



# Designing for Constructability

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Topics for  
Designing for  
Constructability

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  - Contractor's Responsibility
- 3 Structures Design Guidelines
  - Design Requirements
  - Deck Casting on Continuous Steel Girders
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## Stability of Partially Completed Superstructures



### Bridge Failure during Construction

On May 15, 2004 in Golden Colorado, a steel girder on an interstate overpass bridge widening project collapsed onto an interstate killing two adults and a child travelling in an SUV.



## NTSB Conclusions

- The accident was caused by the failure of the contractor's temporary bracing system.
- Issues with the Colorado DOT:
  - Lack of uniform and consistent bracing standards
  - Narrow definition of falsework, which did not include lateral bracing
  - Failure to effectively oversee safety-critical contract work for the project



## FDOT Construction Specifications

Section 5 Control of the Work

Section 7 Legal Requirements and Responsibility to the Public

Section 103 Temporary Works

Sections 450 and 460



# FDOT Construction Specifications

**Section 5 Control of the Work**

**Work Items Requiring Shop Drawings**

- Bridge components not fully detailed in plans
- Temporary works affecting public safety
- Beam and girder temporary bracing
- Major and Unusual Structures

**Review by the Engineer**

- Conformity to the Contract Documents
- Construction Affecting Public Safety

**Certifications**

- Inspects item
- Special Erection Equipment, Fullwork, Temporary Structures

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# FDOT Construction Specifications

**SAFETY FIRST**

Section 7 Legal Requirements and Responsibility to the Public

Control of the Contactor's Equipment

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# FDOT Construction Specifications



103 - Temporary Works



Solely to support construction equipment



# FDOT Construction Specifications



Sections 450 and 460



450-16 Handling, Storage, Shipping and Erection



460-7 Erection Plan



## Stability of Partially Completed Superstructures



## Structures Design Guidelines

Chapter 6.10

Erection Scheme and Girder Stability

Chapter 2

Loads and Load Factors

(Wind & Construction Loads)

Chapter 4/5

Girder Transportation



# Structures Design Guidelines

## Chapter 6.10 Erection Scheme and Beam/Girder Stability



# Structures Design Guidelines



Section 2.4.3 Wind Loads During Construction

2:00 PM  
**Designing for Constructability**  
 Until 2:50 PM

3:00 PM  
**Wind Loads During Construction**  
 Until 3:50 PM




**2.13.1 Constructability Limit State Checks**

In the absence of more accurate information, the following construction loads can be assumed for investigation of the strength and service limit states during construction in accordance with **LRFD [3.4.2]** and **SDG 2.4.3**, and for investigation of deck overhang bracket force effects in accordance with **LRFD [6.10.3.4]**. These loads are applicable to conventional beam or girder superstructures with cast-in-place decks. All construction loads assumed in the design of the structure shall be listed in the plans.

A. **Finishing machine load.** The finishing machine load shall be per the manufacturer's specifications and be applied as a moving load positioned to produce the maximum response. In the absence of manufacturer's specifications, assume the following loads:

W = Bridge Width (ft)	Total Weight of Finishing Machine (kips)
26 ≤ W ≤ 32	7
32 < W ≤ 56	11
56 < W ≤ 80	13
80 < W ≤ 120	16

B. Construction live load: **20 klf** extended over the entire bridge width and **66 feet** longitudinal length centered on the finishing machine.

C. Removable deck cantilever forms with **overhang brackets: 16 klf**

D. Live load at or near the outside edge of deck during deck placement: **75 klf** applied as a moving load over a length of **20 feet** and positioned to produce the maximum response.



## Stages of PS Beam Stability

- 1) Beam lifting
  - a) Forms → dunnage
  - b) dunnage → truck
  - c) truck → bearings
- 2) Beam on dunnage
- 3) Beam on truck
- 4) Crane release (single beam on bearings)
- 5) Two beams erected and braced at ends
- 6) All beams during deck casting

**Fabricator & Contractor**

**Project EOR**

Section 4.3.4 Temporary  
Bracing (Beam Stability)

Contractor  
responsible for all  
aspects of  
handling, shipping,  
and erection.

Designer  
responsible for  
specifying bracing  
requirements for  
stability of the  
beam once seated  
on the  
substructure.



## FDOT Mathcad Program Three Stages of Construction



- **Crane Release** (single beam on bearings)  
Active Construction -- Wind Speed = 20 mph  
- Is anchor bracing required?



- **Braced Beams** (braced at ends)  
Inactive Construction –  
Wind Speed = 0.6 \* Design Wind Speed  
– Is intermediate bracing required?

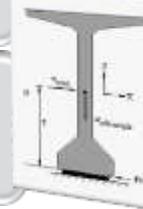


- **Deck Casting**  
Wind Speed = 20 mph  
All loads during deck pour  
- Is intermediate bracing required?

### Beam Stability

Service 1 Limit State  
( $\gamma_{DC} = \gamma_{WS} = 1.0$ )

Cracking – FOS of 1.0  
Rollover – FOS of 1.5



# FDOT Mathcad Program Three Stages of Construction

**TABLE OF TEMPORARY BRACING VARIABLES**

Table Data 2 of 4

SPAN NO.	SPAN LENGTH (FT)	STAGE 2		STAGE 3		STAGE 1	STAGES 2 & 3	
		UNBRACED LENGTH (FT)	BRACE (KIP)	FORCE AT EACH INTERMEDIATE SPAN BRACE (KIP)	MOMENT AT EACH BEAM END AND ANCHOR BRACE (KIP-FT)	OVERTURNING MOMENT AT EACH INTERMEDIATE SPAN BRACE (KIP-FT)	BRACE END PRIOR TO CRANE RELEASE?	TOTAL LINES OF BRACING
							YES/NO	
							YES/NO	
							YES/NO	
							YES/NO	

COMING SOON TDB July 2015



## End of First Day of Beam Placement



# Structures Design Guidelines



Chapter 4 & 5 Superstructure Concrete / Steel (4.1.3 and 5.1.2) **Girder Transportation**



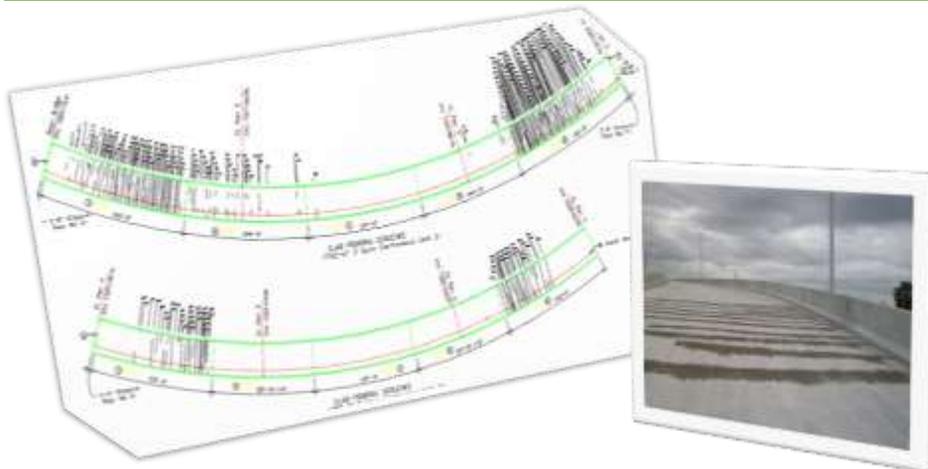
The EOR is responsible for investigating the feasibility of transportation of heavy, long and/or deep girders

- A. Longer than 145 /130 feet
- B. Weighing more than 160,000 pounds (requires coordination through the Department's Permit Office)
- C. Depths exceeding 9'-0"



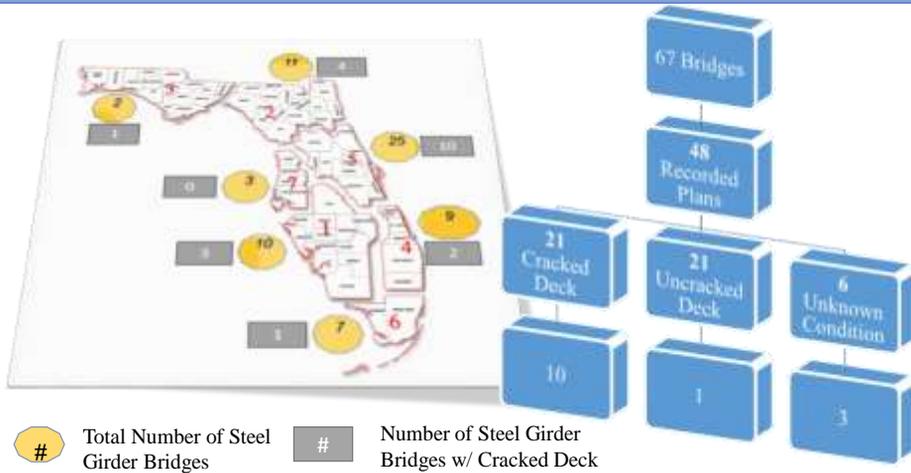
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## CIP Decks on Continuous Steel Girder Bridges



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## FDOT SURVEY

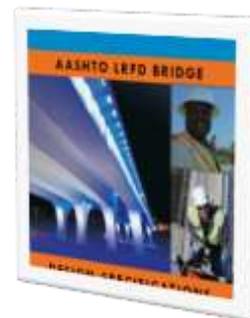
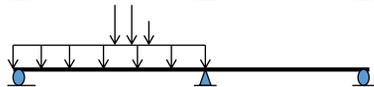
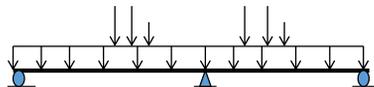


## AASHTO LRFD BDS

- 6.10.1.7 & 6.10.3.2.4 –
  - **Factored Construction Loads = 1.25 DL + 1.5 CL**



- Load Combination Service II = 1.0 DL + 1.3 (LL+IM)

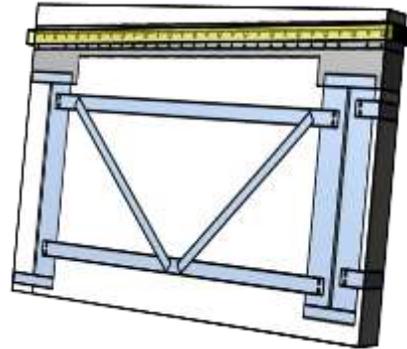


- Check Deck Tensile Stresses < Modulus of Rupture ( $\Phi f_r = 0.24\sqrt{f'_c}$ )
- 1% Reinforcing Steel Requirement → 2/3 Top Layer

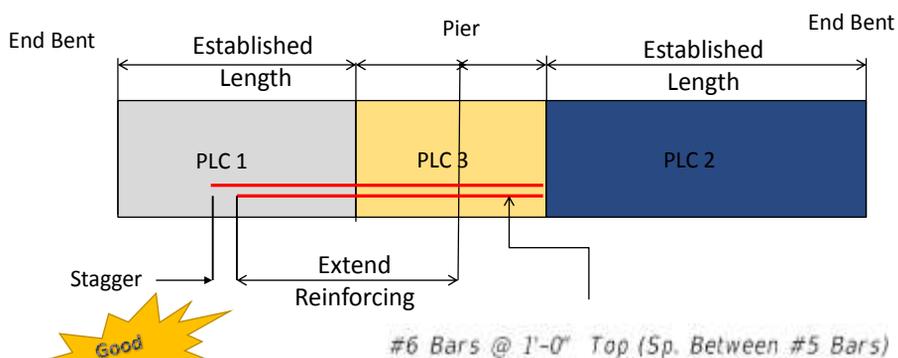


## Reinforcing Detailing

- SDG 4.2.11 Temp & Shrinkage
  - **No. 4 Min. at 12-inch Sp. Max.**
- LRFD BDS 6.10.1.7
  - **No. 6 Max., 12-inch Sp. Max.**
  - **1% Steel – 2/3 in top layer**



## Reinforcing Detailing



## Global Stability of Narrow Steel I-Girder Systems

- Narrow ramp bridges
- Phased construction
- Widening

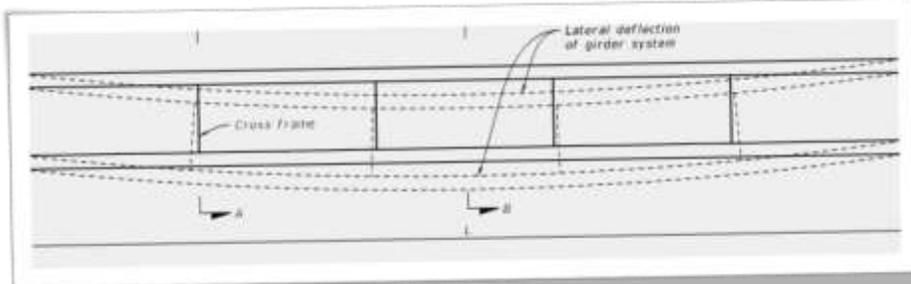
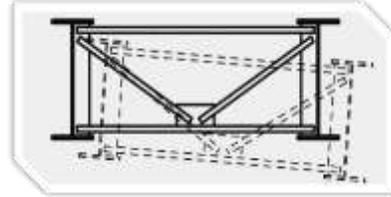


# What is Global Buckling?

Definition being added to AASHTO LRFD

## Global Lateral-Torsional Buckling:

Buckling mode in which a **system of girders buckle as a unit** with an unbraced length equal to the **clear span** of the girders.



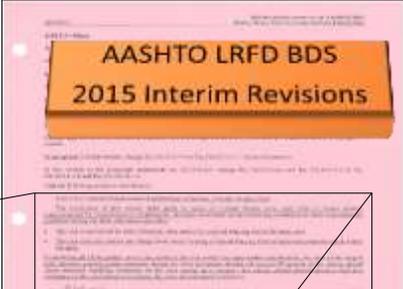
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## Global Buckling



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# Global Disp. Ampl.



**6.10.3.4.2 Global Displacement Amplification on Slender I-Girder Bridge Units**

The provisions of this Article shall apply to spans of I-girder bridge units with three or fewer girders, interconnected by cross-frames or diaphragms, that also meet both of the following conditions in their formcomposite condition during the deck placement operation:

- The unit is not braced by other structural units and/or by external bracing within the span, and
- The unit does not contain any flange level lateral bracing or lateral bracing from a hardened composite deck within the span.

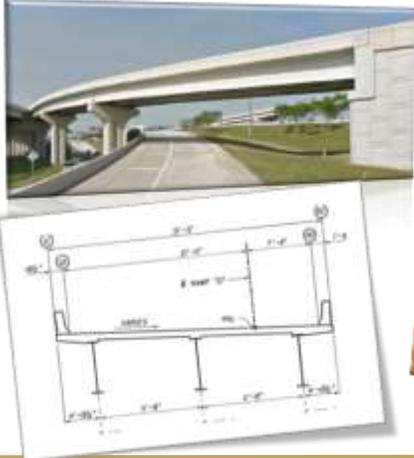
Considering all of the girders across the width of the unit within the span, under consideration, the sum of the largest total flexural positive girder moments during the deck placement should not exceed 50 percent of the elastic global lateral-torsional buckling resistance of the span acting as a system. The elastic global lateral-torsional buckling resistance of the unit acting as a system,  $M_{cr}$ , may be calculated as follows:

$$M_{cr} = \frac{\pi^2 EI_y}{L^2} \sqrt{I_{xt}^2 + I_{xt} I_{yt}}$$

(6.10.3.4.2.1)



# Global Displacement Amplification

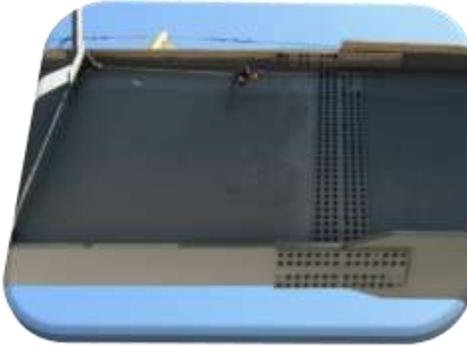


- Three-span continuous unit
- 167-ft maximum span c/c bearings
- 3 girder lines at 11'-8"
- AASHTO 6.10.3.4.2
- $M_u/M_{cr} = 118\% > 50\%$
- **No Good**

How much lateral bracing? (or) Evaluate 2<sup>nd</sup> order displacements



## What's New – Construction Specs



- 460-7.7
- 50% of the web connections
- 50% of the flange connections



## What's New – Construction Specs



- Diaphragms or cross-frames
- Uniformly distributed between all the bolt groups
- The 50% requirement may be waived if a reduced percentage is calculated as sufficient and shown on the approved erection plan



## What's New – Construction Specs



- 400-4.4 Bridge Deck Overhang Falsework for Steel I-Girders



- Locate within 6 inches above the bottom flange.
- If more than 6 inches or if the deck overhang is 4 feet or greater, submit shop drawings and calculations to the Engineer in accordance with Section 5 and Chapter 11 of the Structures Design Guidelines (SDG).



## Designing for Constructability

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Questions/Comments

