(1)</sup>

(1): If the Engineer (or Contract Documents) limits compaction to the static mode only, or for all one-inch thick lifts, compaction shall be in the static mode. No vibratory mode in the vertical direction will be allowed. Other vibratory modes will be allowed, if approved by the Engineer. In either case, the specification limits will be as follows: 92.00 + 3.00, -1.20 percent of  $G_{mm}$ . No additional compensation, cost or time, shall be made.

Table 334-9 Percent Within Limits				
Quality Index	Percent within Limits for Selected Sample Size			
	n = 3	n = 4	n = 5	n = 6
0.00	50.00	50.00	50.00	50.00
0.05	51.38	51.67	51.78	51.84
0.10	52.76	53.33	53.56	53.67
0.15	54.15	55.00	55.33	55.50
0.20	55.54	56.67	57.10	57.32
0.25	56.95	58.33	58.87	59.14

Table 334-9 Percent Within Limits				
Quality Index	Percent within Limits for Selected Sample Size			
	n = 3	n = 4	n = 5	n = 6
0.30	58.37	60.00	60.63	60.94
0.35	59.80	61.67	62.38	62.73
0.40	61.26	63.33	64.12	64.51
0.45	62.74	65.00	65.84	66.27
0.50	64.25	66.67	67.56	68.00
0.55	65.80	68.33	69.26	69.72
0.60	67.39	70.00	70.95	71.41
0.65	69.03	71.67	72.61	73.08
0.70	70.73	73.33	74.26	74.71
0.75	72.50	75.00	75.89	76.32
0.80	74.36	76.67	77.49	77.89
0.85	76.33	78.33	79.07	79.43
0.90	78.45	80.00	80.62	80.93
0.95	80.75	81.67	82.14	82.39
1.00	83.33	83.33	83.64	83.80
1.05	86.34	85.00	85.09	85.18
1.10	90.16	86.67	86.52	86.50
1.15	97.13	88.33	87.90	87.78
1.20	100.00	90.00	89.24	89.01
1.25	100.00	91.67	90.54	90.19
1.30	100.00	93.33	91.79	91.31
1.35	100.00	95.00	92.98	92.37
1.40	100.00	96.67	94.12	93.37
1.45	100.00	98.33	95.19	94.32
1.50	100.00	100.00	96.20	95.19
1.55	100.00	100.00	97.13	96.00
1.60	100.00	100.00	97.97	96.75
1.65	100.00	100.00	98.72	97.42
1.70	100.00	100.00	99.34	98.02
1.75	100.00	100.00	99.81	98.55
1.80	100.00	100.00	100.00	98.99
1.85	100.00	100.00	100.00	99.36
1.90	100.00	100.00	100.00	99.65
1.95	100.00	100.00	100.00	99.85

Table 334-9 Percent Within Limits				
Quality Index	Percent within Limits for Selected Sample Size			
	n = 3	n = 4	n = 5	n = 6
2.00	100.00	100.00	100.00	99.97
2.05	100.00	100.00	100.00	100.00
2.10	100.00	100.00	100.00	100.00
2.15	100.00	100.00	100.00	100.00
2.20	100.00	100.00	100.00	100.00
2.25	100.00	100.00	100.00	100.00
2.30	100.00	100.00	100.00	100.00
2.35	100.00	100.00	100.00	100.00
2.40	100.00	100.00	100.00	100.00
2.45	100.00	100.00	100.00	100.00
2.50	100.00	100.00	100.00	100.00
2.55	100.00	100.00	100.00	100.00
2.60	100.00	100.00	100.00	100.00
2.65	100.00	100.00	100.00	100.00

**334-8.2.3.2 Pay Factors (PF):** Pay Factors will be calculated by using the following equation:

$$\text{Pay Factor} = (55 + 0.5 \times \text{PWL}) / 100$$

The PWL is determined from Step (8) of 334-8.2.3.1.

**334-8.3 Composite Pay Factor (CPF):** A CPF for the LOT will be calculated based on the individual PFs with the following weighting applied: 35% Density (D), 25% Air Voids ( $V_a$ ), 25% asphalt binder content ( $P_b$ ), 10% Passing No. 200 ( $P_{-200}$ ) and 5% Passing No. 8 ( $P_{.8}$ ). Calculate the CPF by using the following formula:

$$\text{CPF} = [(0.350 \times \text{PF } D) + (0.250 \times \text{PF } V_a) + (0.250 \times \text{PF } P_b) + (0.100 \times \text{PF } P_{-200}) + (0.050 \times \text{PF } P_{.8})]$$

Where the PF for each quality characteristic is determined in either 334-8.2.2 or 334-8.2.3, depending on the number of subplot tests. Note that the number after each multiplication will be rounded to the nearest 0.01.

The pay adjustment shall be computed by multiplying the CPF for the LOT by the bid price per ton.

**334-8.4 Payment:** Payment will be made under:

Item No. 334- 1- Superpave Asphaltic Concrete - per ton.